

BAB VI

KESIMPULAN DAN SARAN

6.1. Kesimpulan

Berdasarkan hasil penelitian mengenai Pengaruh Variasi Kadar *Silica Fume* terhadap Sifat Mekanik *Self-Compacting Fibre Reinforced Concrete* (SCFRC) ini, dapat ditarik kesimpulan seperti tercantum di bawah ini.

1. Berdasarkan hasil pengujian karakteristik beton segar SCFRC dengan metode *Slump flow*, T_{500} *slump flow*, *V-funnel*, *L-shaped box*, dan *J-ring*, semua variasi sampel menunjukkan bahwa SCFRC memenuhi syarat karakteristik beton segar SCC yaitu *filling ability*, *passing ability*, dan *viscosity*.
2. Nilai kuat tekan beton SCC tanpa serat dan beton SCC dengan penambahan serat (SCFRC) secara berturut-turut adalah 40,23 MPa dan 33,07 MPa. Penambahan serat *polypropylene* sebanyak 0,6 kg/m³ pada SCFRC menurunkan kuat tekan sebesar 17,8% dibandingkan dengan beton SCC tanpa serat.
3. Nilai kuat tekan SCFRC dengan variasi kadar *silica fume* 0%, 5%, 10%, dan 15% secara berturut-turut adalah 33,07 MPa, 41,06 MPa, 50,38 MPa, dan 44,29 MPa. Hasil tertinggi terdapat pada penambahan *silica fume* dengan kadar 10%, yaitu meningkat 52,35% dibandingkan dengan SCFRC tanpa *silica fume*.
4. Nilai kuat tarik belah beton SCC tanpa serat dan beton SCC dengan penambahan serat (SCFRC) secara berturut-turut adalah 3,39 MPa dan

3,70 MPa. Penambahan serat *polypropylene* sebanyak 0,6 kg/m³ pada SCFRC menaikkan kuat tarik belah sebesar 8,97% dibandingkan dengan beton SCC tanpa serat.

5. Nilai kuat tarik belah SCFRC dengan variasi kadar *silica fume* 0%, 5%, 10%, dan 15% secara berturut-turut adalah 3,70 MPa, 3,62 MPa, 4,10 MPa, dan 3,79 MPa. Hasil tertinggi terdapat pada penambahan *silica fume* dengan kadar 10%, yaitu meningkat 10,92% dibandingkan dengan SCFRC tanpa *silica fume*.
6. Nilai kuat lentur beton SCC tanpa serat dan beton SCC dengan penambahan serat (SCFRC) secara berturut-turut adalah 4,04 MPa dan 4,20 MPa. Penambahan serat *polypropylene* sebanyak 0,6 kg/m³ pada SCFRC menaikkan kuat lentur sebesar 3,96% dibandingkan dengan beton SCC tanpa serat.
7. Nilai kuat lentur SCFRC dengan variasi kadar *silica fume* 0%, 5%, 10%, dan 15% secara berturut-turut adalah 4,20 MPa, 5,05 MPa, 6,11 MPa, dan 6,01 MPa. Hasil tertinggi terdapat pada penambahan *silica fume* dengan kadar 10%, yaitu meningkat 45,60% dibandingkan dengan SCFRC tanpa *silica fume*.
8. Variasi kadar *silica fume* yang paling optimal pada penelitian ini adalah dengan penambahan 10% *silica fume* sebagai substitusi semen. Hal ini terbukti dengan terjadi peningkatan terbesar pada kuat tekan, kuat tarik belah, dan kuat lentur SCFRC.

6.2. Saran

Saran yang dapat penulis berikan setelah melihat hasil penelitian ini adalah seperti tercantum di bawah ini.

1. Perlu dilakukan penelitian lebih lanjut mengenai sifat beton segar SCC terhadap ketahanan segregasi agar parameter beton segar SCFRC ini menjadi lebih lengkap.
2. Dapat dilakukan penelitian lebih lanjut tentang penggunaan kadar *silica fume* antara 10% – 15% pada SCFRC agar didapat hasil yang lebih rinci mengenai kadar optimumnya.
3. Dalam proses pencampuran bahan campuran beton yang digunakan perlu ketelitian agar bahan yang digunakan tidak ada yang terbuang.
4. Pentingnya mengetahui cara penggunaan alat uji beton segar SCC agar selama proses pengambilan data menjadi lebih lancar.
5. Perlu dilakukan penelitian dan pengkajian lebih lanjut mengenai pemanfaatan SCFRC ini dalam dunia konstruksi beton.

DAFTAR PUSTAKA

- Amri, S., 2005, *Teknologi Beton A-Z*, Penerbit Yayasan John Hi-Tech Idetama, Jakarta.
- Antoni dan Nugraha, P., 2007, *Teknologi Beton*, Penerbit ANDI, Yogyakarta.
- Antonius dan Setiyawan, P., 2006, *Kajian Besaran Mekanis Beton Berserat Mutu Tinggi (Studi Eksperimen)*, Jurnal Teknik Sipil, Politeknik Negeri Semarang Akreditasi No: 49/Dikti/Kep/2004
- Arde, 2005, Penggunaan Polypropylene Fiber Dintinjau terhadap Mekanisme Tekan dan Lentur pada Campuran Beton Normal, *Laporan Penelitian Tugas Akhir UPN "Veteran" Jawa Timur*, Surabaya.
- ASTM C.33 - 02a, 2002, *Standard Specification for Concrete Aggregates*, Annual Books of ASTM Standards ,USA.
- ASTM. 1982. *Standard Specification for Chemical Admixture for Concrete*, American Society for Testing Materials, ASTM C 494-82 Philadelphia.
- Campion, Michael J dan Josh, P., 2000, *Self Compacting Concrete Expanding the Possibilities of Concrete Design and Placement*, Concrete International, April. 31-34.
- Citrakusuma dan Juwita L., 2012, Kuat Tekan *Self Compacting Concrete* dengan Kadar Superplasticizer yang Bervariasi, *Laporan Penelitian Tugas Akhir Universitas Jember*, Jember.
- Dehn, F., Holschemacher, K. dan Weiße, D., 2000, *Self-Compacting Concrete (SCC) Time Development of the Material Properties and the Bond Behaviour*, LACER No.5., Leipzig.
- Dina, 1999, Pengaruh Penggunaan Polypropylene Fiber Terhadap Penyusutan Pada Saat Pre-hardening Stage, *Laporan Penelitian Tugas Akhir UPN "Veteran" Jawa Timur*, Surabaya.
- Dipohusodo, I., 1996, *Struktur Beton Bertulang*, Penerbit PT. Gramedia Pustaka Utama, Jakarta.
- EFNARC, 2002, *Specification and Guidelines for Self-Compacting Concrete*.
- EFNARC, BIBM, CEMBUREAU, EFCA, ERMCO, 2005, *The European Guidelines for Self-Compacting Concrete*.

- Hannant, D.J., 1978, *Fiber Cements and Fiber Concretes*, John Wiley & Sons, Chichester.
- Japan Society of Civil Engineers. 2007. *Standart specifications for Concrete Structures "Materials and Construction"*.
- Kartini, W., 2007, *Penggunaan Serat Polypropylene untuk Meningkatkan Kuat Tarik Belah Beton*, Jurnal Rekayasa Perencanaan, Vol. 4, No. 1.
- Kusumo, A.D., 2013, Pengaruh Penambahan Serat Baja Lokal (Kawat Bendrat) pada Beton Memadat Mandiri (*Self Compacting Concrete*), *Laporan Tugas Akhir Universitas Atma Jaya Yogyakarta*, Yogyakarta.
- Mariani, S.V., Ahmad, A.G., 2009, *Pengaruh Penambahan Admixture terhadap Karakteristik Self Compacting Concrete (SCC)*, Jurnal SMARTek, Vol.7, No.3, pp 176-183.
- Mindess, S., Young, J.F., dan Darwin, D., 2003, *Concrete second edition*, Prentice Hall, New Jersey.
- Miranty, R., 2014, Pengaruh Penggunaan Silica Fume, Fly Ash dan Superplasticizer Pada Beton Mutu Tinggi Memadat Mandiri, *Laporan Penelitian Tugas Akhir Universitas Atma Jaya Yogyakarta*, Yogyakarta.
- Mulyono, T., 2004, *Teknologi Beton*, Penerbit ANDI, Yogyakarta.
- Nawy, E.G., (Terjemahan), 1998, *Beton Bertulang Suatu Pendekatan Mendasar*, Refika Aditama, Bandung.
- Neville, A.M., Brooks, J.J., *Concrete Technology*, Longman Group Ltd, London.
- Okamura, H. dan Ozawa, K., 1994, *Self-Compacting High-Performance Concrete in Japan*, ACI SP-159 : International Workshop on High Performance Concrete, Michigan.
- Ouchi, N., Osterberg, Hallberg, dan Lwin, 2003, Applications of Self Compacting Concrete in Japan, Europe and The United States.
- Persson, B., 2000, *A Comparison Between Mechanical Properties of Self-Compacting Concrete and the Corresponding Properties of Normal Concrete*, Cement and Concrete Research, Vol. 31, Pergamon.
- Prastiya, D.D. dan Fachrudin, N.M, 2006, Analisis Pengaruh Penambahan Serat Polypropylene terhadap Kuat Tekan dan Kuat Tarik Beton Mutu K500, *Laporan Penelitian Tugas Akhir Universitas Diponegoro*, Semarang.

PT. Sika Indonesia, 2005, *Sika Fibre*, Technical Data Sheet.

PT. Sika Indonesia, 2005, *Sika Fume*, Technical Data Sheet.

PT. Sika Indonesia, 2013, *Viscocrete-1003*, Product Data Sheet.

Silitonga, D., 2011, Pengaruh Pemakaian Portland Composite Cement (PCC) terhadap Ketahanan Sulfat pada Self Compacting Concrete (SCC), *Laporan Penelitian Tugas Akhir Universitas Indonesia*, Jakarta.

SNI 03-1974-1990, *Metode Pengujian Kuat Tekan Beton*, Badan Standardisasi Nasional.

SNI 03-2491-2002, *Metode Pengujian Kuat Tarik Belah Beton*, Badan Standardisasi Nasional.

SNI 03-2834-2000, *Tata Cara Pembuatan Rencana Campuran Beton Normal*, Badan Standardisasi Nasional.

SNI 03-4431-2011, *Cara Uji Kuat Lentur Beton Normal dengan Dua Titik Pembebanan*, Badan Standardisasi Nasional.

SNI 2847:2013, *Persyaratan Beton Struktural untuk Bangunan Gedung*, Badan Standardisasi Nasional.

Tjaronge, M.W., Akkas, Abd. M., Masdar, J., Studi Pengaruh Serat Polypropylene (PP) terhadap Kuat Tekan dan Tarik Belah Self Compacting Concrete (SCC), *Laporan Penelitian Tugas Akhir Universitas Hasanuddin*, Makassar.

Tjokrodimuljo, K., 1996, *Teknologi Beton*, Buku Ajar, Jurusan Teknik Sipil, Fakultas Teknik, Universitas Gadjah Mada, Yogyakarta.

Widodo, S., 2008, Uji Karakteristik Beton Segar Akibat Penambahan Serat Polypropylene Dalam Adukan Self-Consolidating Concrete, *Prosiding Seminar Nasional Aplikasi Teknologi Prasarana Perkotaan*, ISBN 978-979-18342-0-9, B-154.

Yunita, N., 2008, Rancang Campur High Strength Self Compacting Concrete (HSSCC) dengan Menggunakan Adva Superplasticizer, *Laporan Penelitian Tugas Akhir Universitas Indonesia*, Jakarta.

Zai, K.A., Karolina, R., Syahrizal, 2014, *Pengaruh Penambahan Silica Fume dan Superplasticizer Terhadap Kuat Tekan Beton Mutu Tinggi Dengan Metode ACI (American Concrete Institute)*, Jurnal Teknik Sipil USU, Vol. 3, No.2.





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Lampiran 1
Halaman 94

Laporan No. : Dikerjakan :
Pekerjaan : Diperiksa :
..... Tgl. Pemeriksaan :

ANALISIS SARINGAN AGREGAT HALUS

BERAT KERING : 1000 gram						
Nomor Saringan	B.Saringan (gram)	Berat Saringan + Tertahan (gram)	B.Tertahan (gram)	Σ B.Tertahan (gram)	Percentase	
					B.Tertahan %	Lolos %
3/4" (19,1 mm)	572	572	0	0	0	100
1/2" (12,7 mm)	454	454	0	0	0	100
3/8" (9,52mm)	460	460	0	0	0	100
No.4(4,75 mm)	532	535	3	3	0.3	99.7
No.8(2,36 mm)	327	349	22	25	2.5	97.5
No.30(0,60mm)	293	774	481	506	50.6	49.4
No.50(0,30mm)	378	599	221	727	72.7	27.3
No.100(0,15mm)	352	534	182	909	90.9	9.1
No.200(0,75mm)	338	405	67	976	97.6	2.4
PAN	374	398	24	1000	100	0

Masuk Gradasi Pasir No. 2 (Sedang)

$$\text{MHB Agregat Halus} = \frac{3146}{1000} = 3.146$$



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Lampiran 1
Halaman 95

Laporan No. : Dikerjakan :
Pekerjaan : Diperiksa :
..... Tgl. Pemeriksaan :

PEMERIKSAAN
BERAT JENIS & PENYERAPAN AGREGAT HALUS

	NOMOR PEMERIKSAAN	I
A	Berat Contoh Jenuh Kering Permukaan (SSD) – (500)	500
B	Berat Contoh Kering	495
C	Berat Labu + Air , Temperatur 25° C	712
D	Berat Labu+Contoh (SSD) + Air, Temperatur 25° C	1029
E	Berat Jenis Bulk $= \frac{(B)}{(C + 500 - D)}$	2.73
F	BJ.Jenuh Kering Permukaan(SSD) $= \frac{(B)}{(C + 500 - D)}$	2.70
G	Berat Jenis Semu (Apparent) $= \frac{(B)}{(C + B - D)}$	2.78
H	Penyerapan (Absorption) $= \frac{(500 - B)}{(B)} \times 100 \%$	1.01%

PERSYARATAN UMUM :

- Absorption : 5%
- Berat Jenis :



PEMERIKSAAN KANDUNGAN LUMPUR AGREGAT HALUS

- I. Waktu pemeriksaan: 29 Maret 2016
- II. Bahan
 - a. Pasir kering tungku, asal : Kali Progo, berat : 100 gram
 - b. Air jernih asal : LSBB Prodi TS FT-UAJY
- III. Alat
 - a. Gelas ukur, ukuran: 250 cc
 - b. Timbangan
 - c. Tungku (*oven*), suhu antara 105-110°C
 - d. Pasir + piring masuk tungku tanggal 29 Maret 2016 jam 10.00 WIB
- IV. Hasil
Pasir + piring keluar tungku tanggal 30 Maret 2016 jam 11.00 WIB
 - a. Berat pasir = 100 gram
 - b. Berat pasir kering oven = 99 gram

$$\text{Kandungan lumpur} = \frac{100 - 99}{100} \times 100\% = 1\%$$

Kesimpulan: Kandungan lumpur 1% < 5%, syarat terpenuhi (OK)



PEMERIKSAAN KANDUNGAN ZAT ORGANIK AGREGAT HALUS

I. Waktu pemeriksaan : 29 Maret 2016

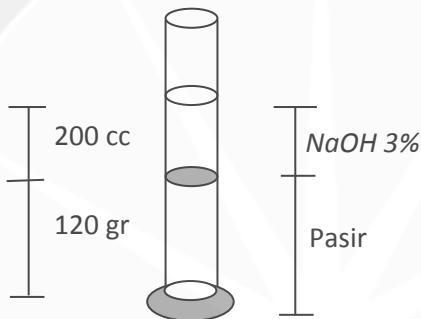
II. Bahan

- Pasir kering tungku, asal : Kali Progo, berat : 120 gram
- Larutan NaOH 3%

III. Alat

Gelas ukur, ukuran : 250 cc

IV. Sketsa



V. Hasil

Setelah didiamkan selama 24 jam, warna larutan di atas pasir sesuai dengan warna *Gardner Standard Color* sesuai dengan No. 8.

Kesimpulan: Warna *Gardner Standard Color* No. 8 yaitu kuning muda, maka syarat terpenuhi (OK).



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Lampiran 2
Halaman 98

Laporan No. : Dikerjakan :
Pekerjaan : Diperiksa :
..... Tgl. Pemeriksaan :

ANALISIS SARINGAN AGREGAT KASAR

BERAT KERING : 1000 gram						
Nomor Saringan	B.Saringan (gram)	Berat Saringan + Tertahan (gram)	B.Tertahan (gram)	Σ B.Tertahan (gram)	Percentase	
					B.Tertahan %	Lolos %
3/4" (19,1 mm)	572	572	0	0	0	100
1/2" (12,7 mm)	455	527	72	72	7.2	92.8
3/8" (9,52mm)	460	858	398	470	47	53
No.4(4,75 mm)	532	1042	510	980	98	2
No.8(2,36 mm)	327	335	8	988	98.8	1.2
No.30(0,60mm)	293	296	3	991	99.1	0.9
No.50(0,30mm)	378	380	2	993	99.3	0.7
No.100(0,15mm)	353	355	2	995	99.5	0.5
No.200(0,75mm)	338	340	2	997	99.7	0.3
PAN	374	377	3	1000	100	0

$$\text{MHB Agregat Kasar} = \frac{6486}{1000} = 6.486$$



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Lampiran 2
Halaman 99

Laporan No. : Dikerjakan :
Pekerjaan : Diperiksa :
..... Tgl. Pemeriksaan :

PEMERIKSAAN
BERAT JENIS & PENYERAPAN AGREGAT KASAR

	NOMOR PEMERIKSAAN	I
A	Berat Contoh Kering	972
B	Berat Contoh Jenuh Kering Permukaan (SSD)	1000
C	Berat Contoh Dalam Air	618
D	Berat Jenis Bulk	$= \frac{(A)}{(B)-(C)}$ 2.54
E	BJ.Jenuh Kering Permukaan (SSD)	$= \frac{(B)}{(B)-(C)}$ 2.62
F	Berat Jenis Semu (Apparent)	$= \frac{(A)}{(A)-(C)}$ 2.75
G	Penyerapan (Absorption)	$= \frac{(B)-(A)}{(A)} \times 100 \%$ 2.88%

PERSYARATAN UMUM :

- Absorption : 5%
- Berat Jenis :



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Lampiran 2
Halaman 100

Laporan No. : Dikerjakan :
Pekerjaan : Diperiksa :
..... Tgl. Pemeriksaan :

**PEMERIKSAAN KEAUSAN AGREGAT
DENGAN MESIN LOS ANGELES**

GRADASI SARINGAN		NOMOR CONTOH	
		I	II
LOLOS	TERTAHAN	BERAT MASING-MASING AGREGAT	BERAT MASING-MASING AGREGAT
3/8"	1/4"	2500	-
1/4"	No. 4	2500	-

NOMOR CONTOH	I
BERAT SEBELUMNYA (A)	5000 gram
BERAT SESUDAH DIAYAK SARINGAN NO.12 (B)	3929 gram
BERAT SESUDAH (A)-(B)	1071 gram
KEAUSAN = $\frac{(A)-(B)}{(A)} \times 100\%$	21.42 %

UKURAN SARINGAN		BERAT AGREGAT			
LOLOS	TERTAHAN	A	B	C	D
1 1/2"	1"	1250			
1"	3/4"	1250			
3/4"	1/2"	1250	2500		
1/2"	3/8"	1250	2500		
3/8"	1/4"			2500	
1/4"	No. 4			2500	
No. 4	No. 8				5000
TOTAL		5000	5000	5000	5000
JUMLAH BOLA BAJA		12	11	8	6



PERHITUNGAN MIX DESIGN

(SNI 03-2834-2000)

A. Data Bahan

1. Bahan Agregat halus (pasir) : Kali Progo, Yogyakarta.
2. Bahan Agregat kasar : Clereng, Yogyakarta.
3. Jenis semen : Semen PPC merek Gresik

B. Data Specific Gravity

1. *Specific gravity* agregat halus (pasir) : 2,70
2. *Specific gravity* agregat kasar (krikil) : 2,62

C. Hitungan

1. Kuat tekan beton yang disyaratkan (f'_c) pada umur 28 hari. $F'_c = 40$ MPa.
2. Menentukan nilai devisiasi standar berdasarkan tingkat mutu pengendalian pelaksanaan campuran.
3. Nilai margin ditentukan sebesar 12 MPa karena jumlah benda uji yang kurang dari 15 buah
4. Menetapkan kuat tekan beton rata-rata yang direncanakan berdasarkan SNI butir 4.2.3.1 3.

$$f_c = f_c' + M = 40 + 12 = 52 \text{ MPa.}$$

5. Menentukan jenis semen
Jenis semen PPC dengan merek Gresik.
6. Menetapkan jenis agregat
 - a) Agregat halus : Pasir alam (Golongan 2).
 - b) Agregat kasar : Batu Pecah



7. Menetapkan faktor air-semen, berdasarkan jenis semen yang dipakai dan kuat tekan rata-rata silinder beton yang direncanakan pada umur tertentu.
Direncanakan sebesar 0,31
8. Menetapkan faktor air semen maksimum.

Persyaratan Jumlah Semen Minimum dan Faktor Air Semen Maksimum Untuk Berbagai Macam Pembetonan dalam Lingkungan Khusus

Lokasi	Jumlah Semen minimum Per m ³ beton (kg)	Nilai Faktor Air Semen Maksimum
Beton di dalam ruang bangunan :		
a. Keadaan keliling non-korosif	275	0,6
b. Keadaan keliling korosif disebabkan oleh kondensasi atau uap korosif	325	0,52
Beton diluar ruangan bangunan :		
a. tidak terlindung dari hujan dan terik matahari langsung	325	0,60
b. terlindung dari hujan dan terik matahari langsung	275	0,60
Beton masuk kedalam tanah :		
a. mengalami keadaan basah dan kering berganti-ganti	325	0,55
b. mendapat pengaruh sulfat dan alkali dari tanah		Lihat Tabel 5
Beton yang kontinu berhubungan:		
a. Air tawar		Lihat Tabel 6
b. Air laut		

(Sumber : SNI 03-2834-2000 : Tabel 4)

Berdasarkan Tabel 4 SNI 03-2834-2000, untuk beton dalam ruang bangunan sekeliling non-korosif fas maksimum 0,6. Dibandingkan dengan No.7, dipakai terkecil. Jadi digunakan fas 0,31.

9. Menetapkan nilai *slump*, direncanakan sebesar 60-180 mm
10. Ukuran butiran maksimum (krikil) adalah 10 mm.
11. Menetapkan jumlah air yang diperlukan tiap m³ beton.
 - a) Ukuran butir maksimum 10 mm.



- b) Nilai *Slump* 60-180 mm.
- c) Agregat halus berupa batu tak di pecah, maka $W_h = 225$
- d) Agregat kasar berupa batu pecah, maka $W_k = 250$

$$W = \frac{2}{3}W_h + \frac{1}{3}W_k$$

dengan : W_h adalah perkiraan jumlah air untuk agregat halus

W_k adalah perkiraan jumlah air untuk agregat kasar

$$W = \frac{2}{3} \times 225 + \frac{1}{3} \times 250 = 233,25 \text{ liter}$$

12. Menghitung berat semen yang diperlukan :

- a) Berdasarkan tabel 4 SNI 03-2834-2000, diperoleh semen minimum 275 kg.

b) Berdasarkan $f_{as} = 0,31$. Semen per m^3 beton = $\frac{A}{f_{as}} = \frac{233,25}{0,31}$
 $= 752,419 \text{ kg}$

Dipilih berat semen paling besar. Digunakan berat semen 752,419 kg.

13. Penyesuaian jumlah air atau f_{as} .

$$f_{as \text{ rencana}} = 0,31$$

$$f_{as \text{ mak}} > f_{as \text{ rencana}}$$

$$0,6 > 0,31 \dots \dots \text{ ok!}$$

14. Perbandingan agregat halus dan kasar

- a) Ukuran maksimum 10 mm.
- b) Nilai *Slump* 60 mm – 180 mm
- c) $f_{as} 0,31$.
- d) Jenis gradasi pasir no. 2.



Diambil proporsi pasir = 47,5%.

15. Berat jeis agregat campuran :

$$= \frac{P}{100} \times B_j \text{ agregat halus} + \frac{K}{100} \times B_j \text{ agregat kasar}$$
$$= 2,658$$

dimana :

P = % agregat halus terhadap agregat campuran

K = % agregat kasar terhadap agregat campuran

16. Berat jenis beton, diperoleh hasil 2340 kg/m^3

17. Berat agregat campuran

$$= \text{berat tiap } \text{m}^3 - \text{keperluan air dan semen}$$
$$= 1354,331 \text{ kg/m}^3$$

18. Menghitung berat agregat halus

$$\begin{aligned} \text{berat agregat halus} &= \% \text{ berat agregat halus} \times \text{keperluan agregat} \\ &\quad \text{campuran} \\ &= 643,307 \text{ kg/m}^3 \end{aligned}$$

19. Menghitung berat agregat kasar

$$\begin{aligned} \text{berat agregat kasar} &= \% \text{ berat agregat kasar} \times \text{keperluan agregat} \\ &\quad \text{campuran} \\ &= 711,024 \text{ kg/m}^3 \end{aligned}$$



Proporsi Campuran Adukan Beton untuk Setiap Variasi Per 1 m³

Kode	Semen (kg)	Pasir (kg)	Split (kg)	Silica Fume (kg)	Serat (kg)	Air (liter)	SP (liter)
BN	752,42	643,31	711,02	0	0	233,25	6,62
BSF-0	752,42	643,31	711,02	0	0,6	233,25	6,62
BSF-5	714,8	643,31	711,02	37,62	0,6	233,25	6,62
BSF-10	677,18	643,31	711,02	75,24	0,6	233,25	6,62
BSF-15	639,56	643,31	711,02	112,86	0,6	233,25	6,62

Proporsi Campuran Adukan Beton untuk Setiap Variasi Per Satu Kali Adukan

Kode	Semen (kg)	Pasir (kg)	Split (kg)	Silica Fume (kg)	Serat (kg)	Air (liter)	SP (liter)
BN	57,78	49,4	54,6	0	0	17,91	0,51
BSF-0	57,78	49,4	54,6	0	0,05	17,91	0,51
BSF-5	54,89	49,4	54,6	2,89	0,05	17,91	0,51
BSF-10	52	49,4	54,6	5,78	0,05	17,91	0,51
BSF-15	49,12	49,4	54,6	8,67	0,05	17,91	0,51



LABORATORIUM TEKNIK SIPIL
PROGRAM STUDI S-1 TEKNIK SIPIL
FAKULTAS TEKNIK JURUSAN TEKNIK SIPIL
UNIVERSITAS MUHAMMADIYAH YOGYAKARTA

Alamat : Jl. Lingkar Selatan, Tamantirto, Kasihan, Bantul, Yogyakarta. Phone (0274) 387656 Fax (0274) 387646

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Pekerjaan : Pengujian karakteristik beton segar *Self-Compacting Concrete*
Dikerjakan oleh : Patria Yudha Asmara
Universitas Atma Jaya Yogyakarta

HASIL PENGUJIAN

Kode	<i>Filling ability</i>	
	<i>Slumpflow (mm)</i>	<i>V-Funnel (detik)</i>
BN	730	6.5
BSF-0	710	7
BSF-5	680	9.5
BSF-10	660	10
BSF-15	635	12

Kode	<i>Passing ability</i>	
	<i>L-Shaped Box (h2/h1)</i>	<i>J-Ring (mm)</i>
BN	1	5
BSF-0	1	5
BSF-5	0.94	0
BSF-10	0.88	0
BSF-15	0.81	0

Kode	<i>Viscosity</i>	
	<i>T500 Slumpflow (detik)</i>	<i>V-Funnel (detik)</i>
BN	2.8	6.5
BSF-0	3.5	7
BSF-5	3.6	9.5
BSF-10	4.3	10
BSF-15	4.8	12



PENGUJIAN KUAT TEKAN SILINDER BETON

Kode	No	Berat	Dimensi		Berat Volume	Beban Maks	Kuat Tekan D10x20	Konversi	Kuat Tekan Terkonversi	Rata - rata
		Kg	D (cm)	T (cm)	Kg/m ³	KN	MPa		MPa	MPa
BN	1	3.893	10.01	20.21	2446.73	170	21.59	0.97	20.95	40.23
	2	3.835	10.1	20.14	2375.74	175	21.83	0.97	21.18	
	3	3.929	9.94	20.32	2490.70	290	37.36	0.97	36.24	
	4	3.825	9.92	20.01	2472.27	360	46.56	0.97	45.16	
	5	3.912	10	19.99	2490.70	350	44.55	0.97	43.21	
	6	3.864	10.02	20.07	2440.56	335	42.47	0.97	41.19	
	7	3.796	10.12	20.11	2345.79	325	40.39	0.97	39.18	
	8	3.917	10	20.12	2477.77	295	37.55	0.97	36.42	

Contoh perhitungan : Kode BN-1

1. Berat Volume

$$\begin{aligned} &= 3.893 / (0.25 \times \pi \times 0.1001^2 \times 0.2021) \\ &= 2446.73 \text{ Kg/m}^3 \end{aligned}$$

2. Kuat tekan D10 x 20

$$\begin{aligned} &= 170*1000 / (0.25 \times \pi \times 100.1^2) \\ &= 21.59 \text{ MPa} \end{aligned}$$

3. Kuat tekan terkonversi

$$\begin{aligned} &= 21.59 \times 0.97 \\ &= 20.95 \text{ MPa} \end{aligned}$$



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Kode	No	Berat	Dimensi		Berat Volume	Beban Maks	Kuat Tekan D10x20	Konversi	Kuat Tekan Terkonversi	Rata - rata
		Kg	D (cm)	T (cm)	Kg/m ³	KN	MPa		MPa	MPa
BSF-0	1	3.811	10.1	19.91	2388.14	275	34.31	0.97	33.28	33.07
	2	3.797	9.88	19.89	2489.01	265	34.55	0.97	33.51	
	3	3.792	10.11	20.03	2357.33	305	37.98	0.97	36.84	
	4	3.839	9.89	19.99	2498.90	255	33.18	0.97	32.19	
	5	3.777	9.95	20.01	2426.55	250	32.14	0.97	31.17	
	6	3.829	10.05	20.1	2400.45	260	32.76	0.97	31.78	
	7	3.806	10	19.98	2424.42	265	33.73	0.97	32.72	
	8	3.785	10.18	20.03	2320.73	190	23.33	0.97	22.63	

Kode	No	Berat	Dimensi		Berat Volume	Beban Maks	Kuat Tekan D10x20	Konversi	Kuat Tekan Terkonversi	Rata - rata
		Kg	D (cm)	T (cm)	Kg/m ³	KN	MPa		MPa	MPa
BSF-5	1	3.77	10.06	20.05	2364.65	190	23.89	0.97	23.18	41.06
	2	3.723	10.03	20.06	2347.99	320	40.48	0.97	39.27	
	3	3.737	9.98	20	2387.63	330	42.17	0.97	40.90	
	4	3.877	9.87	20.29	2496.40	350	45.73	0.97	44.35	
	5	3.841	9.99	19.94	2456.54	330	42.08	0.97	40.82	
	6	3.822	10.04	20.17	2392.50	355	44.82	0.97	43.48	
	7	3.697	10.1	20.16	2287.98	310	38.68	0.97	37.52	
	8	3.888	10.02	19.96	2469.25	180	22.82	0.97	22.13	



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Lampiran 5
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Kode	No	Berat	Dimensi		Berat Volume	Beban Maks	Kuat Tekan D10x20	Konversi	Kuat Tekan Terkonversi	Rata - rata
		Kg	D (cm)	T (cm)	Kg/m ³	KN	MPa		MPa	MPa
BSF-10	1	3.719	9.91	19.99	2411.02	370	47.95	0.97	46.51	50.38
	2	3.81	9.87	19.94	2496.32	430	56.18	0.97	54.49	
	3	3.794	10.12	20.1	2345.72	415	51.57	0.97	50.03	
	4	3.807	10	20.03	2419.01	405	51.55	0.97	50.00	
	5	3.793	10.02	20.07	2395.72	380	48.17	0.97	46.73	
	6	3.785	10.11	20.14	2340.13	420	52.30	0.97	50.73	
	7	3.748	10.1	20.15	2320.69	440	54.90	0.97	53.25	
	8	3.817	9.87	19.96	2498.40	405	52.91	0.97	51.32	

Kode	No	Berat	Dimensi		Berat Volume	Beban Maks	Kuat Tekan D10x20	Konversi	Kuat Tekan Terkonversi	Rata - rata
		Kg	D (cm)	T (cm)	Kg/m ³	KN	MPa		MPa	MPa
BSF-15	1	3.766	10.1	20.01	2348.15	375	46.79	0.97	45.38	44.29
	2	3.761	9.95	19.99	2418.69	345	44.35	0.97	43.02	
	3	3.772	10.05	20.11	2363.54	360	45.36	0.97	44.00	
	4	3.726	10.08	20.2	2310.50	350	43.84	0.97	42.53	
	5	3.687	9.84	19.9	2435.37	350	46.01	0.97	44.63	
	6	3.782	10.02	20	2397.13	380	48.17	0.97	46.73	
	7	3.726	9.93	20.03	2401.04	340	43.88	0.97	42.57	
	8	3.702	10.02	20.04	2341.74	370	46.90	0.97	45.50	



PENGUJIAN KUAT TARIK BELAH

Contoh perhitungan : Kode BN-1

1. Berat Volume

$$\begin{aligned} &= 12.637 / (0.25 \times \pi \times 0.1503^2 \times 0.3015) \\ &= 2361.42 \text{ Kg/m}^3 \end{aligned}$$

2. Kuat tarik belah

$$\begin{aligned} &= 2 \times 235 \times 1000 / (\pi \times 150.3 \times 301.5) \\ &= 3.30 \text{ MPa} \end{aligned}$$

3. Rata - rata

$$\begin{aligned} &= \Sigma \text{ hasil pengujian} / \Sigma \text{ benda uji} \\ &= 3.39 \text{ MPa (Kode BN)} \end{aligned}$$

Kode	No.	Tinggi (mm)	Diameter (mm)	Berat (kg)	B. Jenis (kg/m ³)	Beban (KN)	Kuat Terik Belah (MPa)	Rata-rata (MPa)
BN	1	301.5	150.3	12.637	2361.42	235	3.30	3.39
	2	302.3	150.9	12.767	2360.52	260	3.63	
	3	299.7	149.4	12.653	2407.36	255	3.62	
	4	300.6	150.4	12.682	2373.77	235	3.31	
	5	300.9	150.1	12.71	2386.15	220	3.10	
BSF-0	1	301.2	150.5	12.753	2379.14	275	3.86	3.70
	2	300.5	150.3	12.875	2413.90	260	3.66	
	3	300.7	150	12.818	2411.24	250	3.53	
	4	301.1	149.8	12.897	2429.35	285	4.02	
	5	299.5	149.6	12.8	2430.44	240	3.41	
BSF-5	1	300.7	149.9	12.621	2377.35	240	3.39	3.62
	2	300.5	150	12.463	2346.02	280	3.95	
	3	299.9	149.6	12.463	2363.30	260	3.69	
	4	301.7	150.7	12.702	2359.42	255	3.57	
	5	302.2	150.9	12.477	2307.66	250	3.49	



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Kode	No.	Tinggi (mm)	Diameter (mm)	Berat (kg)	B. Jenis (kg/m3)	Beban (KN)	Kuat Terik Belah (MPa)	Rata-rata (MPa)
BSF-10	1	301.5	150.6	12.42	2311.64	280	3.92	4.10
	2	300.3	150	12.474	2349.65	315	4.45	
	3	299.5	150.1	12.481	2354.11	290	4.11	
	4	299.1	149.5	12.546	2388.59	265	3.77	
	5	300	149.8	12.584	2379.08	300	4.25	
BSF-15	1	300.9	150.6	12.348	2302.82	265	3.72	3.79
	2	301	150.1	12.293	2307.09	230	3.24	
	3	299.7	150	12.285	2318.68	275	3.89	
	4	300.5	149.9	12.299	2318.23	290	4.10	
	5	301.5	150.2	12.376	2315.73	285	4.00	



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Contoh perhitungan : Kode BN-1

1. Beban Maksimum
 $= 900 \text{ kgf} \times 9.81$
 $= 8829 \text{ N}$

PENGUJIAN KUAT LENTUR

2. Kuat lentur

$$\begin{aligned} &= 8829 \times 450 / (100 \times 100^2) \\ &= 3.97 \text{ MPa} \end{aligned}$$

Keterangan	BN				BSF-0				BSF-5			
	1	2	3	4	1	2	3	4	1	2	3	4
Umur Benda Uji (hari)	28	28	28	28	28	28	28	28	28	28	28	28
Berat Benda Uji (kg)	11.902	11.553	12.216	11.993	11.815	12.342	11.908	12.576	11.471	11.494	11.492	11.072
Beban Maksimum (kgf)	900	950	915	895	900	1060	950	895	1000	1025	1250	1300
Beban Maksimum (N)	8829	9319.50	8976.15	8779.95	8829	10398.6	9319.50	8779.95	9810	10055.25	12262.50	12753
Panjang Tampak Melintang = p (mm)	450	450	450	450	450	450	450	450	450	450	450	450
Lebar Tapak melintang $= b$ (mm)	100	100	100	100	100	100	100	100	100	100	100	100
Tinggi tampak Melintang = h (mm)	100	100	100	100	100	100	100	100	100	100	100	100
Kuat Lentur Uji (MPa)	3.97	4.19	4.04	3.95	3.97	4.68	4.19	3.95	4.41	4.52	5.52	5.74
Rata-Rata (MPa)	-	-	-	4.04	-	-	-	4.20	-	-	5.05	-

Keterangan	BSF-10				BSF-15			
	1	2	3	4	1	2	3	4
Umur Benda Uji (hari)	28	28	28	28	28	28	28	28
Berat Benda Uji (kg)	11.95	11.871	11.956	11.777	12.002	12.424	12.352	12.101
Beban Maksimum (kgf)	1450	1300	1400	1390	1200	1180	1500	1565
Beban Maksimum (N)	14224.50	12753.00	13734.00	13635.90	11772.00	11575.80	14715.00	15352.65
Panjang Tampak Melintang = p (mm)	450	450	450	450	450	450	450	450
Lebar Tapak melintang = b (mm)	100	100	100	100	100	100	100	100
Tinggi tampak Melintang = h (mm)	100	100	100	100	100	100	100	100
Kuat Lentur Uji (MPa)	6.40	5.74	6.18	6.14	5.30	5.21	6.62	6.91
Kuat Lentur Rata-Rata (MPa)	6.11				6.01			

SikaFume®

Densified Silica Fume

Description	SikaFume is a new generation concrete additive in a fine powder form based on Silica fume technology. SikaFume is used as a high effective additive for the production of high quality concrete. Complies with ASTM C 1240-00
Use	SikaFume is used to increase the density, durability and compressive strength of concrete.
Advantages	<p>The use of SikaFume improves the performance characteristics of concrete in the following ways :</p> <ul style="list-style-type: none"> ■ Increased workability over a longer period of time. ■ Improved cohesiveness and stability of green concrete ■ Durability greatly increased. ■ Water permeability of concrete greatly reduced ■ Permeability to gases greatly decreased ■ Greatly improved resistance to carbonation ■ Infiltration of chlorides greatly reduced ■ Very high early and ultimate strengths <p>SikaFume contains no chlorides or other potentially corrosive substances. It can therefore be used with complete safety in reinforced and prestressed concrete</p>
Dosage	3% - 10% by weight of cement SikaFume is compatible with most of Sika admixtures. Please consult our Technical Service Division for further information.
Instruction for Use	<p>SikaFume should be dry-mixed with other concrete components before the mixing water is added. After the water is added, further mixing is required to allow the even distribution of ingredients throughout the mixed concrete.</p> <p>For increased effectiveness, it is advisable to incorporate a super plasticizer such as Sikament-range into the concrete mix.</p> <p>It advisable to carry out trial mixes to establish exact dosage rate required.</p>
Technical Data	
Form	Powder
Colour	Greyish
Bulk Density	Approx. 0.60 kg/ ltr
Shelf life	Minimum 3 years if stored properly in its original bag in dry place
Storage	Dry, shaded place
Packaging	20 kg bag



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SikaFibre®

Polypropylene Fibres for Concrete

Description	SikaFibre is high quality micro monofilament polypropylene fibres. It is designed to minimize and control plastic shrinkage cracks in concrete. SikaFibre is available in pre-measured, ready to use degradable bags for 1 m ³ of concrete.
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Use	SikaFibre reinforces fresh concrete and reduce the incidence of shrinkage cracking in pre-hardening stage. SikaFibre is used in : <ul style="list-style-type: none">■ Slabs■ Pavements■ Precast concrete products■ Heavy-duty industrial floors■ Overlays■ Shotcrete■ Mortar screeds and plasters
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Note : Polypropylene fibres are not intended to replace reinforcement steel.

Advantages	Thanks to their fineness and special-surface treatment, SikaFibre is uniformly distributed to provide internal reinforcement to : <ul style="list-style-type: none">■ Reduce plastic shrinkage cracking■ Improve fresh concrete cohesion■ Improve impact and abrasion resistance■ Improve concrete durability
------------	--

Instructions for Use	Put 1 (one) bag of 0.6 kg SikaFibre per m ³ concrete directly into the mixture. A mixing time of 3 to 5 minutes is necessary to ensure that the bag is fully degraded and ensure uniform fibre dispersion throughout the mix. SikaFibre is compatible with all Sika admixtures. The standard procedures for placing, finishing and curing concrete shall be followed. In addition, proper reinforcement and joint spacing should be observed.
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Product Data

Base	Polypropylene fibres with surface agent
Colour	Natural
Specific Gravity	0.91 g/cm ³
Fibre Length	12 mm
Fibre Diameter	18 micron – nominal
Tensile strength	300 – 440 MPa
Elastic Modulus	6000 – 9000 N/mm ²
Water absorption	Nil
Softening Point	160 °C
Shelf Life	3 years if stored in original unopened packaging in cool, dry condition
Packaging	0.6 kg/bag at 40 bags per box

Construction
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Sika® Viscocrete®-1003

Concrete Admixture for High Flow / Self-Compacting Concrete

Description	Sika® Viscocrete®-1003 is a third generation superplasticiser for concrete and mortar. It is particularly developed for the production of high flow concrete with exceptional flow retention properties and significant reduction in bleeding and segregation.
Uses	<p>Sika® Viscocrete®-1003 facilitates extreme water reduction, excellent flowability with optimal cohesion and strong self-compacting behaviour.</p> <p>Sika® Viscocrete®-1003 is used for the following types of concrete:</p> <ul style="list-style-type: none">■ High flow concrete■ Self-compacting concrete (S.C.C.)■ Concrete with very high water reduction (up to 30 %)■ High strength concrete■ Concrete in hot weather and with extended transportation and workability requirements etc. <p>The combination of high water reduction, excellent flowability and high early strength provides clear benefits in the above mentioned applications.</p>
Advantages	<p>Sika® Viscocrete®-1003 acts by surface adsorption on the cement particles producing a sterical separation effect. Concrete produced with Sika® Viscocrete®-1003 exhibits the following properties:</p> <ul style="list-style-type: none">■ Excellent flowability (resulting in highly reduced placing and compacting efforts)■ Strong self-compacting behaviour■ Extremely high water reduction (resulting in high density and strengths)■ Improved shrinkage and creep behaviour■ Increased carbonation resistance of the concrete■ Improved finish■ Reduce tendency to bleeding and segregation <p>Sika® Viscocrete®-1003 does not contain chlorides or other ingredients which promotes steel corrosion. Therefore, it may be used without restriction for reinforced and pre-stressed concrete construction.</p> <p>Sika® Viscocrete®-1003 gives the concrete extended workability and depending on the mix design and the quality of materials used, self-compacting properties can be maintained for more than 1 hour at 30°C.</p>



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Product Data		Lampiran 8
Type	Aqueous solution of modified polycarboxylate copolymers	Halaman 117
Appearance	Brownish	
Specific Gravity	1.065 ± 0.01 kg/ltr	
Shelf Life & Storage	12 months from the date of production when stored in original unopened packaging in a cool, dry place	
Packaging	200 ltr drums and bulk deliveries	
Instruction for Use		
Dosage	<ul style="list-style-type: none"> ■ For soft plastic concrete ■ For flowing and self compacting concrete (S.C.C.) 	0.2 - 0.6% by weight of binder 0.6 - 1.6% by weight of binder
Dispensing	<p>Sika® Viscocrete®-1003 is added to the gauging water or simultaneously poured with it into the concrete mixer. For optimum utilisation of its high water reduction property, we recommend thorough mixing at a minimal wet mixing time of 60 seconds.</p> <p>The addition of the remaining gauging water (to fine tune concrete consistency) may only be started after two-thirds of the wet mixing time, to avoid surplus water in the concrete.</p>	
Concrete Placing	<p>With the use of Sika® Viscocrete®-1003, concrete of the highest quality is produced. The standard rules of good concreting practice (production as well as placing) must also be observed with Sika® Viscocrete®-1003 concrete.</p> <p>Fresh concrete must be cured properly.</p>	
Combinations	<p>Sika® Viscocrete®-1003 may be combined with the following products:</p> <ul style="list-style-type: none"> ■ Plastiment VZ ■ SikaFume ■ SikaAER ■ Sika Control <p>Pre-trials are recommended if combinations with the above products are required. Please consult our Technical Service Department.</p> <p>To produce flowing and/or self-compacting concrete, special concrete mix design is required. Pre-trials are mandatory. Please consult our Technical Service Department.</p>	
Safety Precautions	<p>Wear gloves and goggles during application. If in contact with skin, wash thoroughly with soap and water. If in contact with eyes or mucous membrane, flush immediately with plenty of water and seek medical attention without delay. Use with adequate ventilation.</p> <p>For more information, refer to our Material Safety Data Sheet (available upon request).</p>	
Legal Notes	<p>The information, and, in particular, the recommendations relating to the application and end-use of Sika products, are given in good faith based on Sika's current knowledge and experience of the product when properly stored, handled and applied under normal conditions in accordance with Sika's recommendations. In practice, the differences in materials, substrates and actual site conditions are such that no warranty in respect of merchantability or of fitness for a particular purpose, nor any liability arising out of any legal relationship whatsoever, can be inferred either from this information, or from any written recommendations, or from any other advice offered. The user of the product must test the product's suitability for the intended application and purpose. Sika reserves the right to change the properties of its products. The proprietary rights of third parties must be observed. All orders are accepted subject to our current terms of sale and delivery. Users must always refer to the most recent issue of the local Product Data Sheet for the product concerned, copies of which will be supplied on request.</p>	

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DOKUMENTASI PENELITIAN



Pengujian Agregat Kasar



Pengujian Agregat Halus



Pencampuran bahan dengan molen



Penuangan mortar ke dalam bekisting



Pengujian *Slump flow*



Pengujian *V-Funnel*



Pengujian *J-Ring*



Pengujian *L-Shaped Box*



Benda uji yang digunakan



Penimbangan benda uji



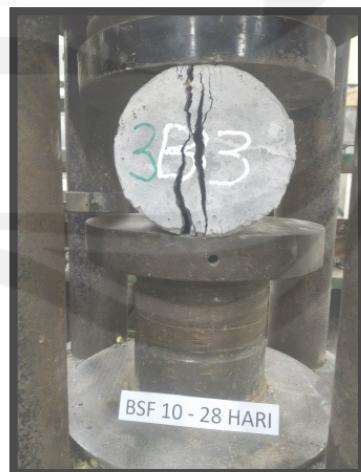
Pengukuran dimensi



Pengujian kuat lentur



Pengujian kuat tekan



Pengujian kuat tarik belah