

BAB VI

KESIMPULAN & SARAN

6.1. Kesimpulan

Berdasarkan hasil penelitian dan pembahasan maka diperoleh beberapa kesimpulan sebagai berikut :

1. Beban maksimum yang dapat diterima benda uji PLBR-01, PLBR-02, PLBRB-01, PLBRB-02, PLC-01 dan PLC-02 berturut-turut adalah 33,45 kN; 35,55 kN; 33,13 kN; 33,56 kN; 14,03 kN dan 14,38 kN.
2. Beban retak pertama yang terjadi pada PLBR-01, PLBR-02, PLBRB-01, PLBRB-02, PLC-01 dan PLC-02 berturut-turut adalah 8,79 kN; 9,38 kN; 5,89 kN; 5,74 kN; 7,33 kN dan 5,60 kN.
3. Kekakuan pelat pada benda uji PLBR-01, PLBR-02, PLBRB-01, PLBRB-02, PLC-01 dan PLC-02 berturut-turut adalah 2,59 kN/mm; 2,52 kN/mm; 2,49 kN/mm; 2,42 kN/mm; 1,87 kN/mm dan 1,80 kN/mm.
4. Kuat tekan silinder beton rata-rata SBR 1, SBR 2, SBR 3 dan SBR 4 berturut-turut adalah 10,85 MPa; 11,47 MPa; 11,51 MPa dan 11,60 MPa. Sedangkan untuk kuat tekan kubus KBR 1 dan KBR 2 hasil konversi ke bentuk silinder berturut-turut adalah 5,82 MPa dan 4,26 MPa.
5. Modulus elastisitas silinder beton ringan rata-rata sebesar 16607,37 MPa.
6. Berat jenis silinder beton rata-rata SBR 1, SBR 2, SBR 3 dan SBR 4 berturut-turut adalah 1843 kg/m³, 1823 kg/m³, 1834 kg/m³ dan 1840 kg/m³. Sedangkan untuk berat jenis kubus beton KBR 1 dan KBR 2 berturut-turut adalah 824 kg/m³ dan 832 kg/m³.

7. Berdasarkan perbandingan hasil pengujian lentur beban maksimum rata-rata yang dapat diterima PLBR, PLBRB dan PLC berturut-turut adalah 34,50 kN; 33,35 kN dan 14,21 kN dengan persentase PLBRB sebesar 3,33% terhadap PLBR dan persentase PLC sebesar 58,81% terhadap PLBR.
8. Benda uji panel lantai PLBR dan PLBRB memiliki kuat lentur dan kuat tekan yang lebih besar dibandingkan dari Panel lantai Citicon namun untuk beratnya sendiri masih lebih unggul Panel Lantai Citicon yang 2 kali lebih ringan dibandingkan panel lantai PLBR dan PLBRB.

6.2. Saran

Berdasarkan penelitian yang telah dilakukan, hal – hal yang perlu diperhatikan untuk penelitian selanjutnya adalah sebagai berikut :

1. Disarankan untuk menggunakan bahan campuran beton ringan yang lain dengan harapan agar mendapatkan berat jenis beton yang lebih ringan lagi. Contohnya beton berpori atau beton ACC.
2. Jika ingin menggunakan material agregat kasar ringan perlu diperhatikan ukuran gradasi agregat. Karena proses pemecahan yang cukup sulit dan lama sehingga ukuran gradasi agregat tidak seluruhnya seragam.
3. Perlu diperhatikan bahan tambah yang digunakan terhadap susunan campuran beton dengan agregat kasar ringan karena dapat mempengaruhi ikatan antara agregat yang mengakibatkan terjadinya segregasi.
4. Menggunakan nilai fraksi yang berbeda.

5. Untuk penelitian selanjutnya disarankan untuk menggunakan pelat 2 arah dengan pembebanan yang dilakukan berupa beban merata.
6. Untuk pembuatan rongga pada panel disarankan menggunakan bahan dan metode yang lain.

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LAMPIRAN



LAMPIRAN I

PENGUJIAN GRADASI BESAR BUTIRAN PASIR

Bahan : Pasir

Asal : Kali Progo

Diperiksa : 3 Mei 2017

HASIL PEMERIKSAAN

No. Saringan	Berat Tertahan	Kumulatif	% Tertahan	% Lolos
3/8 (9.52 mm)	0,000	0,000	0,000	100,000
4 (4.75 mm)	0,000	0,000	0,000	100,000
8 (2.36 mm)	0,000	0,000	0,000	100,000
30 (0.6 mm)	24,860	24,860	2,486	97,514
50 (0.3 mm)	604,680	629,540	62,954	37,046
100 (0.15 mm)	326,180	955,720	95,572	4,428
Pan	44,280	1000,000	100,000	0,000
Total	1000,000		261,012	

$$\text{Modulus Halus Butir} = \frac{261,012}{100} = 2,610$$

Kesimpulan : MHB Pasir $1,5 \leq 2,61 \leq 3,8$(OK)



PEMERIKSAAN BERAT JENIS DAN PENYERAPAN PASIR

Bahan : Pasir

Asal : Kali Progo

Diperiksa : 3 Mei 2017

HASIL PEMERIKSAAN

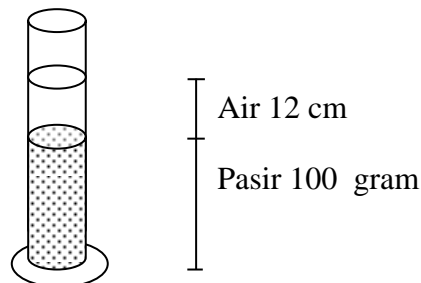
Nomor Pemeriksaan	I
Berat Awal (V)	500,08 gram
Berat Kering Oven (A)	499,07 gram
Jumlah Air Total yang Digunakan (W)	310 gram
Berat Jenis Bulk = $\frac{A}{V - W}$	2,6256 gr/cm ³
Berat Jenis SSD = $\frac{V}{V - W}$	2,6309 gr/cm ³
Berat Jenis Semu (<i>Apparent</i>) = $\frac{A}{(V - W) - (V - A)}$	2,6396 gr/cm ³
Penyerapan (<i>Absorption</i>) = $\frac{V - A}{A} \times 100\%$	0,2024 %



PEMERIKSAAN KADAR LUMPUR DALAM PASIR

- I. Waktu Pemeriksaan : 3 Mei 2017
- II. Bahan
 - a. Pasir kering tungku, Asal : Kali Progo, Berat : 100,15 gram
 - b. Air jernih asal : LSBB Prodi TS FT-UAJY
- III. Alat
 - a. Gelas ukur, ukuran: 250 cc
 - b. Timbangan
 - c. Tungku (*oven*), suhu dibuat antara 105 - 110°C
 - d. Pengocokan dilakukan sebanyak 8 kali
 - e. Pasir+piring masuk tungku tanggal 3 Mei jam 11.56 WIB

IV. Sketsa



V. Hasil

Setelah pasir keluar tungku tanggal 4 Mei jam 12.00 WIB

- a. Berat piring+pasir = 224,2 gram
- b. Berat piring kosong = 126,57 gram
- c. Berat pasir = 98,38 gram

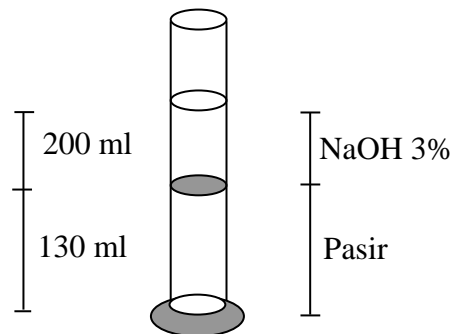
$$\text{Kandungan Lumpur} = \frac{100,15 - 98,38}{100,15} \times 100\% = 1,77 \%$$



PEMERIKSAAN KANDUNGAN ZAT ORGANIK DALAM PASIR

- I. Waktu Pemeriksaan : 3 Mei 2017
- II. Bahan
 - a. Pasir kering tungku, Asal: Kali Progo, Volume: 130 ml
 - b. Larutan NaOH 3%
- III. Alat

Gelas ukur, ukuran: 250cc
- IV. Sketsa



- V. Hasil

Setelah didiamkan selama 24 jam, warna larutan di atas pasir sesuai dengan warna *Gardner Standard Color* No. 8.



PEMERIKSAAN BERAT JENIS DAN PENYERAPAN AGREGAT KASAR

Bahan : Breksi Batu Apung

Asal : Desa Bawuran

Diperiksa : 4 Mei 2017

HASIL PEMERIKSAAN

Nomor Pemeriksaan	I
Berat Kering Oven (A)	1420 gram
Berat. Jenuh SSD (B)	2120 gram
Berat. Dalam Air (C)	640 gram
Berat Jenis <i>Bulk</i> = $\frac{A}{B - C}$	0,96 gr/cm ³
Berat Jenis SSD = $\frac{B}{B - C}$	1,43 gr/cm ³
Berat Jenis Semu = $\frac{A}{A - C}$	1,82 gr/cm ³
Penyerapan Air = $\frac{B - A}{A} \times 100\%$	49,30 %



PEMERIKSAAN KEAUSAN AGREGAT KASAR DENGAN MESIN LAA

Bahan : Breksi Batu Apung

Asal : Desa Bawuran

Diperiksa : 21 Agustus 2017

HASIL PEMERIKSAAN

Gradasi Saringan		Nomor Pemeriksaan
		I
Lolos	Tertahan	Berat Masing-masing Agregat
$\frac{3}{4}$ "	$\frac{1}{2}$ "	2500 gram
$\frac{1}{2}$ "	$\frac{3}{8}$ "	2500 gram

Nomor Pemeriksaan	I
Berat awal (A)	5000 gram
Berat sesudah diayak saringan No.12 (B)	1748 gram
Keausan Agregat = $\frac{(A)-(B)}{(A)} \times 100\%$	65,04 %



LAMPIRAN II

PENGUJIAN KUAT TARIK BAJA TULANGAN

Bahan : Baja Tulangan P8

Kode : US – 8 – SNI – TP24

Diperiksa : 12 Mei 2017

HASIL PEMERIKSAAN

Nomor Pemeriksaan	I
Diameter Tulangan	7,8 mm
Luas	47,78 mm ²
Beban Maksimum	2370 kgf
Tegangan Leleh (<i>f_y</i>)	348,90 MPa
Tegangan Maksimum (<i>f_u</i>)	486,41 MPa



LAMPIRAN III

RENCANA CAMPURAN BETON RINGAN SK SNI 03-3449-2002

A. Spesifikasi bahan:

1. Pasir yang berasal dari Kali Progo, Sleman, Yogyakarta.
2. *Pumice* yang digunakan memiliki ukuran agregat maksimal 20 mm.
3. Semen yang digunakan Merk Holcim.
4. Bahan tambah yang digunakan berupa *Ligno P-100*

B. Data Specific Gravity

1. *Specific gravity* agregat halus (pasir) : 2,63 gr/cm³.
2. *Specific gravity* agregat kasar (*Pumice*) : 1,43 gr/cm³.
3. *Absorption* agregat halus (pasir) : 0,20 %
4. *Absorption* agregat kasar (*Pumice*) : 49,30 %

C. Perhitungan :

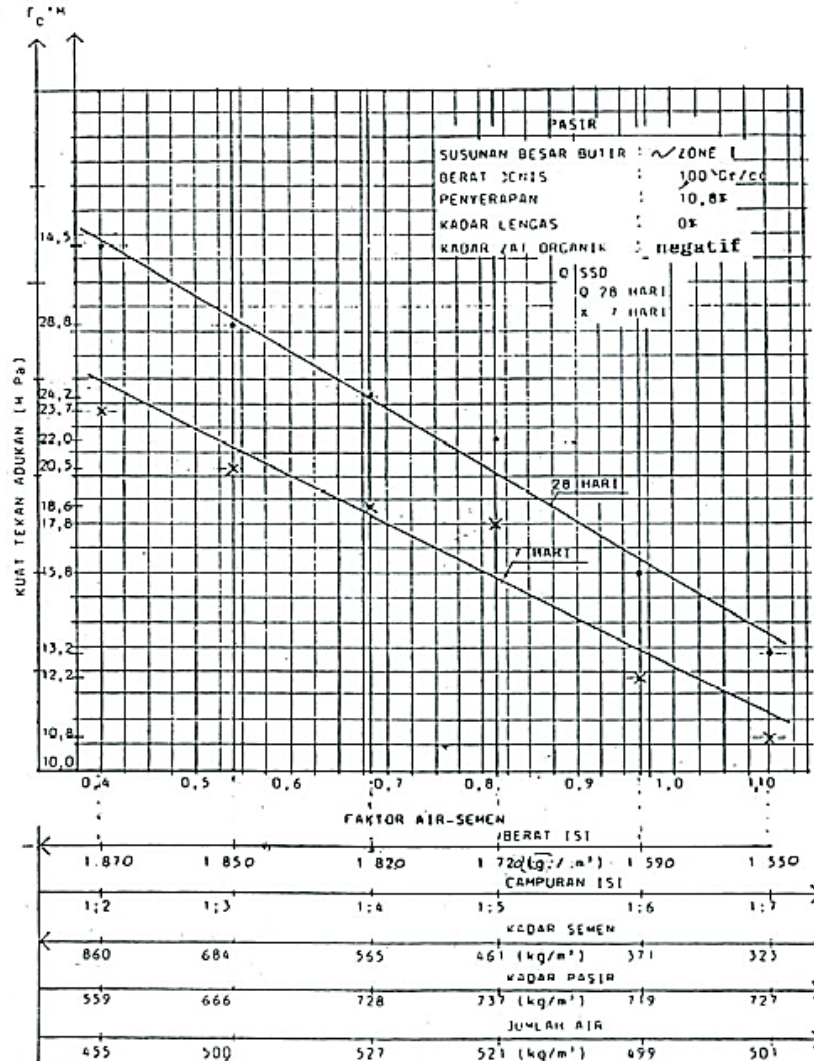
1. Kuat tekan yang disyaratkan (f'_c) pada umur 28 hari. $f'_c = 25$ MPa.
2. Menentukan nilai deviasi standar berdasarkan tingkat mutu pengendalian pelaksanaan campuran.
3. Kuat tekan yang harus di tambahkan.
(margin) = $k \times s = 1,64 \times 4,5 = 7,38$ MPa.
4. Kuat tekan rata-rata yang ditargetkan, $F_c, Br (F_c', B+ M) = 33$ MPa.
5. Jenis semen disyaratkan : Semen Portland Merk *Holcim*.
6. Jenis Agregat, disyaratkan untuk :
 - Agregat kasar : Batu apung (*Pumice*).
 - Agregat halus : Pasir alam.



7. Kuat hancur Agregat kasar (f_c, A) diketahui atau dari gambar = 5 MPa

Gambar Grafik Hubungan antara Kuat Tekan Adukan yang Memakai

Agregat Batu Apung Susunan Campuran Beton



8. Berat jenis agregat, diketahui untuk :

- Agregat kasar, $P_A = 1,43$

- Agregat halus, $P_S = 2,63$

9. Bobot maksimum beton (BIB), Disyaratkan = 1900 kg/m^3

10. Jumlah fraksi agregat kasar, $n_f, 0,35$



MIX DESIGN BETON RINGAN

Kuat Tekan Rencana	25 MPa	
Deviasi Standar	4,5 MPa	(perkiraan)
Berat Isi Beton Maksimum Rencana	1800 kg/m ³	
Nilai Tambah (M)	7,38	
Kuat Tekan Rata2	33	
Nilai fraksi	0,35	
Berat jenis agregat ringan	1,43	
Berat Isi Beton Yang disyaratkan (BIB)		
Minimal	1700	
Maksimal	1900	
Jenis Semen	Portland	
Jenis Agregat		
Berdasarkan Konstruksi	Struktural Ringan	
Agregat Kasar	Breksi Batu Apung	
Agregat Halus	Pasir Alami	
Kuat Hancur Agregat	5 MPa	
Kuat tekan adukan fc M	33	
FAS rencana	0,4 maks	(grafik 4)
Bandingkan dengan 1/15 fc M	2,2	kuat hancur agregat lebih besar maka aman
Bandingkan dengan 1/2 fc M	16,5	kuat hancur agregat lebih kecil maka aman
kuat tekan dipilih	33 MPa	
Berat isi dipilih	1870 kg/m ³	
Campuran Isi	1 : 2	
Berat Semen	860 kg/m ³	
Berat Pasir	559 kg/m ³	dari grafik
Berat Air	455 kg/m ³	
Total	1.874 kg/m ³	



KEBUTUHAN CAMPURAN UNTUK 1 PELAT DAN 2 SILINDER

Pelat Solid

Susunan Campuran Beton	Berat	Volume	SF	Kebutuhan
Agregat Kasar <i>Pumice</i>	500,5 kg/m ³	0,0925 m ³	1,2	55,556 kg
Semen	559 kg/m ³			62,049 kg
Pasir Alam	363,35 kg/m ³			40,332 kg
Berat Air	295,75 kg/m ³			32,828 kg
Ligno 0,6%				372,294 ml

Pelat Berongga

Susunan Campuran Beton	Berat	Volume	SF	Kebutuhan
Agregat Kasar <i>Pumice</i>	500,5 kg/m ³	0,0862 m ³	1,2	51,772 kg
Semen	559 kg/m ³			57,823 kg
Pasir Alam	363,35 kg/m ³			37,585 kg
Berat Air	295,75 kg/m ³			30,592 kg
Ligno 0,6%				346,938 ml



LAMPIRAN IV

HASIL PENGUJIAN KUAT TEKAN DAN BERAT JENIS SILINDER BETON RINGAN

KODE BENDA UJI SILINDER	DIMENSI (cm)		BERAT (kg)	TANGGAL PEMBUATAN	FAS	TANGGAL PENGUJIAN 28 HARI	BERAT JENIS (kg/m ³)	RATA - RATA	BEBAN (kN)	KUAT TEKAN (MPa)	RATA - RATA
	D	T									
SBR 1A	15,09	30,20	9,84	09/06/17	0,16	07/07/17	1821	1843	190	10,62	10,85
SBR 1B	14,96	29,93	9,81	09/06/17	0,16	07/07/17	1864		195	11,09	
SBR 2A	14,95	30,16	9,72	14/06/17	0,2	14/07/17	1835	1823	190	10,82	11,47
SBR 2B	15,03	29,99	9,64	14/06/17	0,2	14/07/17	1811		215	12,11	
SBR 3A	15,01	30,06	9,62	13/06/17	0,16	11/07/17	1808	1834	210	11,86	11,51
SBR 3B	15,11	30,14	10,06	13/06/17	0,16	11/07/17	1861		200	11,15	
SBR 4A	15,01	30,26	9,82	20/06/17	0,19	21/07/17	1833	1840	210	11,86	11,60
SBR 4B	14,98	30,11	9,80	20/06/17	0,19	21/07/17	1846		200	11,34	

SBR = Silinder Beton Ringan

SBR 1 & 2 untuk benda uji panel lantai solid sedangkan SBR 3 & 4 untuk benda uji panel lantai berongga



KODE BENDA UJI	DIMENSI (cm)	BERAT (kg)	TANGGAL PENGUJIAN	BERAT JENIS (kg/m ³)	RATA - RATA	BEBAN (kg)	KUAT TEKAN KUBUS (MPa)	RATA - RATA	KUAT TEKAN SILINDER*) (MPa)	RATA - RATA
	KUBUS									
KBR 1	10 x 10	0,824	28/08/17	824	828	7410	7,27	6,29	5,82	5,04
KBR 2	10 x 10	0,832	28/08/17	832		5420	5,32		4,26	

KBR = Kubus Beton Ringan

***) Nilai konversi kuat tekan kubus 100 x 100 mm ke silinder 150 x 300 mm = 0,8 (ASTM, 1986)**



Dokumentasi Hasil Pengujian Kubus Beton Ringan



HASIL PENGUJIAN MODULUS ELASTISITAS SILINDER BETON

Modulus Elastisitas SBR 5A

Tanggal Pembuatan : 09 Juni 2017

Tanggal Pengujian : 07 Juli 2017

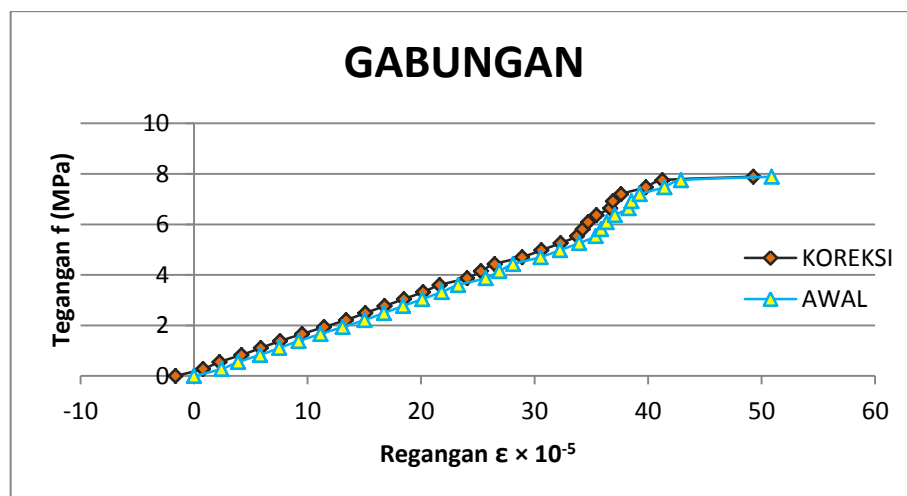
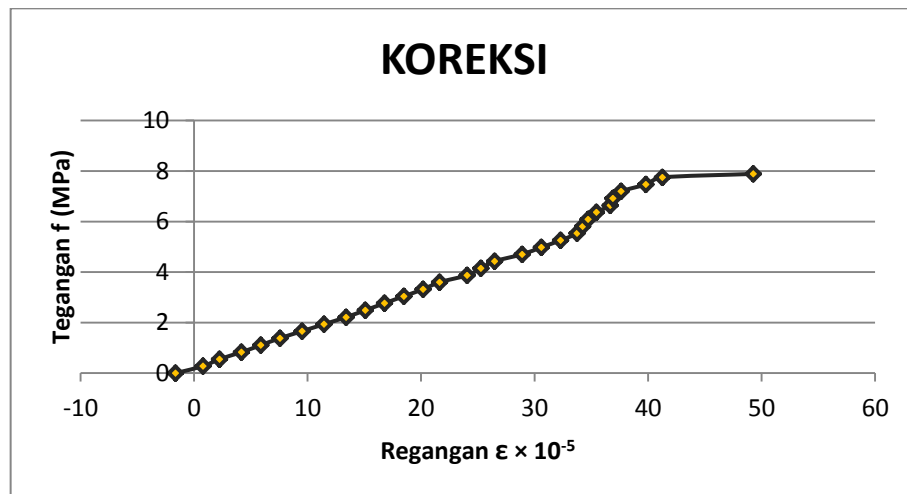
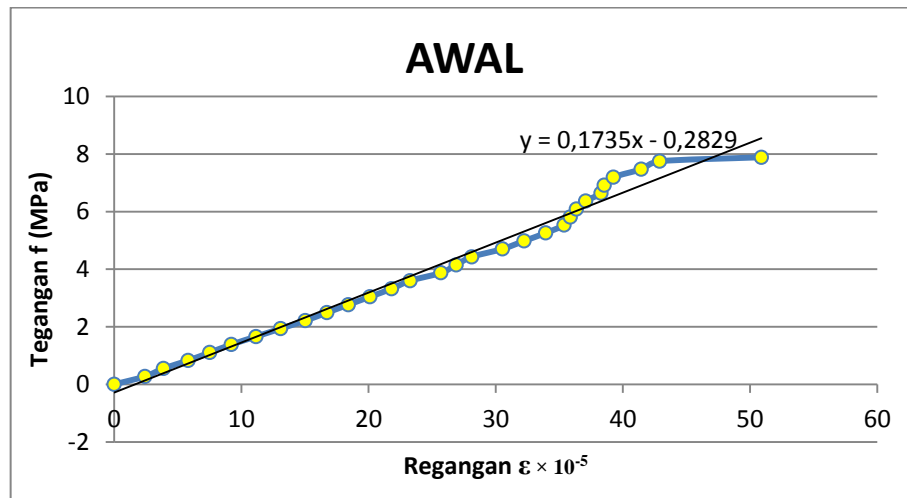
Diameter Silinder : 150,2 mm

Luas Permukaan : 17718,61 mm²

Po : 20,63 cm

Modulus Elastisitas : 18766,88 MPa

Beban		Pembacaan Strainometer	Pembacaan Strainometer / 2	Tegangan	Regangan	Regangan Koreksi
Kgf	N	10 ⁻³	10 ⁻³	Mpa	10 ⁻⁵	10 ⁻⁵
0	0	0	0	0	0	-1,63055
500	4903,355	10	5	0,276735	2,423655	0,79311
1000	9806,71	16	8	0,553469	3,877848	2,24730
1500	14710,07	24	12	0,830204	5,816772	4,18622
2000	19613,42	31	15,5	1,106939	7,51333	5,88278
2500	24516,78	38	19	1,383673	9,209889	7,57934
3000	29420,13	46	23	1,660408	11,14881	9,51826
3500	34323,49	54	27	1,937143	13,08774	11,45719
4000	39226,84	62	31	2,213877	15,02666	13,39611
4500	44130,2	69	34,5	2,490612	16,72322	15,09267
5000	49033,55	76	38	2,767347	18,41978	16,78923
5500	53936,91	83	41,5	3,044081	20,11634	18,48579
6000	58840,26	90	45	3,320816	21,81289	20,18235
6500	63743,62	96	48	3,597551	23,26709	21,63654
7000	68646,97	106	53	3,874286	25,69074	24,06019
7500	73550,33	111	55,5	4,15102	26,90257	25,27202
8000	78453,68	116	58	4,427755	28,1144	26,48385
8500	83357,04	126	63	4,70449	30,53805	28,90750
9000	88260,39	133	66,5	4,981224	32,23461	30,60406
9500	93163,75	140	70	5,257959	33,93117	32,30062
10000	98067,1	146	73	5,534694	35,38536	33,75481
10500	102970,5	148	74	5,811428	35,87009	34,23954
11000	107873,8	150	75	6,088163	36,35482	34,72428
11500	112777,2	153	76,5	6,364898	37,08192	35,45137
12000	117680,5	158	79	6,641632	38,29375	36,66320
12500	122583,9	159	79,5	6,918367	38,53611	36,90556
13000	127487,2	162	81	7,195102	39,26321	37,63266
13500	132390,6	171	85,5	7,471836	41,4445	39,81395





Modulus Elastisitas SBR 5B

Tanggal Pembuatan : 09 Juni 2017

Tanggal Pengujian : 07 Juli 2017

Diameter Silinder : 149,6 mm

Luas Permukaan : 17577,34 mm²

Po : 20,23 cm

Modulus Elastisitas : 14447,86 MPa

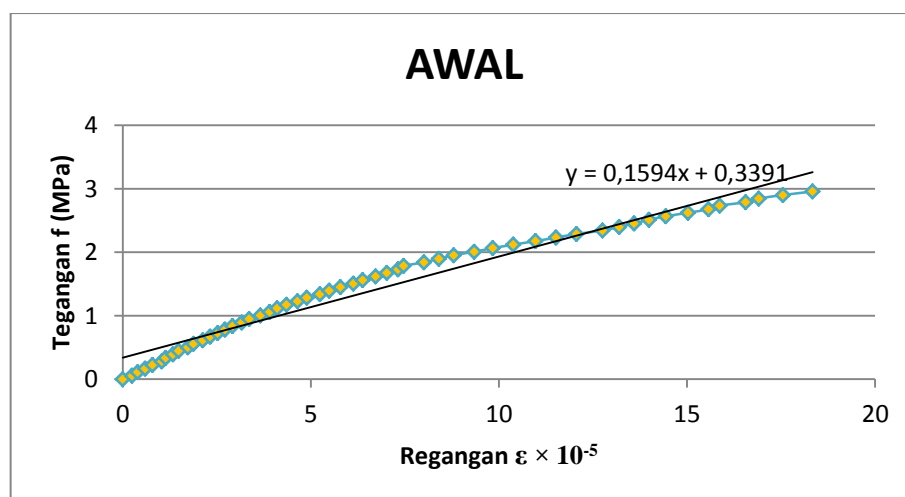
Beban		Pembacaan Strainometer	Pembacaan Strainometer / 10	Tegangan	Regangan	Regangan Koreksi
Kgf	KN	10 ⁻³	10 ⁻³	Mpa	10 ⁻⁵	10 ⁻⁵
0	0	0	0	0	0	2,127353
100	980,671	5	0,5	0,055792	0,24716	2,37451
200	1961,342	8	0,8	0,111584	0,39545	2,522805
300	2942,013	12	1,2	0,167375	0,59318	2,720531
400	3922,684	16	1,6	0,223167	0,79090	2,918257
500	4903,355	21	2,1	0,278959	1,03806	3,165415
600	5884,026	23	2,3	0,334751	1,13693	3,264278
700	6864,697	27	2,7	0,390543	1,33465	3,462004
800	7845,368	30	3	0,446334	1,48295	3,610299
900	8826,039	35	3,5	0,502126	1,73010	3,857456
1000	9806,71	38	3,8	0,557918	1,87840	4,005751
1100	10787,38	43	4,3	0,613710	2,12556	4,252909
1200	11768,05	47	4,7	0,669501	2,32328	4,450635
1300	12748,72	51	5,1	0,725293	2,52101	4,648361
1400	13729,39	55	5,5	0,781085	2,71873	4,846087
1500	14710,07	59	5,9	0,836877	2,91646	5,043813
1600	15690,74	64	6,4	0,892669	3,16362	5,290971
1700	16671,41	68	6,8	0,948460	3,36134	5,488697
1800	17652,08	74	7,4	1,004252	3,65793	5,785286
1900	18632,75	79	7,9	1,060044	3,90509	6,032444
2000	19613,42	83	8,3	1,115836	4,10282	6,23017
2100	20594,09	88	8,8	1,171628	4,34998	6,477328
2200	21574,76	94	9,4	1,227419	4,64656	6,773917
2300	22555,43	99	9,9	1,283211	4,89372	7,021075
2400	23536,1	106	10,6	1,339003	5,23974	7,367096
2500	24516,78	111	11,1	1,394795	5,48690	7,614253
2600	25497,45	117	11,7	1,450586	5,78349	7,910842
2700	26478,12	124	12,4	1,506378	6,12951	8,256863
2800	27458,79	129	12,9	1,562170	6,37667	8,504021
2900	28439,46	136	13,6	1,617962	6,72269	8,850042
3000	29420,13	142	14,2	1,673754	7,01928	9,146631
3100	30400,8	148	14,8	1,729545	7,31587	9,44322

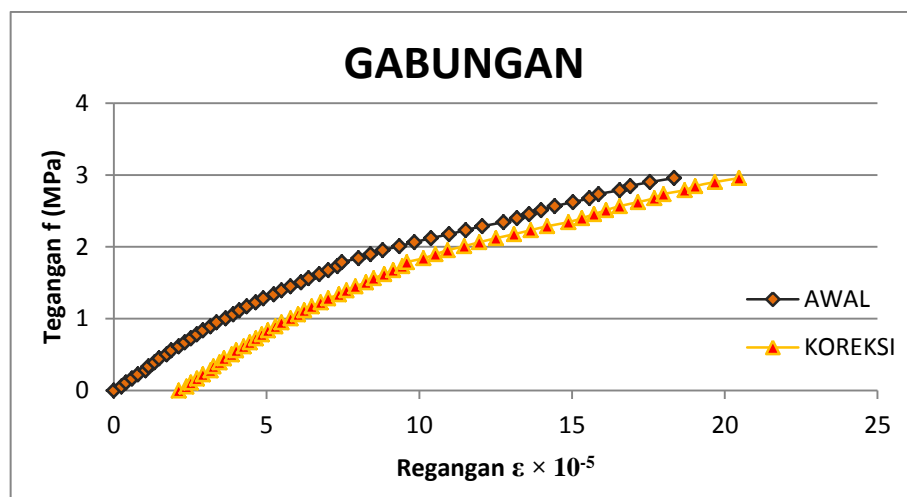
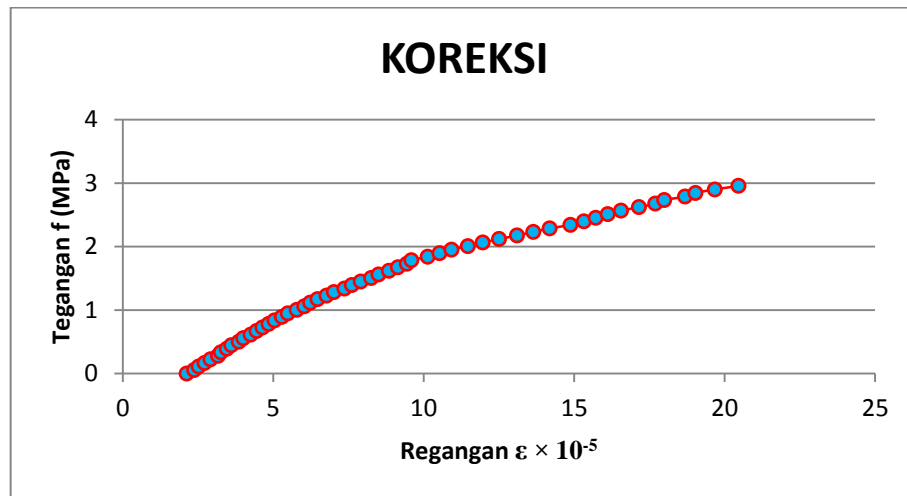


UNIVERSITAS ATMA JAYA YOGYAKARTA
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3200	31381,47	151	15,1	1,785337	7,46416	9,591515
3300	32362,14	162	16,2	1,841129	8,00791	10,13526
3400	33342,81	170	17	1,896921	8,40336	10,53071
3500	34323,49	178	17,8	1,952713	8,79881	10,92617
3600	35304,16	189	18,9	2,008504	9,34256	11,46991
3700	36284,83	199	19,9	2,064296	9,83688	11,96423
3800	37265,5	210	21	2,120088	10,38062	12,50798
3900	38246,17	222	22,2	2,175880	10,97380	13,10115
4000	39226,84	233	23,3	2,231671	11,51755	13,6449
4100	40207,51	244	24,4	2,287463	12,06130	14,18865
4200	41188,18	258	25,8	2,343255	12,75334	14,88069
4300	42168,85	267	26,7	2,399047	13,19822	15,32557
4400	43149,52	275	27,5	2,454839	13,59367	15,72103
4500	44130,2	283	28,3	2,510630	13,98913	16,11648
4600	45110,87	292	29,2	2,566422	14,43401	16,56136
4700	46091,54	304	30,4	2,622214	15,02719	17,15454
4800	47072,21	315	31,5	2,678006	15,57093	17,69829
4900	48052,88	321	32,1	2,733798	15,86752	17,99488
5000	49033,55	335	33,5	2,789589	16,55957	18,68692
5100	50014,22	342	34,2	2,845381	16,90559	19,03294
5200	50994,89	355	35,5	2,901173	17,54820	19,67555
5300	51975,56	371	37,1	2,956965	18,33910	20,46645







LAMPIRAN V

PERHITUNGAN ANALISIS BEBAN MAKSIMUM, BEBAN RETAK PERTAMA, DEFLEKSI DAN KEKAKUAN PELAT

A. Perhitungan Beban Maksimum Yang Terjadi

1. Perhitungan Beban PLBR-01

a. Dimensi PLBR-01

- 1). Panjang : 1350 mm
- 2). Lebar : 450 mm
- 3). Tebal : 125 mm
- 4). Selimut Beton : 20 mm
- 5). f'_c : 10,85 MPa

b. Dimensi Baja Tulangan P8

- 1). Diameter : 7,8 mm
- 2). f_y : 348,904 MPa
- 3). f_u : 486,413 MPa

c. Perhitungan

- 1). Menentukan luas tulangan dalam keadaan seimbang

$$\rho_b = \frac{0,85 \cdot f'_c \cdot \beta_1}{f_y} \times \frac{600}{600 + f_y}$$

$$\rho_b = \frac{0,85 \times 10,85 \times 0,85}{348,904} \times \frac{600}{600 + 348,904}$$

$$\rho_b = 0,0142$$

$$\rho_{max} = 0,0403$$

$$A_{sb} = 0,0142 \times 450 \times 101 = 645,691$$



$$2). A_s = 5 \times \left(\frac{1}{4} \pi d^2 \right)$$

$$A_s = 5 \times \left(\frac{1}{4} \pi \cdot 7,8^2 \right)$$

$$A_s = 251,429 \text{ mm}^2$$

$$\text{Keadaan } \textit{underreinforced}, A_s = 251,429 \leq A_{sb} = 645,691$$

3). Kapasitas momen pelat

$$a = \frac{A_s \cdot f_y}{0,85 \cdot f_c \cdot b}$$

$$a = \frac{251,429 \times 348,904}{0,85 \times 10,85 \times 450}$$

$$a = 21,14 \text{ mm}$$

$$M_n = A_s \cdot f_y (d - 0,5 \cdot a)$$

$$M_n = 251,429 \times 348,904 \times (101 - 0,5 \times 21,14)$$

$$M_n = 7,930 \text{ kN.m}$$

$$M_u = \phi \cdot M_n$$

$$M_u = \phi \cdot 7,930 = 7,137 \text{ kN.m}$$

4). Beban Maksimum Pelat

$$M = \frac{PL}{6}$$

$$P = \frac{7,137 \times 6}{1,35}$$

$$P = 31,72 \text{ kN} = 3235,12 \text{ kg}$$



2. Perhitungan Beban PLBR-02

a. Dimensi PLBR-02

- 1). Panjang : 1350 mm
- 2). Lebar : 450 mm
- 3). Tebal : 125 mm
- 4). Selimut Beton : 20 mm
- 5). f'_c : 11,47 MPa

b. Dimensi Baja Tulangan P8

- 1). Diameter : 7,8 mm
- 2). f_y : 348,904 MPa
- 3). f_u : 486,413 MPa

c. Perhitungan

- 1). Menentukan luas tulangan dalam keadaan seimbang

$$\rho_b = \frac{0,85 \cdot f'_c \cdot \beta_1}{f_y} \times \frac{600}{600 + f_y}$$

$$\rho_b = \frac{0,85 \times 11,47 \times 0,85}{348,904} \times \frac{600}{600 + 348,904}$$

$$\rho_b = 0,0150$$

$$\rho_{max} = 0,0403$$

$$A_{sb} = 0,0150 \times 450 \times 101 = 682,587$$



$$2). A_s = 5 \times \left(\frac{1}{4} \pi d^2 \right)$$

$$A_s = 5 \times \left(\frac{1}{4} \pi \cdot 7,8^2 \right)$$

$$A_s = 251,429 \text{ mm}^2$$

$$\text{Keadaan } \textit{underreinforced}, A_s = 251,429 \leq A_{sb} = 682,587$$

3). Kapasitas momen pelat

$$a = \frac{A_s \cdot f_y}{0,85 \cdot f_c \cdot b}$$

$$a = \frac{251,429 \times 348,904}{0,85 \times 11,47 \times 450}$$

$$a = 20,00 \text{ mm}$$

$$M_n = A_s \cdot f_y (d - 0,5 \cdot a)$$

$$M_n = 251,429 \times 348,904 \times (101 - 0,5 \times 20,00)$$

$$M_n = 7,981 \text{ kN.m}$$

$$M_u = \phi \cdot M_n$$

$$M_u = \phi \cdot 7,981 = 7,182 \text{ kN.m}$$

4). Beban Maksimum Pelat

$$M = \frac{PL}{6}$$

$$P = \frac{7,182 \times 6}{1,35}$$

$$P = 31,92 \text{ kN} = 3255,63 \text{ kg}$$



3. Perhitungan Beban PLBRB-01

a. Dimensi PLBRB-01

- 1). Panjang : 1350 mm
- 2). Lebar : 450 mm
- 3). Tebal : 125 mm
- 4). Selimut Beton : 20 mm
- 5). f'_c : 11,51 MPa

b. Dimensi Baja Tulangan P8

- 1). Diameter : 7,8 mm
- 2). f_y : 348,904 MPa
- 3). f_u : 486,413 MPa

c. Perhitungan

- 1). Menentukan luas tulangan dalam keadaan seimbang

$$\rho_b = \frac{0,85 \cdot f'_c \cdot \beta_1}{f_y} \times \frac{600}{600 + f_y}$$

$$\rho_b = \frac{0,85 \times 11,51 \times 0,85}{348,904} \times \frac{600}{600 + 348,904}$$

$$\rho_b = 0,0151$$

$$\rho_{max} = 0,0403$$

$$A_{sb} = 0,0151 \times 450 \times 101 = 684,968$$



$$2). A_s = 5 \times \left(\frac{1}{4} \pi d^2 \right)$$

$$A_s = 5 \times \left(\frac{1}{4} \pi \cdot 7,8^2 \right)$$

$$A_s = 251,429 \text{ mm}^2$$

$$\text{Keadaan } \textit{underreinforced}, A_s = 251,429 \leq A_{sb} = 684,968$$

3). Kapasitas momen pelat

$$a = \frac{A_s \cdot f_y}{0,85 \cdot f_c \cdot b}$$

$$a = \frac{251,429 \times 348,904}{0,85 \times 11,51 \times 450}$$

$$a = 19,93 \text{ mm}$$

$$M_n = A_s \cdot f_y (d - 0,5 \cdot a)$$

$$M_n = 251,429 \times 348,904 \times (101 - 0,5 \times 19,93)$$

$$M_n = 7,984 \text{ kN.m}$$

$$M_u = \phi \cdot M_n$$

$$M_u = \phi \cdot 7,984 = 7,185 \text{ kN.m}$$

4). Beban Maksimum Pelat

$$M = \frac{PL}{6}$$

$$P = \frac{7,185 \times 6}{1,35}$$

$$P = 31,93 \text{ kN} = 3256,88 \text{ kg}$$



4. Perhitungan Beban PLBRB-02

a. Dimensi PLBRB-02

- 1). Panjang : 1350 mm
- 2). Lebar : 450 mm
- 3). Tebal : 125 mm
- 4). Selimut Beton : 20 mm
- 5). f'_c : 11,60 MPa

b. Dimensi Baja Tulangan P8

- 1). Diameter : 7,8 mm
- 2). f_y : 348,904 MPa
- 3). f_u : 486,413 MPa

c. Perhitungan

- 1). Menentukan luas tulangan dalam keadaan seimbang

$$\rho_b = \frac{0,85 \cdot f'_c \cdot \beta_1}{f_y} \times \frac{600}{600 + f_y}$$

$$\rho_b = \frac{0,85 \times 11,60 \times 0,85}{348,904} \times \frac{600}{600 + 348,904}$$

$$\rho_b = 0,0152$$

$$\rho_{max} = 0,0403$$

$$A_{sb} = 0,0152 \times 450 \times 101 = 690,324$$



$$2). A_s = 5 \times \left(\frac{1}{4} \pi d^2 \right)$$

$$A_s = 5 \times \left(\frac{1}{4} \pi \cdot 7,8^2 \right)$$

$$A_s = 251,429 \text{ mm}^2$$

Keadaan *underreinforced*, $A_s = 251,429 \leq A_{sb} = 690,324$

3). Kapasitas momen pelat

$$a = \frac{A_s \cdot f_y}{0,85 \cdot f_c \cdot b}$$

$$a = \frac{251,429 \times 348,904}{0,85 \times 11,60 \times 450}$$

$$a = 19,77 \text{ mm}$$

$$M_n = A_s \cdot f_y (d - 0,5 \cdot a)$$

$$M_n = 251,429 \times 348,904 \times (101 - 0,5 \times 19,77)$$

$$M_n = 7,990 \text{ kN.m}$$

$$M_u = \phi \cdot M_n$$

$$M_u = \phi \cdot 7,990 = 7,191 \text{ kN.m}$$

4) Beban Maksimum Pelat

$$M = \frac{PL}{6}$$

$$P = \frac{7,191 \times 6}{1,35}$$

$$P = 31,96 \text{ kN} = 3259,65 \text{ kg}$$



5. Perhitungan Beban PLC-01

a. Dimensi PLC-01

- 1). Panjang : 1350 mm
- 2). Lebar : 450 mm
- 3). Tebal : 125 mm
- 4). Selimut Beton : 20 mm
- 5). f'_c : 5,82 MPa

b. Dimensi Baja Tulangan P8

- 1). Diameter : 6 mm
- 2). f_y : 320 MPa (asumsi)
- 3). f_u : 400 MPa (asumsi)

c. Perhitungan

- 1). Menentukan luas tulangan dalam keadaan seimbang

$$\rho_b = \frac{0,85 \cdot f'_c \cdot \beta_1}{f_y} \times \frac{600}{600 + f_y}$$

$$\rho_b = \frac{0,85 \times 5,82 \times 0,85}{320} \times \frac{600}{600 + 320}$$

$$\rho_b = 0,0086$$

$$\rho_{max} = 0,0403$$

$$A_{sb} = 0,0086 \times 450 \times 102 = 393,357$$



$$2). A_s = 5 \times \left(\frac{1}{4} \pi d^2 \right)$$

$$A_s = 5 \times \left(\frac{1}{4} \pi \cdot 6^2 \right)$$

$$A_s = 141,372 \text{ mm}^2$$

$$\text{Keadaan } \textit{underreinforced}, A_s = 141,372 \leq A_{sb} = 393,357$$

3). Kapasitas momen pelat

$$a = \frac{A_s \cdot f_y}{0,85 \cdot f_c \cdot b}$$

$$a = \frac{141,372 \times 320}{0,85 \times 5,82 \times 450}$$

$$a = 20,32 \text{ mm}$$

$$M_n = A_s \cdot f_y (d - 0,5 \cdot a)$$

$$M_n = 141,372 \times 320 \times (102 - 0,5 \times 20,32)$$

$$M_n = 4,153 \text{ kN.m}$$

$$M_u = \phi \cdot M_n$$

$$M_u = \phi \cdot 4,135 = 3,738 \text{ kN.m}$$

4) Beban Maksimum Pelat

$$M = \frac{PL}{6}$$

$$P = \frac{3,738 \times 6}{1,35}$$

$$P = 16,61 \text{ kN} = 1694,34 \text{ kg}$$



6. Perhitungan Beban PLC-02

a. Dimensi PLC-02

- 1). Panjang : 1350 mm
- 2). Lebar : 450 mm
- 3). Tebal : 125 mm
- 4). Selimut Beton : 20 mm
- 5). f'_c : 4,26 MPa

d. Dimensi Baja Tulangan P8

- 1). Diameter : 6 mm
- 2). f_y : 320 MPa (asumsi)
- 3). f_u : 400 MPa (asumsi)

e. Perhitungan

- 1). Menentukan luas tulangan dalam keadaan seimbang

$$\rho_b = \frac{0,85 \cdot f'_c \cdot \beta_1}{f_y} \times \frac{600}{600 + f_y}$$

$$\rho_b = \frac{0,85 \times 4,26 \times 0,85}{320} \times \frac{600}{600 + 320}$$

$$\rho_b = 0,0063$$

$$\rho_{max} = 0,0403$$

$$A_{sb} = 0,0063 \times 450 \times 102 = 287,921$$



$$2). A_s = 5 \times \left(\frac{1}{4} \pi d^2 \right)$$

$$A_s = 5 \times \left(\frac{1}{4} \pi \cdot 6^2 \right)$$

$$A_s = 141,372 \text{ mm}^2$$

$$\text{Keadaan } \textit{underreinforced}, A_s = 141,372 \leq A_{sb} = 287,921$$

3). Kapasitas momen pelat

$$a = \frac{A_s \cdot f_y}{0,85 \cdot f_c \cdot b}$$

$$a = \frac{141,372 \times 320}{0,85 \times 4,26 \times 450}$$

$$a = 27,76 \text{ mm}$$

$$M_n = A_s \cdot f_y (d - 0,5 \cdot a)$$

$$M_n = 141,372 \times 320 \times (102 - 0,5 \times 27,76)$$

$$M_n = 3,984 \text{ kN.m}$$

$$M_u = \phi \cdot M_n$$

$$M_u = \phi \cdot 3,984 = 3,586 \text{ kN.m}$$

4) Beban Maksimum Pelat

$$M = \frac{PL}{6}$$

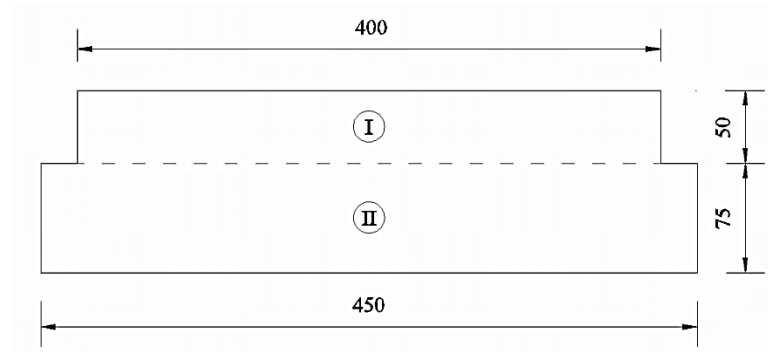
$$P = \frac{3,586 \times 6}{1,35}$$

$$P = 15,94 \text{ kN} = 1625,47 \text{ kg}$$



B. Perhitungan Beban Retak Pertama

1) Momen Inersia Penampang *Solid*



$$y = \frac{(450 \times 75) \times 37,5 + (400 \times 50) \times 100}{(450 \times 75) + (400 \times 50)}$$

$$y = 60,76 \text{ mm}$$

$$I_1 = \frac{1}{12} \times 400 \times 50^3 + (100 - 60,76)^2 \times (400 \times 50) = 34962218,67 \text{ mm}^4$$

$$I_2 = \frac{1}{12} \times 450 \times 75^3 + (60,76 - 37,5)^2 \times (450 \times 75) = 34079994 \text{ mm}^4$$

$$I_g = I_1 + I_2 = 69042212,67 \text{ mm}^4$$

2) Beban Retak Pertama PLBR-01

$$f_r = 0,62 \lambda \times \sqrt{f'c}$$

$$f_r = 0,62 \times 0,85 \times \sqrt{10,85}$$

$$f_r = 1,736 \text{ MPa}$$

$$M_{cr} = \frac{f_r \times I_g}{y_t}$$

$$M_{cr} = \frac{1,736 \times 69042212,67}{60,76} \times 10^{-6}$$

$$M_{cr} = 1,973 \text{ kN.m}$$

$$P = \frac{1,973 \times 6}{1,35}$$



$$P = 8,77 \text{ kN}$$

3) Beban Retak Pertama PLBR-02

$$f_r = 0,62\lambda \times \sqrt{f'_c}$$

$$f_r = 0,62 \times 0,85 \times \sqrt{11,47}$$

$$f_r = 1,785 \text{ MPa}$$

$$M_{cr} = \frac{f_r \times I_g}{y_t}$$

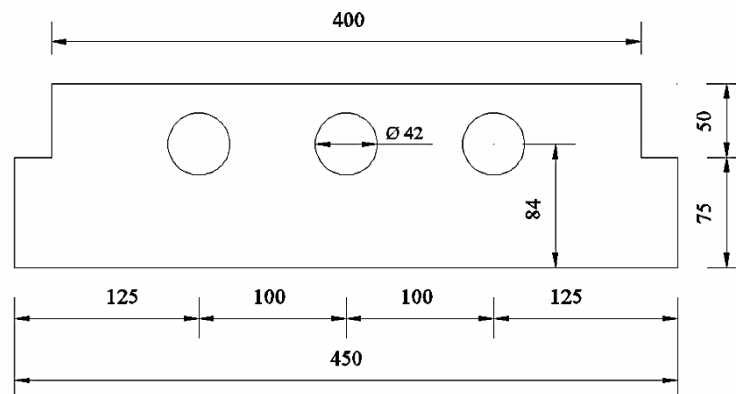
$$M_{cr} = \frac{1,785 \times 69042212,67}{60,76} \times 10^{-6}$$

$$M_{cr} = 2,028 \text{ kN.m}$$

$$P = \frac{2,028 \times 6}{1,35}$$

$$P = 9,01 \text{ kN}$$

4) Momen Inersia Penampang Berongga



$$y = \frac{(450 \times 75) \times 37,5 + (400 \times 50) \times 100 - ((\pi \times 21^2 \times 84) \times 3)}{(450 \times 75) + (400 \times 50) - ((\pi \times 21^2) \times 3)}$$

$$y = 58,81 \text{ mm}$$

$$I_1 = \frac{1}{12} \times 400 \times 50^3 + (100 - 58,81)^2 \times (400 \times 50) = 38098988,67 \text{ mm}^4$$



$$I_2 = \frac{1}{12} \times 450 \times 75^3 + (58,81 - 37,5)^2 \times (450 \times 75) = 31146730,88 \text{ mm}^4$$

Inersia Lingkaran

$$I_{x1} = \frac{\pi \times 21^4}{4} + (\pi \times 21^2) \times (84 - 58,81)^2 = 1031862,118 \text{ mm}^4$$

$$I_{x2} = \frac{\pi \times 21^4}{4} + (\pi \times 21^2) \times (84 - 58,81)^2 = 1031862,118 \text{ mm}^4$$

$$I_{x3} = \frac{\pi \times 21^4}{4} + (\pi \times 21^2) \times (84 - 58,81)^2 = 1031862,118 \text{ mm}^4 +$$
$$= 3095586,36 \text{ mm}^4$$

$$I_{y1} = \frac{\pi \times 21^4}{4} + (\pi \times 21^2) \times (225 - 125)^2 = 14007145,02 \text{ mm}^4$$

$$I_{y2} = \frac{\pi \times 21^4}{4} + (\pi \times 21^2) \times (225 - 225)^2 = 152745,02 \text{ mm}^4$$

$$I_{y3} = \frac{\pi \times 21^4}{4} + (\pi \times 21^2) \times (325 - 225)^2 = 14007145,02 \text{ mm}^4 +$$
$$= 28167035,06 \text{ mm}^4$$

$$I_p = I_x + I_y = 31262621,42 \text{ mm}^4$$

$$I_g = I_{beton} - I_{lubang} = 37983098,13 \text{ mm}^4$$

5) Beban Retak Pertama PLBRB-01

$$f_r = 0,62 \lambda \times \sqrt{f'_c}$$

$$f_r = 0,62 \times 0,85 \times \sqrt{11,51}$$

$$f_r = 1,788 \text{ MPa}$$

$$M_{cr} = \frac{f_r \times I_g}{y_t}$$

$$M_{cr} = \frac{1,788 \times 37983098,13}{58,81} \times 10^{-6}$$

$$M_{cr} = 1,15 \text{ kN.m}$$



$$P = \frac{1,15 \times 6}{1,35}$$

$$P = 5,13 \text{ kN}$$

6) Beban Retak Pertama PLBRB-02

$$f_r = 0,62 \lambda \times \sqrt{f'c}$$

$$f_r = 0,62 \times 0,85 \times \sqrt{11,60}$$

$$f_r = 1,795 \text{ MPa}$$

$$M_{cr} = \frac{f_r \times I_g}{y_t}$$

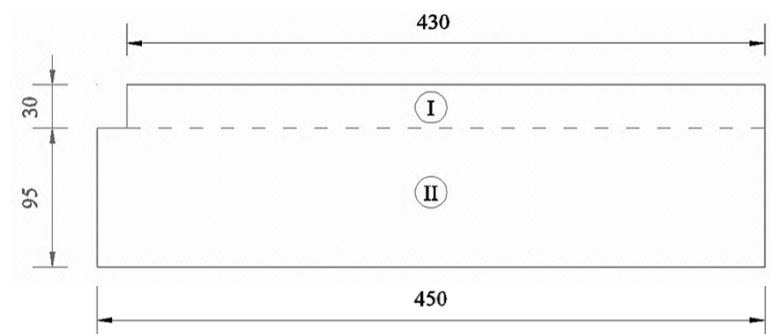
$$M_{cr} = \frac{1,795 \times 37983098,13}{58,81} \times 10^{-6}$$

$$M_{cr} = 1,16 \text{ kN.m}$$

$$P = \frac{1,16 \times 6}{1,35}$$

$$P = 5,15 \text{ kN}$$

7) Momen Inersia Penampang Citicon



$$y = \frac{(450 \times 95) \times 47,5 + (430 \times 30) \times 110}{(450 \times 95) + (430 \times 30)}$$

$$y = 61,99 \text{ mm}$$

$$I_1 = \frac{1}{12} \times 430 \times 30^3 + (110 - 61,99)^2 \times (430 \times 30) = 30701485,29 \text{ mm}^4$$



$$I_2 = \frac{1}{12} \times 450 \times 95^3 + (61,99 - 47,5)^2 \times (450 \times 95) = 41127356,78 \text{ mm}^4$$

$$I_g = I_1 + I_2 = 71828842,07 \text{ mm}^4$$

8) Beban Retak Pertama PLC-01

$$f_r = 0,62\lambda \times \sqrt{f'c}$$

$$f_r = 0,62 \times 0,75 \times \sqrt{5,82}$$

$$f_r = 1,122 \text{ MPa}$$

$$M_{cr} = \frac{f_r \times I_g}{y_t}$$

$$M_{cr} = \frac{1,122 \times 71828842,07}{61,99} \times 10^{-6}$$

$$M_{cr} = 1,3 \text{ kN.m}$$

$$P = \frac{1,3 \times 6}{1,35}$$

$$P = 5,78 \text{ kN}$$

9) Beban Retak Pertama PLC-02

$$f_r = 0,62\lambda \times \sqrt{f'c}$$

$$f_r = 0,62 \times 0,75 \times \sqrt{4,26}$$

$$f_r = 0,960 \text{ MPa}$$

$$M_{cr} = \frac{f_r \times I_g}{y_t}$$

$$M_{cr} = \frac{0,960 \times 71828842,07}{61,99} \times 10^{-6}$$

$$M_{cr} = 1,112 \text{ kN.m}$$

$$P = \frac{1,112 \times 6}{1,35}$$

$$P = 4,94 \text{ kN}$$



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Benda Uji	L (m)	f'_c (MPa)	λ	I_g (mm ⁴)	y_t (mm)	f_r (MPa)	M_{cr} (kN.m)	P (kN)
PLBR-01	1,35	10,85	0,85	69042212,67	60,76	1,736	1,973	8,77
PLBR-02	1,35	11,47	0,85	69042212,67	60,76	1,785	2,028	9,01
PLBRB-01	1,35	11,51	0,85	37983098,13	58,81	1,788	1,155	5,13
PLBRB-02	1,35	11,6	0,85	37983098,13	58,81	1,795	1,159	5,15
PLBC-01	1,35	5,82	0,75	71828842,07	61,99	1,122	1,300	5,78
PLC-02	1,35	4,26	0,75	71828842,07	61,99	0,960	1,112	4,94



LAMPIRAN VI
TABEL BEBAN, DEFLEKSI, KEKAKUAN DAN MOMEN PELAT

PLBR-01 : PANEL LANTAI BETON RINGAN 01



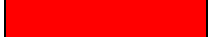
BEBAN (P) kN	DEFLEKSI (δ) mm	KEKAKUAN (k) kN/mm	MOMEN (M) kN.m				
-0,008	0,0336	-0,2428	-0,0018	10,036	4,8564	2,0665	2,2580
-0,009	0,0345	-0,2468	-0,0019	9,933	5,1094	1,9440	2,2349
-0,004	0,0353	-0,0996	-0,0008	10,320	5,2594	1,9622	2,3220
0,004	0,0386	0,0933	0,0008	10,649	5,4039	1,9706	2,3960
0,255	0,1304	1,9523	0,0573	10,955	5,5452	1,9756	2,4649
0,432	0,1992	2,1695	0,0972	11,258	5,6866	1,9797	2,5330
0,614	0,2702	2,2744	0,1383	11,553	5,8279	1,9824	2,5994
0,808	0,3455	2,3374	0,1817	11,834	5,9659	1,9836	2,6627
1,011	0,4245	2,3817	0,2275	12,111	6,0978	1,9862	2,7250
1,245	0,5124	2,4298	0,2801	12,361	6,2357	1,9824	2,7813
1,569	0,6340	2,4754	0,3531	12,609	6,3819	1,9757	2,8370
1,908	0,7574	2,5196	0,4294	12,888	6,5279	1,9743	2,8998
2,243	0,8763	2,5595	0,5046	13,176	6,6731	1,9745	2,9646
2,559	0,9889	2,5878	0,5758	13,461	6,8202	1,9736	3,0286
2,654	1,0414	2,5487	0,5972	13,715	6,9645	1,9693	3,0858
2,999	1,1585	2,5887	0,6748	13,997	7,1184	1,9663	3,1492
3,244	1,2435	2,6090	0,7300	14,285	7,2818	1,9617	3,2141
3,540	1,3475	2,6271	0,7965	14,579	7,4435	1,9587	3,2804
3,842	1,4524	2,6452	0,8644	14,872	7,5968	1,9577	3,3463
4,157	1,5636	2,6584	0,9352	15,160	7,7513	1,9557	3,4109
4,479	1,6789	2,6680	1,0079	15,453	7,9069	1,9543	3,4768
4,800	1,7972	2,6709	1,0800	15,749	8,0643	1,9529	3,5434
5,112	1,9128	2,6722	1,1501	16,048	8,2252	1,9511	3,6108
5,416	2,0290	2,6695	1,2187	16,204	8,3991	1,9293	3,6459
5,721	2,1451	2,6669	1,2872	16,342	8,5832	1,9040	3,6770
6,030	2,2640	2,6636	1,3568	16,597	8,7523	1,8963	3,7343
6,340	2,3849	2,6583	1,4265	16,908	8,9139	1,8968	3,8044
6,650	2,5099	2,6497	1,4963	17,213	9,0721	1,8974	3,8730
6,962	2,6360	2,6410	1,5664	17,504	9,2282	1,8968	3,9384
7,274	2,7642	2,6314	1,6366	17,789	9,3829	1,8959	4,0026
7,548	2,9114	2,5926	1,6983	18,082	9,5388	1,8957	4,0686
7,719	3,1070	2,4846	1,7369	18,374	9,6960	1,8950	4,1341
8,021	3,2492	2,4686	1,8047	18,663	9,8519	1,8944	4,1993
8,299	3,3845	2,4521	1,8674	18,946	10,0057	1,8936	4,2629
8,365	3,5572	2,3516	1,8822	19,216	10,1721	1,8891	4,3236
8,599	3,7091	2,3185	1,9349	19,491	10,3212	1,8884	4,3854
8,793	3,8559	2,2803	1,9784	19,773	10,4744	1,8877	4,4489
8,990	4,0173	2,2378	2,0227	20,053	10,6274	1,8869	4,5120
9,301	4,1561	2,2378	2,0927	20,340	10,7875	1,8855	4,5765
9,607	4,2923	2,2383	2,1617	20,627	10,9493	1,8839	4,6411
9,765	4,4801	2,1797	2,1972	20,918	11,1078	1,8832	4,7065
10,018	4,6463	2,1562	2,2541	21,212	11,2706	1,8820	4,7726
				20,934	11,5076	1,8191	4,7101
				20,997	11,7235	1,7911	4,7244
				21,438	11,9010	1,8014	4,8237



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21,804	12,0710	1,8063	4,9060
22,140	12,2400	1,8088	4,9814
22,465	12,4106	1,8102	5,0547
22,786	12,5859	1,8104	5,1268
23,102	12,7636	1,8100	5,1979
23,396	12,9434	1,8076	5,2641
23,720	13,1236	1,8075	5,3371
24,046	13,3052	1,8073	5,4103
24,377	13,4861	1,8076	5,4849
24,720	13,6674	1,8087	5,5621
25,051	13,8545	1,8081	5,6364
25,399	14,0408	1,8089	5,7148
25,764	14,2255	1,8111	5,7969
26,137	14,4095	1,8139	5,8808
26,514	14,5959	1,8165	5,9655
26,892	14,7814	1,8193	6,0507
27,271	14,9673	1,8220	6,1359
27,646	15,1537	1,8244	6,2204
28,011	15,3419	1,8258	6,3025
28,385	15,5298	1,8278	6,3867
28,762	15,7202	1,8296	6,4713
29,106	15,9118	1,8292	6,5489
29,388	16,1012	1,8252	6,6124
29,757	16,2892	1,8268	6,6953
30,129	16,4814	1,8281	6,7791
30,503	16,6741	1,8294	6,8632
30,875	16,8674	1,8304	6,9468
31,240	17,0607	1,8311	7,0289
31,591	17,2544	1,8309	7,1079
31,946	17,4495	1,8308	7,1878
32,187	17,6522	1,8234	7,2420
32,377	17,8618	1,8126	7,2848
32,541	18,0728	1,8005	7,3216
32,664	18,2876	1,7861	7,3493
32,828	18,4989	1,7746	7,3862
32,888	18,7194	1,7569	7,3999
32,933	18,9503	1,7378	7,4099
32,555	19,2189	1,6939	7,3249
32,458	19,4654	1,6675	7,3031
32,520	19,6975	1,6510	7,3171
32,627	19,9215	1,6378	7,3412
32,763	20,1445	1,6264	7,3716
32,913	20,3669	1,6160	7,4055
33,064	20,5897	1,6059	7,4395
33,219	20,8095	1,5964	7,4743
33,341	21,0334	1,5852	7,5018
33,412	21,2626	1,5714	7,5177
33,447	21,4951	1,5560	7,5255
33,417	21,7325	1,5377	7,5189
33,415	21,9688	1,5210	7,5184
33,437	22,1665	1,5084	7,5233

	Beban Retak Pertama
	Kekakuan Pelat
	Beban Max



PLBR-02 : PANEL LANTAI BETON RINGAN 02

BEBAN (P) kN	DEFLEKSI (δ) mm	KEKAKUAN (k) kN/mm	MOMEN (M) kN.m
-0,024	0,0381	-0,6309	-0,0054
0,026	0,0698	0,3680	0,0058
0,069	0,1328	0,5187	0,0155
0,099	0,2155	0,4590	0,0223
0,519	0,3012	1,7223	0,1167
0,531	0,3899	1,3617	0,1195
0,713	0,4813	1,4824	0,1605
0,906	0,5248	1,7273	0,2040
1,110	0,6687	1,6601	0,2498
1,344	0,7094	1,8947	0,3024
1,668	0,8131	2,0517	0,3754
2,007	0,9403	2,1346	0,4516
2,342	1,0791	2,1701	0,5269
2,658	1,0843	2,4514	0,5981
2,753	1,1119	2,4760	0,6195
3,098	1,2274	2,5241	0,6970
3,343	1,3777	2,4266	0,7522
3,639	1,5291	2,3797	0,8187
3,941	1,6866	2,3365	0,8867
4,256	1,8661	2,2804	0,9575
4,578	1,9228	2,3810	1,0301
4,899	2,0765	2,3592	1,1023
5,210	2,2303	2,3362	1,1723
5,515	2,3840	2,3135	1,2410
5,820	2,5377	2,2933	1,3095
6,129	2,6915	2,2773	1,3791
6,439	2,8452	2,2631	1,4487
6,749	2,9990	2,2505	1,5186
7,061	3,1693	2,2278	1,5887
7,373	3,2367	2,2778	1,6588
7,647	3,4161	2,2386	1,7206
7,818	3,6138	2,1635	1,7591
8,120	3,7980	2,1379	1,8270
8,398	3,9760	2,1123	1,8896
8,698	4,1599	2,0910	1,9571
9,377	4,3469	2,1571	2,1098
9,611	4,5355	2,1191	2,1625
10,117	4,7341	2,1371	2,2764
10,135	4,9987	2,0274	2,2803
10,032	5,1908	1,9326	2,2571
10,419	5,3838	1,9352	2,3442
10,748	5,5756	1,9276	2,4182
11,054	5,7674	1,9166	2,4872
11,357	5,9624	1,9047	2,5552
11,652	6,1689	1,8888	2,6217




11,933	6,3805	1,8703	2,6850
12,210	6,5826	1,8549	2,7473
12,460	6,7802	1,8378	2,8036
12,708	6,9730	1,8224	2,8592
12,987	7,1638	1,8129	2,9221
13,275	7,3554	1,8048	2,9868
13,559	7,5464	1,7968	3,0509
13,814	7,7384	1,7851	3,1081
14,095	7,9345	1,7765	3,1715
14,384	8,1568	1,7634	3,2363
14,678	8,3446	1,7590	3,3026
14,971	8,5342	1,7543	3,3686
15,259	8,7231	1,7492	3,4332
15,552	8,9159	1,7443	3,4991
15,848	9,1104	1,7395	3,5657
16,147	9,3094	1,7345	3,6330
16,303	9,5123	1,7139	3,6682
16,441	9,7154	1,6923	3,6992
16,696	9,9220	1,6827	3,7566
17,007	10,1304	1,6788	3,8266
17,312	10,3363	1,6749	3,8952
17,603	10,5420	1,6698	3,9607
17,888	10,7377	1,6659	4,0249
18,181	10,9493	1,6605	4,0908
18,473	11,1601	1,6552	4,1563
18,762	11,3735	1,6497	4,2215
19,045	11,5818	1,6444	4,2852
19,315	11,7916	1,6380	4,3459
19,589	12,0030	1,6320	4,4076
19,872	12,2145	1,6269	4,4711
20,152	12,4300	1,6213	4,5343
20,439	12,6473	1,6161	4,5987
20,726	12,8540	1,6124	4,6634
21,017	13,0728	1,6077	4,7288
21,311	13,2888	1,6037	4,7949
21,033	13,4978	1,5582	4,7323
21,096	13,7076	1,5390	4,7467
21,537	13,9227	1,5469	4,8459
21,903	14,1553	1,5473	4,9282
22,239	14,3686	1,5477	5,0037
22,564	14,5904	1,5465	5,0770
22,885	14,8057	1,5457	5,1491
23,201	15,0367	1,5429	5,2202
23,495	15,2577	1,5399	5,2864
23,819	15,4752	1,5392	5,3593
24,145	15,6968	1,5382	5,4326
24,476	15,9229	1,5372	5,5071
24,819	16,1458	1,5372	5,5844



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25,150	16,3726	1,5361	5,6587
25,498	16,5934	1,5366	5,7370
25,863	16,8270	1,5370	5,8192
26,236	17,0874	1,5354	5,9030
26,612	17,3742	1,5317	5,9878
26,991	17,7235	1,5229	6,0730
27,370	18,0261	1,5183	6,1582
27,745	18,3128	1,5151	6,2426
28,110	18,5844	1,5126	6,3248
28,484	18,8503	1,5111	6,4089
28,860	19,1106	1,5102	6,4936
29,205	19,3817	1,5068	6,5712
29,487	19,6641	1,4996	6,6347
29,856	19,9345	1,4977	6,7175
30,228	20,2089	1,4958	6,8014
30,602	20,4867	1,4937	6,8854
30,974	20,7670	1,4915	6,9691
31,339	21,0439	1,4892	7,0512
31,690	21,3169	1,4866	7,1302
32,045	21,5854	1,4846	7,2101
32,286	21,8525	1,4774	7,2643
32,640	22,1243	1,4753	7,3439
33,037	22,3942	1,4753	7,4334
33,545	22,6609	1,4803	7,5477
33,850	22,9317	1,4761	7,6162
34,154	23,2042	1,4719	7,6846
34,536	23,4738	1,4713	7,7707
34,789	23,7403	1,4654	7,8276
35,065	24,0090	1,4605	7,8897
35,454	24,2011	1,4650	7,9772
35,547	24,4560	1,4535	7,9981

	Beban Retak Pertama
	Kekakuan Pelat
	Beban Max



PLBRB-01 : PANEL LANTAI BETON RINGAN BERONGGA 01

BEBAN (P)	DEFLEKSI (δ)	KEKAKUAN (k)	MOMEN (M)
kN	mm	kN/mm	kN.m
-0,005	-0,0084	0,6523	-0,0012
0,384	0,1458	2,6341	0,0864
0,839	0,3331	2,5175	0,1887
1,053	0,4249	2,4776	0,2369
1,274	0,5196	2,4520	0,2866
1,508	0,6066	2,4860	0,3393
1,785	0,6887	2,5923	0,4017
2,063	0,7707	2,6773	0,4643
2,168	0,8038	2,6977	0,4879
2,394	0,8634	2,7731	0,5387
2,637	0,9378	2,8117	0,5933
2,909	1,0156	2,8639	0,6545
3,257	1,1157	2,9191	0,7328
3,631	1,2304	2,9511	0,8170
3,972	1,3344	2,9764	0,8936
4,373	1,4530	3,0097	0,9840
4,760	1,5696	3,0329	1,0711
5,145	1,6876	3,0488	1,1577
5,512	1,8123	3,0413	1,2401
5,890	1,9475	3,0242	1,3252
6,229	2,0905	2,9795	1,4014
6,514	2,2625	2,8789	1,4656
6,652	2,4944	2,6669	1,4967
6,964	2,6628	2,6154	1,5669
7,273	2,8144	2,5840	1,6363
7,597	2,9615	2,5654	1,7094
7,891	3,1225	2,5271	1,7755
7,980	3,3453	2,3853	1,7954
8,246	3,5200	2,3427	1,8554
8,546	3,6558	2,3376	1,9227
8,802	3,7774	2,3302	1,9805
9,074	3,9041	2,3243	2,0417
9,359	4,0341	2,3199	2,1057
9,680	4,1743	2,3188	2,1779
10,014	4,3251	2,3154	2,2533
10,342	4,4755	2,3108	2,3270
10,657	4,6207	2,3064	2,3979
10,959	4,7635	2,3006	2,4657
11,237	4,9018	2,2925	2,5284
11,517	5,0390	2,2856	2,5914
11,789	5,1758	2,2777	2,6525
12,005	5,3232	2,2552	2,7011
12,214	5,4888	2,2252	2,7480
12,518	5,6328	2,2223	2,8165
12,794	5,7713	2,2169	2,8788
13,051	5,9037	2,2106	2,9364




13,311	6,0432	2,2027	2,9950
13,612	6,1963	2,1968	3,0627
13,488	6,4221	2,1003	3,0348
13,777	6,6215	2,0807	3,0999
14,146	6,7876	2,0841	3,1828
14,472	6,9513	2,0820	3,2563
14,819	7,1185	2,0817	3,3342
15,163	7,2895	2,0800	3,4116
15,503	7,4630	2,0773	3,4881
15,848	7,6385	2,0748	3,5658
16,192	7,8155	2,0718	3,6432
16,536	7,9960	2,0680	3,7206
16,884	8,1791	2,0643	3,7990
17,236	8,3620	2,0613	3,8782
17,586	8,5432	2,0585	3,9569
17,940	8,7273	2,0556	4,0366
18,290	8,9123	2,0522	4,1152
18,634	9,0986	2,0480	4,1926
18,964	9,2873	2,0419	4,2669
19,282	9,4762	2,0348	4,3384
19,631	9,6656	2,0310	4,4170
19,982	9,8526	2,0281	4,4961
20,333	10,0391	2,0254	4,5750
20,683	10,2260	2,0226	4,6537
21,035	10,4142	2,0198	4,7329
21,389	10,6016	2,0175	4,8124
21,744	10,7898	2,0152	4,8924
22,099	10,9780	2,0130	4,9722
22,456	11,1687	2,0106	5,0526
22,818	11,3601	2,0086	5,1340
23,182	11,5532	2,0066	5,2160
23,548	11,7483	2,0044	5,2984
23,914	11,9426	2,0024	5,3806
24,279	12,1380	2,0002	5,4628
24,646	12,3335	1,9983	5,5454
25,014	12,5305	1,9962	5,6281
25,378	12,7254	1,9943	5,7101
25,748	12,9221	1,9926	5,7934
26,120	13,1192	1,9909	5,8769
26,491	13,3184	1,9890	5,9604
26,860	13,5197	1,9867	6,0436
27,227	13,7241	1,9839	6,1260
27,590	13,9317	1,9804	6,2077
27,947	14,1382	1,9767	6,2881
28,271	14,3410	1,9713	6,3609
28,633	14,5479	1,9682	6,4424
28,984	14,7604	1,9636	6,5214
29,332	14,9749	1,9587	6,5996
29,657	15,1904	1,9523	6,6728



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29,977	15,4066	1,9458	6,7449
30,306	15,6245	1,9397	6,8189
30,626	15,8455	1,9328	6,8909
30,939	16,0694	1,9254	6,9614
31,236	16,2957	1,9168	7,0281
31,410	16,5358	1,8995	7,0672
31,619	16,7739	1,8850	7,1143
31,734	17,0175	1,8648	7,1402
31,525	17,2982	1,8224	7,0931
31,475	17,5719	1,7912	7,0818
31,521	17,8378	1,7671	7,0921
31,568	18,1013	1,7440	7,1029
31,626	18,3642	1,7221	7,1158
31,708	18,6258	1,7024	7,1343
31,806	18,8877	1,6840	7,1564
31,897	19,1499	1,6656	7,1768
31,996	19,4121	1,6483	7,1992
32,073	19,6808	1,6296	7,2164
32,074	19,9565	1,6072	7,2166
32,136	20,2277	1,5887	7,2306
32,223	20,4966	1,5721	7,2503
32,316	20,7672	1,5561	7,2710
32,411	21,0369	1,5407	7,2925
32,525	21,3033	1,5268	7,3181
32,642	21,5674	1,5135	7,3444
32,738	21,8334	1,4994	7,3660
32,801	22,0975	1,4844	7,3801
32,901	22,3574	1,4716	7,4028
33,017	22,6189	1,4597	7,4288
33,127	22,8800	1,4479	7,4537
32,931	23,0209	1,4305	7,4095
32,683	23,0313	1,4190	7,3536
32,539	23,0335	1,4127	7,3212
32,433	23,0338	1,4081	7,2975
32,349	23,0337	1,4044	7,2786
32,279	23,0338	1,4014	7,2627
32,217	23,0341	1,3987	7,2488
32,161	23,0340	1,3962	7,2363
32,111	23,0343	1,3940	7,2249
32,064	23,0341	1,3920	7,2145
32,021	23,0338	1,3902	7,2047
31,979	23,0334	1,3884	7,1952
31,940	23,0343	1,3866	7,1864
31,948	23,0374	1,3868	7,1884

	Beban Retak Pertama
	Kekakuan Pelat
	Beban Max



PLBRB-02 : PANEL LANTAI BETON RINGAN BERONGGA 02

BEBAN (P)	DEFLEKSI (δ)	KEKAKUAN (k)	MOMEN (M)
kN	mm	kN/mm	kN.m
0,788	0,3339	2,3607	0,1774
1,317	0,5583	2,3595	0,2964
1,865	0,7932	2,3513	0,4196
2,215	0,9393	2,3578	0,4983
2,467	1,0347	2,3841	0,5550
2,945	1,2169	2,4203	0,6627
3,431	1,4084	2,4360	0,7720
3,959	1,6095	2,4600	0,8909
4,571	1,8435	2,4797	1,0285
5,202	2,0920	2,4867	1,1705
5,736	2,3174	2,4753	1,2907
6,255	2,5330	2,4693	1,4073
6,750	2,7575	2,4477	1,5186
7,202	3,0041	2,3974	1,6204
7,623	3,2503	2,3454	1,7152
8,047	3,5038	2,2967	1,8106
8,546	3,7573	2,2745	1,9228
9,053	3,9825	2,2731	2,0368
9,572	4,2200	2,2684	2,1538
10,060	4,4720	2,2495	2,2635
10,455	4,7289	2,2108	2,3523
10,899	4,9890	2,1846	2,4523
11,407	5,2421	2,1760	2,5666
11,836	5,5000	2,1520	2,6631
12,286	5,7585	2,1335	2,7643
12,721	6,0158	2,1145	2,8622
13,175	6,2630	2,1036	2,9643
13,641	6,5169	2,0931	3,0692
14,077	6,7731	2,0784	3,1674
14,245	7,0581	2,0182	3,2051
14,674	7,3179	2,0052	3,3016
15,112	7,5767	1,9945	3,4002
15,465	7,8468	1,9708	3,4795
15,764	8,1385	1,9369	3,5468
16,195	8,4004	1,9279	3,6439
16,630	8,6585	1,9207	3,7418
17,052	8,9225	1,9112	3,8368
17,456	9,2258	1,8921	3,9277
17,863	9,5698	1,8666	4,0192
18,277	9,8285	1,8596	4,1122
18,687	10,0865	1,8527	4,2046
19,112	10,3461	1,8472	4,3001
19,547	10,6665	1,8325	4,3980
19,983	10,9569	1,8238	4,4961
20,421	11,2217	1,8198	4,5947
20,866	11,4861	1,8166	4,6949

21,321	11,7580	1,8133	4,7972
21,778	12,0248	1,8111	4,9000
22,229	12,2905	1,8086	5,0015
22,688	12,5580	1,8067	5,1048
23,138	12,8280	1,8037	5,2061
23,574	13,1740	1,7894	5,3041
24,028	13,4527	1,7861	5,4063
24,496	13,7211	1,7853	5,5116
24,974	13,9915	1,7850	5,6192
25,450	14,2619	1,7845	5,7262
25,922	14,5361	1,7833	5,8325
26,391	14,8113	1,7818	5,9380
26,857	15,0862	1,7802	6,0428
27,323	15,3624	1,7786	6,1478
27,779	15,6410	1,7760	6,2503
28,190	15,9275	1,7699	6,3428
28,638	16,2073	1,7670	6,4437
29,053	16,4930	1,7616	6,5370
29,282	16,8014	1,7428	6,5885
29,487	17,1153	1,7228	6,6345
29,449	17,4561	1,6871	6,6261
29,430	17,7992	1,6534	6,6217
29,555	18,1308	1,6301	6,6498
29,709	18,4615	1,6092	6,6844
29,882	18,7870	1,5906	6,7235
30,069	19,1107	1,5734	6,7655
30,221	19,4355	1,5550	6,7998
30,366	19,7631	1,5365	6,8323
30,551	20,0854	1,5211	6,8739
30,757	20,4036	1,5074	6,9202
31,019	20,7235	1,4968	6,9792
31,188	21,0509	1,4815	7,0173
31,305	21,4082	1,4623	7,0437
31,543	21,7437	1,4507	7,0971
31,659	22,0939	1,4329	7,1232
31,873	22,4347	1,4207	7,1713
31,884	22,8086	1,3979	7,1740
31,955	23,1915	1,3779	7,1899
32,145	23,5529	1,3648	7,2326
32,248	23,9263	1,3478	7,2557
32,360	24,3014	1,3316	7,2810
32,413	24,6763	1,3135	7,2930
32,447	25,0602	1,2948	7,3006
32,512	25,4428	1,2778	7,3152
32,563	25,8344	1,2604	7,3266
32,721	26,1998	1,2489	7,3623
32,849	26,5698	1,2363	7,3910
32,796	26,9442	1,2172	7,3790
32,844	27,2986	1,2031	7,3898



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32,962	27,6600	1,1917	7,4164
33,087	28,0148	1,1811	7,4447
33,197	28,3784	1,1698	7,4694
33,303	28,7373	1,1589	7,4932
33,390	29,1014	1,1474	7,5128
33,486	29,4612	1,1366	7,5344
33,559	29,7621	1,1276	7,5509

	Beban Retak Pertama
	Kekakuan Pelat
	Beban Max

PLC-01 : PANEL LANTAI CITICON 01

BEBAN (P) kN	DEFLEKSI (δ) mm	KEKAKUAN (k) kN/mm	MOMEN (M) kN.m
0,097	0,0467	2,0828	0,0219
0,094	0,0459	2,0436	0,0211
0,093	0,0446	2,0772	0,0208
0,094	0,0433	2,1743	0,0212
0,095	0,0427	2,2146	0,0213
0,096	0,0416	2,2994	0,0215
0,097	0,0426	2,2763	0,0218
0,095	0,0422	2,2389	0,0213
0,095	0,0419	2,2782	0,0215
0,099	0,0414	2,3949	0,0223
0,111	0,0445	2,4981	0,0250
0,105	0,0455	2,3146	0,0237
0,099	0,0437	2,2613	0,0222
0,097	0,0429	2,2526	0,0218
0,096	0,0425	2,2665	0,0217
0,203	0,0913	2,2266	0,0457
0,502	0,2422	2,0708	0,1128
0,703	0,3668	1,9165	0,1582
0,921	0,4875	1,8899	0,2073
1,141	0,6111	1,8670	0,2567
1,366	0,7395	1,8469	0,3073
1,595	0,8695	1,8339	0,3588
1,817	0,9978	1,8212	0,4089
1,995	1,1055	1,8047	0,4489
2,078	1,1591	1,7931	0,4676
2,200	1,2326	1,7847	0,4950
2,351	1,3210	1,7795	0,5289
2,543	1,4327	1,7750	0,5722
2,712	1,5340	1,7678	0,6102
2,898	1,6459	1,7608	0,6521
3,104	1,7723	1,7517	0,6985
3,330	1,9084	1,7447	0,7492




3,563	2,0512	1,7373	0,8018
3,800	2,1965	1,7303	0,8551
4,040	2,3415	1,7255	0,9090
4,282	2,4923	1,7183	0,9636
4,530	2,6428	1,7142	1,0193
4,776	2,7908	1,7112	1,0745
4,979	2,9176	1,7067	1,1204
5,256	3,0744	1,7095	1,1826
5,527	3,2266	1,7130	1,2436
5,794	3,3797	1,7143	1,3036
6,061	3,5321	1,7160	1,3638
6,324	3,6871	1,7150	1,4228
6,586	3,8437	1,7134	1,4818
6,848	3,9992	1,7124	1,5409
7,097	4,1560	1,7076	1,5968
7,329	4,3131	1,6993	1,6491
7,275	4,4954	1,6184	1,6370
7,089	4,6944	1,5102	1,5951
7,288	4,8604	1,4994	1,6397
7,489	5,0256	1,4901	1,6849
7,634	5,2014	1,4677	1,7176
7,675	5,3966	1,4221	1,7268
7,578	5,5694	1,3606	1,7050
7,501	5,7099	1,3138	1,6878
7,456	5,7219	1,3031	1,6776
7,487	5,7561	1,3007	1,6845
8,199	6,3884	1,2834	1,8447
8,540	6,9450	1,2297	1,9215
8,707	7,3081	1,1914	1,9591
8,764	7,7011	1,1380	1,9718
8,792	8,1623	1,0772	1,9783
9,033	8,5198	1,0602	2,0324
9,204	8,8456	1,0405	2,0709
9,438	9,1860	1,0274	2,1235
9,675	9,5334	1,0148	2,1768



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9,793	9,9085	0,9883	2,2034
10,028	10,2685	0,9766	2,2564
10,267	10,6208	0,9667	2,3101
10,498	10,9735	0,9567	2,3621
10,719	11,3291	0,9462	2,4119
10,930	11,6844	0,9355	2,4594
11,143	12,0387	0,9256	2,5072
11,340	12,4027	0,9143	2,5515
11,525	12,7639	0,9029	2,5930
11,706	13,1253	0,8919	2,6338
11,872	13,4913	0,8800	2,6713
12,031	13,8573	0,8682	2,7070
12,166	14,2189	0,8556	2,7374
12,303	14,5830	0,8437	2,7683
12,432	14,9477	0,8317	2,7972
12,551	15,3058	0,8200	2,8241
12,658	15,6693	0,8078	2,8480
12,747	16,0644	0,7935	2,8681
12,835	16,4639	0,7796	2,8879
12,914	16,8700	0,7655	2,9057
12,990	17,2858	0,7515	2,9228
13,057	17,7008	0,7376	2,9378
13,123	18,1266	0,7240	2,9527
13,181	18,5912	0,7090	2,9658
13,242	19,0167	0,6964	2,9795
13,293	19,4312	0,6841	2,9909
13,329	19,8560	0,6713	2,9989
13,305	20,3612	0,6535	2,9937
13,265	20,8719	0,6356	2,9847
13,350	21,3231	0,6261	3,0038
13,420	21,7550	0,6169	3,0195
13,474	22,1841	0,6074	3,0316
13,524	22,6065	0,5982	3,0429
13,572	23,0183	0,5896	3,0538
13,616	23,4276	0,5812	3,0635
13,656	23,8367	0,5729	3,0725
13,692	24,2437	0,5648	3,0807
13,731	24,6483	0,5571	3,0895
13,770	25,0533	0,5496	3,0983
13,806	25,4591	0,5423	3,1064
13,839	25,8787	0,5348	3,1138
13,871	26,2910	0,5276	3,1210
13,902	26,6935	0,5208	3,1280
13,935	27,0982	0,5142	3,1353
13,968	27,4943	0,5080	3,1428
14,000	27,8890	0,5020	3,1501
14,031	28,2932	0,4959	3,1571

	Beban Retak Pertama
	Kekakuan Pelat
	Beban Max



PLC-02 : PANEL LANTAI CITICON 02

BEBAN (P)	DEFLEKSI (δ)	KEKAKUAN (k)	MOMEN (M)
kN	mm	kN/mm	kN.m
0,025	-0,0146	-1,7311	0,0057
0,025	-0,0145	-1,7107	0,0056
0,045	-0,0057	-7,9026	0,0101
0,148	0,0521	2,8425	0,0333
0,384	0,1895	2,0253	0,0864
0,672	0,3560	1,8867	0,1511
0,969	0,5290	1,8322	0,2181
1,279	0,7099	1,8021	0,2878
1,585	0,8884	1,7839	0,3566
1,830	1,0356	1,7672	0,4118
2,012	1,1498	1,7496	0,4526
2,263	1,2986	1,7425	0,5091
2,566	1,4834	1,7299	0,5774
2,881	1,6763	1,7183	0,6481
3,202	1,8742	1,7086	0,7205
3,568	2,0998	1,6994	0,8029
3,952	2,3396	1,6891	0,8892
4,330	2,5822	1,6770	0,9743
4,704	2,8315	1,6614	1,0585
5,040	3,0591	1,6476	1,1340
5,423	3,3132	1,6367	1,2201
5,620	3,5058	1,6031	1,2646
5,536	3,5160	1,5745	1,2455
5,496	3,5163	1,5632	1,2367
5,472	3,5154	1,5566	1,2312
5,457	3,5162	1,5519	1,2278
5,484	3,5359	1,5510	1,2339
5,543	3,5770	1,5496	1,2471
5,595	3,6182	1,5464	1,2589
5,647	3,6591	1,5431	1,2705
5,692	3,6989	1,5389	1,2807
5,738	3,7391	1,5345	1,2910
5,782	3,7796	1,5299	1,3010
5,827	3,8188	1,5259	1,3111
5,873	3,8571	1,5225	1,3213
5,916	3,8966	1,5182	1,3311
5,959	3,9358	1,5141	1,3408
6,004	3,9747	1,5107	1,3510
6,047	4,0129	1,5070	1,3607
6,091	4,0526	1,5030	1,3704
6,130	4,0914	1,4983	1,3793
6,210	4,1545	1,4948	1,3973
6,366	4,2746	1,4892	1,4323
6,507	4,3952	1,4805	1,4641
6,644	4,5155	1,4715	1,4950
6,777	4,6351	1,4620	1,5247

6,907	4,7520	1,4535	1,5540
7,039	4,8730	1,4444	1,5837
7,125	5,0000	1,4251	1,6032
7,259	5,1226	1,4171	1,6333
7,342	5,2530	1,3976	1,6519
7,413	5,3928	1,3747	1,6680
7,541	5,5178	1,3667	1,6968
7,671	5,6416	1,3598	1,7260
7,798	5,7650	1,3527	1,7547
7,925	5,8873	1,3460	1,7830
8,047	6,0077	1,3395	1,8106
8,167	6,1296	1,3325	1,8377
8,284	6,2492	1,3256	1,8639
8,400	6,3666	1,3195	1,8901
8,517	6,4812	1,3142	1,9164
8,634	6,5990	1,3084	1,9427
8,750	6,7174	1,3026	1,9688
8,869	6,8360	1,2975	1,9956
8,987	6,9571	1,2918	2,0221
9,098	7,1045	1,2806	2,0470
9,210	7,2343	1,2732	2,0723
9,237	7,3668	1,2539	2,0784
9,318	7,5012	1,2422	2,0965
9,433	7,6289	1,2364	2,1223
9,528	7,7605	1,2277	2,1438
9,596	7,9012	1,2144	2,1590
9,688	8,0390	1,2051	2,1798
9,798	8,1683	1,1995	2,2045
9,903	8,2964	1,1936	2,2281
9,996	8,4266	1,1862	2,2490
10,050	8,5648	1,1734	2,2613
10,084	8,7120	1,1575	2,2689
10,189	8,8448	1,1519	2,2924
10,283	8,9778	1,1454	2,3137
10,370	9,1111	1,1382	2,3333
10,461	9,2463	1,1314	2,3538
10,556	9,3796	1,1254	2,3751
10,651	9,5124	1,1197	2,3965
10,752	9,6472	1,1145	2,4192
10,855	9,7814	1,1098	2,4424
10,959	9,9145	1,1054	2,4658
11,064	10,0477	1,1011	2,4893
11,163	10,1810	1,0965	2,5118
11,240	10,3071	1,0905	2,5289
11,331	10,4364	1,0857	2,5494
11,433	10,5668	1,0820	2,5724
11,526	10,6989	1,0773	2,5933
11,629	10,8280	1,0739	2,6164
11,728	10,9647	1,0696	2,6388



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11,824	11,1011	1,0651	2,6604	13,908	15,7193	0,8848	3,1294
11,915	11,2363	1,0604	2,6810	13,957	15,8684	0,8795	3,1402
12,006	11,3732	1,0556	2,7014	14,005	16,0193	0,8743	3,1511
12,097	11,5103	1,0510	2,7218	14,050	16,1682	0,8690	3,1612
12,189	11,6527	1,0460	2,7424	14,067	16,3030	0,8628	3,1650
12,278	11,7948	1,0410	2,7626	14,019	16,3446	0,8577	3,1543
12,368	11,9319	1,0365	2,7827	14,042	16,4084	0,8558	3,1594
12,451	12,0706	1,0315	2,8015	14,074	16,4853	0,8537	3,1666
12,524	12,2155	1,0252	2,8178	14,097	16,5643	0,8511	3,1719
12,605	12,3554	1,0202	2,8360	14,117	16,6448	0,8482	3,1764
12,689	12,5116	1,0142	2,8550	14,136	16,7244	0,8452	3,1807
12,772	12,6571	1,0091	2,8737	14,153	16,8030	0,8423	3,1844
12,854	12,7967	1,0045	2,8922	14,168	16,8818	0,8392	3,1877
12,933	12,9355	0,9998	2,9100	14,183	16,9604	0,8363	3,1912
13,011	13,0766	0,9950	2,9276	14,199	17,0414	0,8332	3,1947
13,088	13,2173	0,9902	2,9447	14,215	17,1208	0,8303	3,1983
13,162	13,3595	0,9852	2,9613	14,231	17,2019	0,8273	3,2020
13,231	13,5074	0,9795	2,9769	14,247	17,2823	0,8244	3,2056
13,298	13,6634	0,9733	2,9921	14,261	17,3624	0,8214	3,2087
13,257	13,9386	0,9511	2,9829	14,275	17,4444	0,8183	3,2119
13,257	14,2216	0,9321	2,9827	14,287	17,5280	0,8151	3,2146
13,359	14,3768	0,9292	3,0059	14,296	17,6258	0,8111	3,2166
13,437	14,5281	0,9249	3,0232	14,303	17,7105	0,8076	3,2181
13,501	14,6796	0,9197	3,0377	14,313	17,7947	0,8043	3,2204
13,570	14,8294	0,9150	3,0531	14,325	17,8793	0,8012	3,2232
13,630	14,9778	0,9100	3,0667	14,338	17,9634	0,7982	3,2260
13,684	15,1252	0,9047	3,0788	14,350	18,0458	0,7952	3,2287
13,750	15,2727	0,9003	3,0938	14,362	18,1284	0,7922	3,2314
13,806	15,4208	0,8953	3,1064	14,370	18,2117	0,7891	3,2333
13,855	15,5695	0,8899	3,1174	14,375	18,2902	0,7859	3,2343

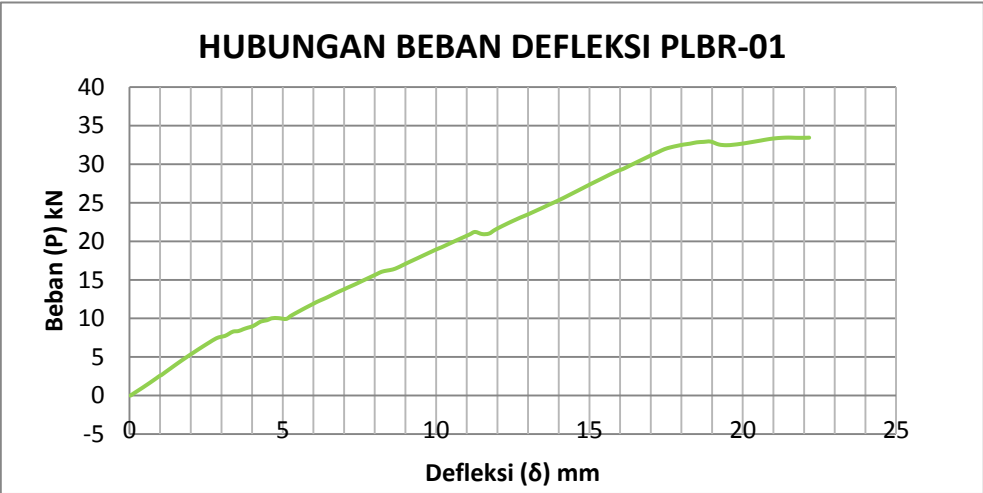
	Beban Retak Pertama
	Kekakuan Pelat
	Beban Max



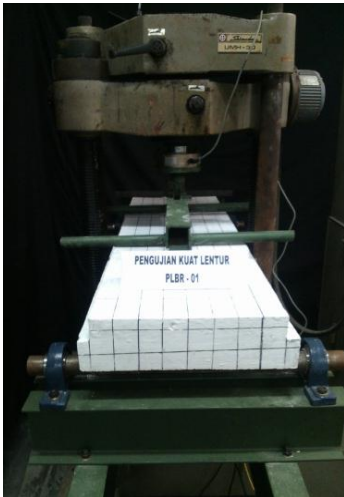
BENDA UJI PLBR-01

Dibuat tanggal : 09 Juni 2017
Diuji tanggal : 25 Juli 2017

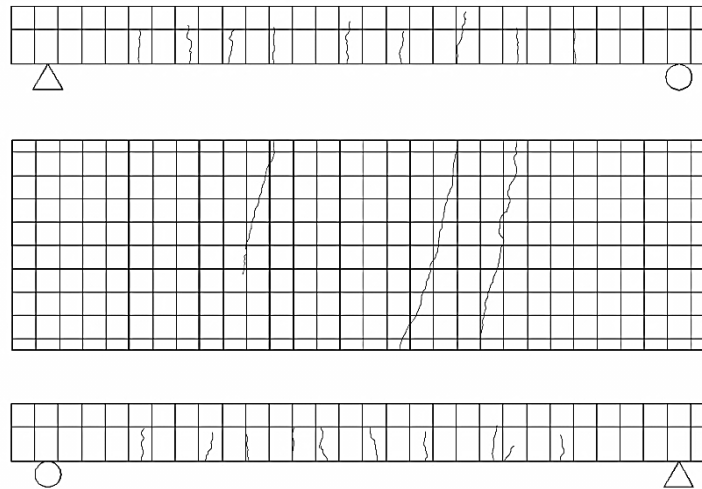
Dimensi : 1500 x 450 x 125 (mm)
Panjang bersih : 1350 mm
Retak pertama : 8,79 kN
Beban max : 33,45 kN
 δ max : 21,50 mm



DOKUMENTASI FOTO



GAMBAR SKETSA RETAK

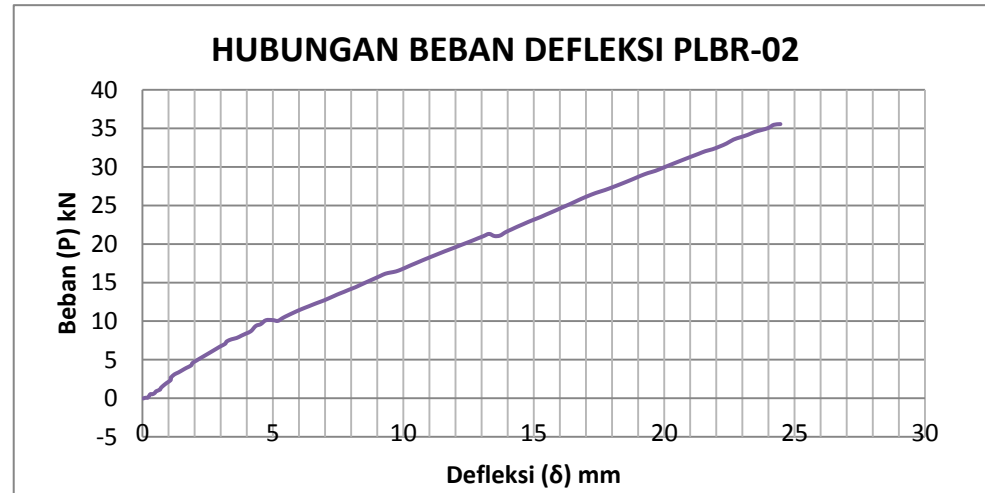




BENDA UJI PLBR-02

Dibuat tanggal : 14 Juni 2017
Diuji tanggal : 26 Juli 2017

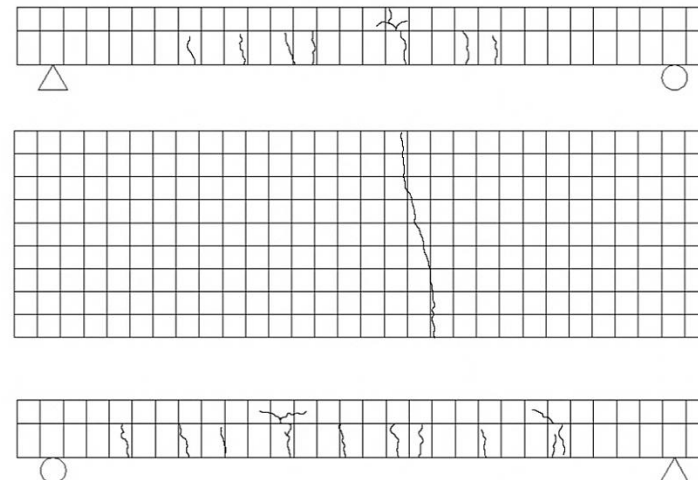
Dimensi : 1500 x 450 x 125 (mm)
Panjang bersih : 1350 mm
Retak pertama : 9,38 kN
Beban max : 35,55 kN
 δ max : 24,46 mm



DOKUMENTASI FOTO



GAMBAR SKETSA RETAK

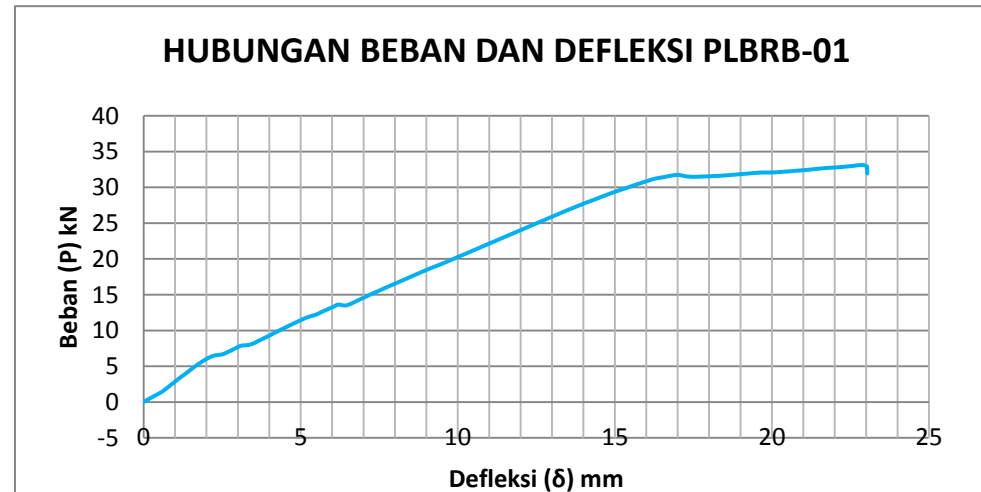



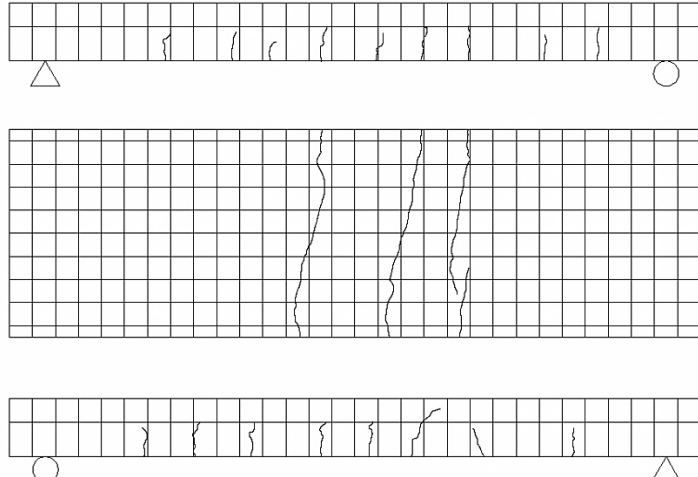


BENDA UJI PLBRB-01

Dibuat tanggal : 13 Juni 2017
Diuji tanggal : 25 Juli 2017

Dimensi : 1500 x 450 x 125 (mm)
Panjang bersih : 1350 mm
Retak pertama : 5,89 kN
Beban max : 33,13 kN
 δ max : 22,88 mm



DOKUMENTASI FOTO	GAMBAR SKETSA RETAK
	

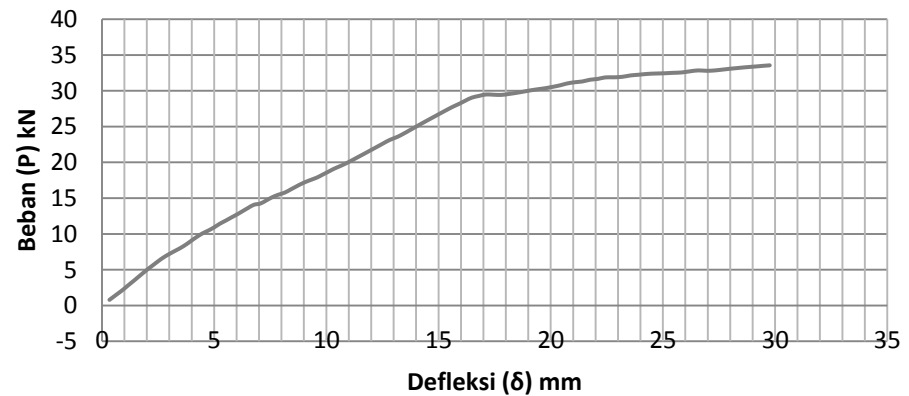


BENDA UJI PLBRB-02

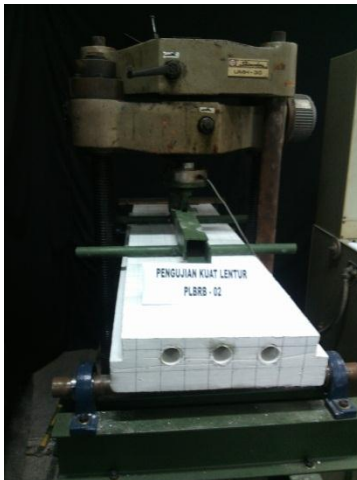
Dibuat tanggal : 20 Juni 2017
Diuji tanggal : 26 Juli 2017

Dimensi : 1500 x 450 x 125 (mm)
Panjang bersih : 1350 mm
Retak pertama : 5,74 kN
Beban max : 33,56 kN
 δ max : 29,76 mm

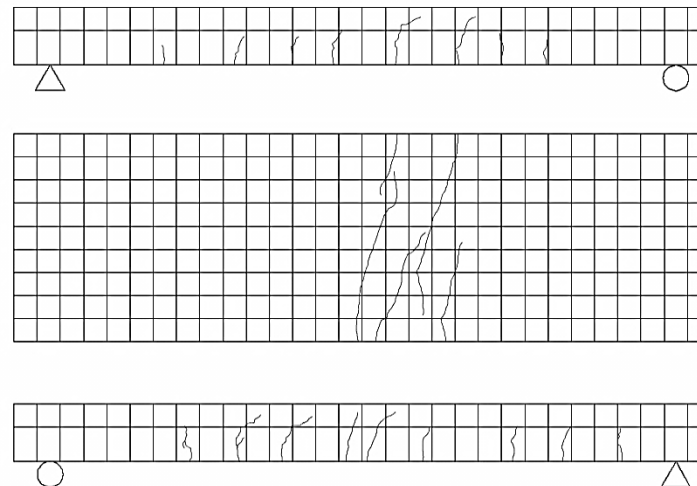
HUBUNGAN BEBAN DAN DEFLEKSI PLBRB-02



DOKUMENTASI FOTO



GAMBAR SKETSA RETAK

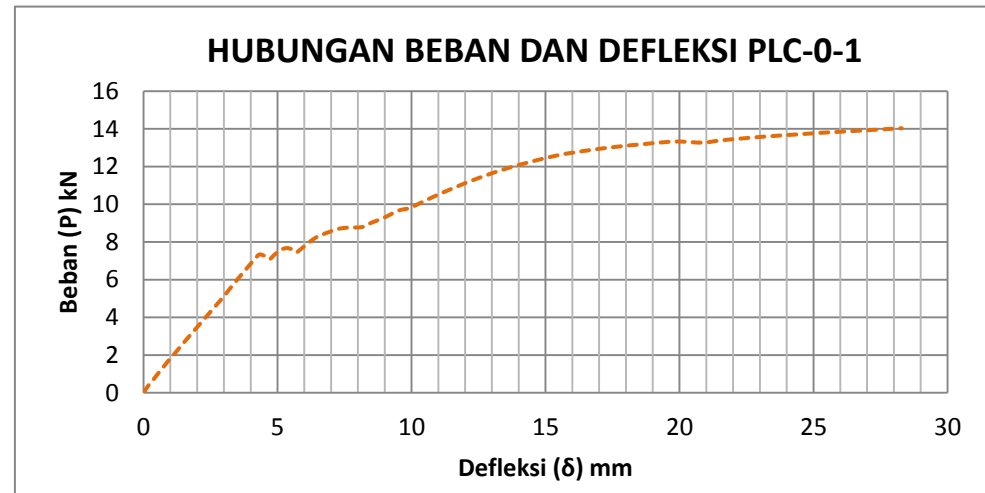




BENDA UJI PLC-01

Dibuat tanggal : -
Diuji tanggal : 27 Juli 2017

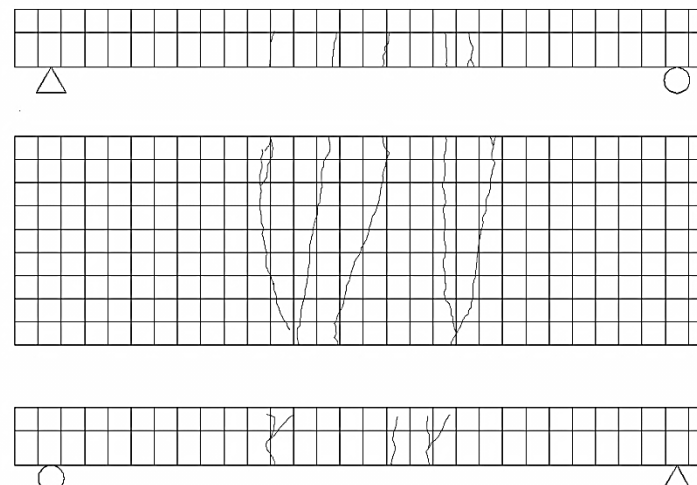
Dimensi 1500 x 450 x 125 (mm)
Panjang bersih 1350 mm
Retak pertama 7,33 kN
Beban max 14,03 kN
 δ max 28,29 mm



DOKUMENTASI FOTO



GAMBAR SKETSA RETAK

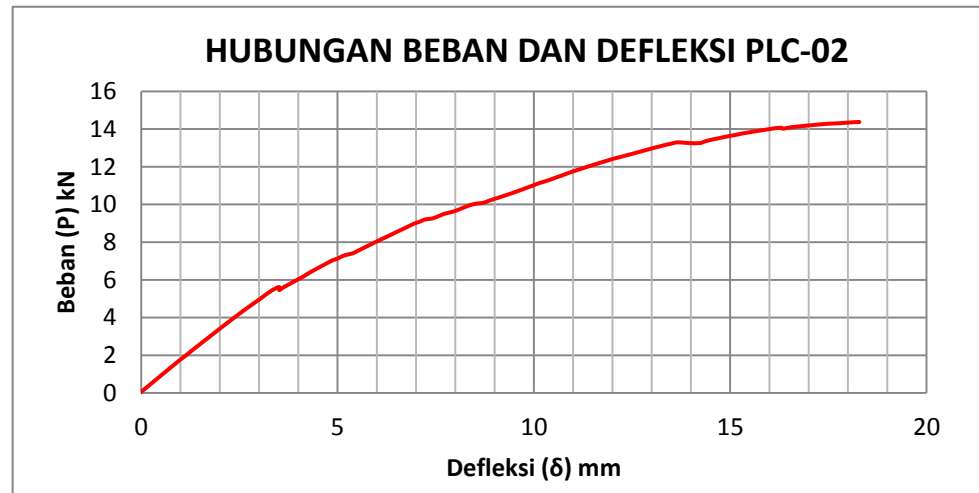




BENDA UJI PLC-02

dibuat tanggal : -
diuji tanggal : 27 Juli 2017

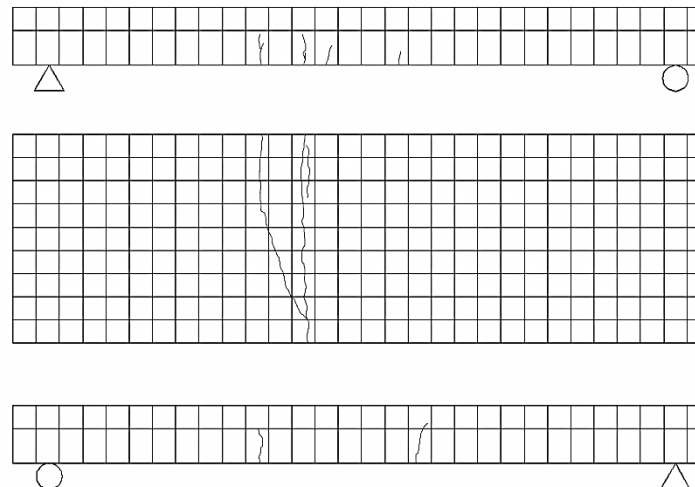
Dimensi 1500 x 450 x 125 (mm)
Panjang bersih 1350 mm
Retak pertama 5,60 kN
Beban max 14,38 kN
 δ max 18,29 mm



DOKUMENTASI FOTO



GAMBAR SKETSA RETAK





LAMPIRAN VII

DOKUMENTASI PENELITIAN





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