

## BAB VI

### KESIMPULAN DAN SARAN

#### 6.1 Kesimpulan

Berdasarkan hasil penelitian yang dilakukan mengenai Pengaruh Penambahan Serat Sabut Kelapa dengan Perlakuan Alkali Terhadap Sifat Mekanik Beton dengan Bahan Tambah *Fly Ash* Sebagai Substitusi Sebagian Semen, dapat diambil beberapa kesimpulan seperti yang tercantum di bawah ini.

1. Nilai kuat tekan beton konvensional, beton konvensional dengan substitusi 15% semen dengan *fly ash*, dan beton serat 0,5% dan 1% dari berat semen tanpa perlakuan alkali berturut-turut adalah 29,07 MPa, 35,68 MPa, 43,15 MPa, 38,23 MPa. Penambahan *fly ash* pada beton konvensional dapat meningkatkan kuat tekan sebesar 18%. Penambahan 0,5% serat sabut kelapa tanpa perlakuan alkali pada beton konvensional dengan substitusi 15% semen dengan *fly ash* dapat meningkatkan kuat tekan sebesar 21,37%. Sedangkan penambahan 1% serat sabut kelapa tanpa perlakuan alkali pada beton konvensional dengan substitusi 15% semen dengan *fly ash* dapat meningkatkan kuat tekan sebesar 6,67%.
2. Nilai kuat tekan beton dengan variasi perlakuan alkali pada beton serat dengan variasi 0M, 1,5M, dan 1,75M untuk beton serat 0,5% secara berturut-turut adalah sebesar 45,38 MPa, 45,67 MPa, dan 43,87 MPa. Sedangkan untuk nilai kuat tekan beton dengan variasi perlakuan alkali 0M, 1,5M, dan 1,75M pada beton serat 1% secara berturut-turut adalah

sebesar 38,23 MPa, 47,89 MPa, dan 39,07 MPa. Nilai kuat tekan yang tertinggi terdapat pada beton dengan kadar 1% serat sabut kelapa dengan perlakuan alkali 1,5 M dengan kuat tekan sebesar 47,89 MPa.

3. Nilai kuat tarik belah beton konvensional, beton konvensional dengan substitusi 15% semen dengan *fly ash*, dan beton serat 0,5% dan 1% dari berat semen tanpa perlakuan alkali berturut-turut adalah 2,04 MPa, 2,77 MPa, 3 MPa, 2,78 MPa. Penambahan *fly ash* pada beton konvensional dapat meningkatkan kuat tarik belah sebesar 26%. Penambahan 0,5% serat sabut kelapa tanpa perlakuan alkali pada beton konvensional dengan substitusi 15% semen dengan *fly ash* dapat meningkatkan kuat tarik belah sebesar 7,67%. Penambahan 1% serat sabut kelapa tanpa perlakuan alkali pada beton konvensional dengan substitusi 15% semen dengan *fly ash* dapat meningkatkan kuat tarik belah sebesar 0,36%.
4. Nilai kuat tarik belah beton dengan variasi perlakuan alkali pada beton serat dengan variasi 0M, 1,5M, dan 1,75M untuk beton serat 0,5% secara berturut-turut adalah sebesar 3 MPa, 3,97 MPa, dan 3,2 MPa. Sedangkan untuk nilai kuat tarik belah beton dengan variasi perlakuan alkali 0M, 1,5M, dan 1,75M pada beton serat 1% secara berturut-turut adalah sebesar 2,78 MPa, 4 MPa, dan 3,15 MPa. Nilai kuat tarik belah yang tertinggi terdapat pada beton dengan kadar 1% serat sabut kelapa dengan perlakuan alkali 1,5 M dengan kuat tekan sebesar 4 MPa.
5. Nilai modulus elastisitas beton konvensional, beton konvensional dengan substitusi 15% semen dengan *fly ash*, beton serat 0,5% dengan perlakuan

alkali 0M, 1,5M dan 1,75M secara berturut-turut adalah sebesar 21.206,81 MPa, 23.866,47 MPa, 26.112,87 MPa, 26.682,45 MPa, dan 23.602,3 MPa. Sedangkan nilai modulus elastisitas beton serat 1% dengan perlakuan alkali 0M, 1,5M dan 1,75M secara berturut-turut adalah sebesar 27.554,83 MPa, 28.690,73 MPa dan 26.412,57 MPa. Hasil modulus elastisitas tertinggi terdapat pada beton dengan 1% serat sabut kelapa dengan perlakuan alkali 1,5M.

6. Secara keseluruhan variasi kadar serat dan variasi kadar perlakuan alkali yang paling optimum terdapat pada kadar serat sebesar 1% dengan kadar alkali 1,5 M. Hal ini terbukti dari hasil penelitian yang menunjukkan bahwa terjadi peningkatan kekuatan beton pada kuat tekan, kuat tarik belah, dan modulus elastisitas.

## 6.2 Saran

Berdasarkan hasil penelitian yang telah diperoleh penulis dapat memberikan beberapa saran yang tercantum di bawah ini.

1. Sebaiknya pengujian kuat lentur (*flexural test*) beton dilakukan untuk dapat memberikan hasil yang lebih maksimal dalam pemanfaatan serat serabut kelapa ini.
2. Perlu dilakukan penelitian selanjutnya dengan jumlah variasi kadar substitusi semen dengan *fly ash* yang berbeda untuk mengetahui kadar substitusi *fly ash* yang paling optimum.

3. Sebaiknya pengetesan kekuatan mekanik pada beton dilakukan bukan hanya pada 28 hari saja namun pada umur 56 hari, karena waktu pengikatan *fly ash* cenderung lebih lama.
4. Perlu dilakukan penelitian selanjutnya dengan menggunakan variasi lama perendaman serat sabut kelapa dengan larutan NaOH.
5. Sebaiknya pada penelitian selanjutnya pencampuran serat sabut kelapa dengan adukan beton dilakukan bersamaan pada saat memasukan adukan beton ke dalam cetakan, agar jumlah serat sabut kelapa sama untuk setiap benda uji, serta untuk menghindari terjadinya penggumpalan serat pada benda uji.

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## **A. Pengujian Bahan**

### **A.1. Pengujian Kandungan Lumpur Pasir**

- I. Waktu pemeriksaan : 5 April 2018
- II. Bahan
- a. Pasir, asal : Kali Progo
  - b. Berat kering : 100 gr
  - c. Air Jernih, asal : LSBB Prodi TS FT – UAJY
- III. Alat
- a. Gelas ukur ukuran : 250 cc
  - b. Timbangan
  - c. Oven
- IV. Hasil
- a. Berat pasir oven : 98,84 gr
  - b. Kandungan lumpur :  $\frac{100 - 98,84}{98,84} \times 100\%$   
: 1,173 %

Kesimpulan : Kandungan lumpur 1,173% < 5%, maka syarat terpenuhi

(OK).

## A.2. Pengujian Zat Organik Pasir

I. Waktu pemeriksaan : 5 April 2018

II. Bahan

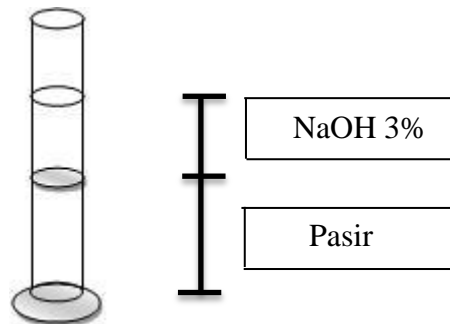
a. Pasir, asal : Kali Progo

b. Larutan NaOH 3%

III. Alat

Gelas ukur ukuran : 250 cc

IV. Sketsa



V. Hasil

Setelah didiamkan selama 24 jam, warna larutan diatas pasir sesuai dengan *Gardner Standart Colour* No. 14.

Kesimpulan : Warna dari pasir *Gardner Standart Colour* No. 14, maka disimpulkan bahwa pasir tersebut Kurang baik digunakan sehingga harus dicuci terlebih dahulu.



### A.3. Pemeriksaan Berat Jenis dan Penyerapan Pasir

- I. Waktu Pemeriksaan : 5 April 2018
- II. Bahan : Pasir
- III. Asal : Kali Progo
- IV. Lokasi Penelitian : Laboratorium Struktur dan Bahan Bangunan (LSBB), Jurusan Teknik Sipil, Universitas Atma Jaya, Yogyakarta
- V. Hasil penelitian

Pengujian Berat Jenis & Penyerapan Pasir		
Berat Awal	500	gram
Berat Kering Oven (A)	494	gram
Berat Labu + Air	714	gram
Berat Labu + Contoh SSD + Air	1034	gram
Berat Jenis Bulk	2,778	gr/cm <sup>3</sup>
Berat Jenis SSD	2,744	gr/cm <sup>3</sup>
Berat Jenis Semu ( <i>Apparent</i> )	2,839	gr/cm <sup>3</sup>
Penyerapan ( <i>Absorption</i> )	1,215	%



#### **A.4. Pemeriksaan Kadar Air Pasir**

- I. Waktu Pemeriksaan : 5 April 2018
- II. Bahan : Pasir
- III. Asal : Kali Progo
- IV. Lokasi Penelitian : Laboratorium Struktur dan Bahan Bangunan (LSBB), Jurusan Teknik Sipil, Universitas Atma Jaya, Yogyakarta
- V. Hasil penelitian

<b>Pengujian Kadar Air Pasir</b>		
Berat Awal	100	gram
Berat Kering Oven	97,69	gram
Kadar air	2,31	%



### A.5. Pengujian Analisis Saringan Pasir

- I. Waktu Pemeriksaan : 6 April 2018
- II. Bahan : Pasir
- III. Asal : Kali Progo
- IV. Lokasi Penelitian : Laboratorium Struktur dan Bahan Bangunan (LSBB), Jurusan Teknik Sipil, Universitas Atma Jaya, Yogyakarta

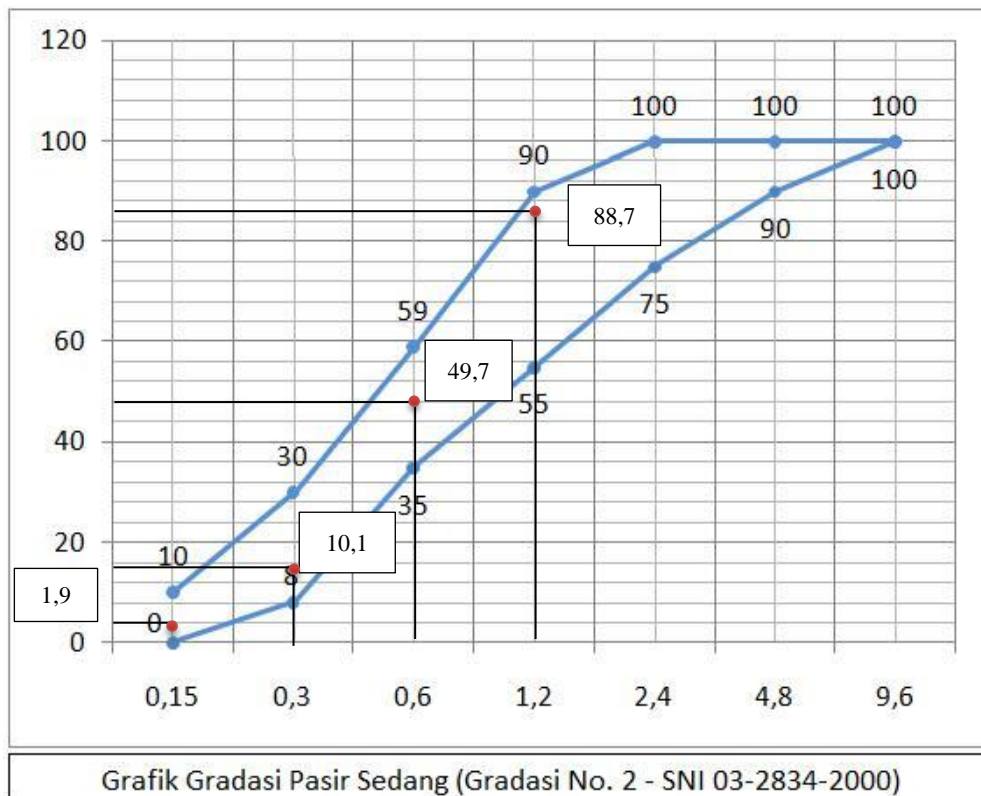
#### V. Hasil penelitian

Ayakan	Berat Saringan	Berat Saringan + Pasir	Berat Pasir	Kumulatif	% Tertahan	% Lolos
3/4"	558	558	0	0	0	100
1/2"	450	450	0	0	0	100
3/8"	456	456	0	0	0	100
No. 4	508	554	46	46	4,6	95,4
No. 8	330	397	67	113	11,3	88,7
No. 30	293	683	390	503	50,3	49,7
No. 50	374	770	396	899	89,9	10,1
No. 100	351	433	82	981	98,1	1,9
Pan	371	390	19	1000	100	0

Kesimpulan : Dari data diatas maka didapat nilai MHB (Modulus Halus Butir) sebesar 3,542. Berdasarkan SK SNI S-04-1989-F (Spesifikasi Bahan Bangunan Bagian A), maka nilai MHB agregat halus tersebut memenuhi syarat karena berada pada kisaran 1,50 – 3,80 (OK).



Berdasarkan data analisis saringan di atas, maka dapat ditentukan untuk daerah golongan pasirnya. Untuk menentukan pasir tersebut termasuk di golongan pasir berapa, dapat dilihat pada grafik di bawah ini.



Setelah angka %lolos saringan dimasukkan ke dalam grafik di atas, maka dapat disimpulkan bahwa agregat halus tersebut termasuk ke dalam pasir golongan 2. Penentuan golongan pasir ini digunakan untuk perencanaan *mix design*.



#### **A.6. Pengujian Kandungan Lumpur Agregat Kasar**

- I. Waktu pemeriksaan : 5 April 2018
- II. Bahan
- a. Pasir, asal : Kali Clereng
  - b. Berat kering : 100 gr
  - c. Air Jernih, asal : LSBB Prodi TS FT – UAJY
- III. Alat
- a. Gelas ukur ukuran : 250 cc
  - b. Timbangan
  - c. Oven
- IV. Hasil
- a. Berat pasir oven : 91,91 gr
  - b. Kandungan lumpur :  $\frac{100-91,91}{91,91} \times 100\%$   
: 8,09 %

Kesimpulan : Kandungan lumpur 8,09 % > 5%, maka syarat tidak terpenuhi.



### **A.7. Pemeriksaan Kadar Air Agregat Kasar**

- I. Waktu Pemeriksaan : 5 April 2018
- II. Bahan : Kerikil
- III. Asal : Kali Clereng
- IV. Lokasi Penelitian : Laboratorium Struktur dan Bahan Bangunan (LSBB), Jurusan Teknik Sipil, Universitas Atma Jaya, Yogyakarta
- V. Hasil penelitian

<b>Pengujian Kadar Air Agregat Kasar</b>		
Berat Awal	100	gram
Berat Kering Oven	99,39	gram
Kadar air	0,61	%





## A.8. Pengujian Analisis Saringan Agregat Kasar

- I. Waktu Pemeriksaan : 6 April 2018
- II. Bahan : Kerikil
- III. Asal : Kali Clereng
- IV. Lokasi Penelitian : Laboratorium Struktur dan Bahan Bangunan (LSBB), Jurusan Teknik Sipil, Universitas Atma Jaya, Yogyakarta

### V. Hasil penelitian

Ayakan	Berat Saringan	Berat Saringan + Split	Berat Split	Kumulatif	% Tertahan	% Lolos
3/4"	558	574	16	16	1,6	98,4
1/2"	450	534	84	100	10	90
3/8"	456	509	53	153	15,3	84,7
No. 4	508	1296	788	941	94,1	5,9
No. 8	331	331	0	941	94,1	5,9
No. 30	292	292	0	941	94,1	5,9
No. 50	374	375	1	942	94,2	5,8
No. 100	351	354	3	945	94,5	5,5
No. 200	269	320	51	996	99,6	0,4
Pan	371	375	4	1000	100	0

Kesimpulan : Dari data diatas maka didapat nilai MHB (Modulus Halus Butir) sebesar 6,975. Berdasarkan SK SNI S-04-1989-F (Spesifikasi Bahan Bangunan Bagian A), maka nilai MHB agregat kasar tersebut memenuhi syarat karena berada pada kisaran 6,00-7,10 (OK).



### **A.9. Pemeriksaan Berat Jenis dan Penyerapan Agregat Kasar**

- I. Waktu Pemeriksaan : 5 April 2018
- II. Bahan : Kerikil
- III. Asal : Kali Clereng
- IV. Lokasi Penelitian : Laboratorium Struktur dan Bahan Bangunan (LSBB), Jurusan Teknik Sipil, Universitas Atma Jaya, Yogyakarta
- V. Hasil penelitian

<b>Pengujian Berat Jenis &amp; Penyerapan Agregat Halus</b>		
Berat Contoh Kering (A)	2000	gram
Berat Contoh Jenuh Kering Permukaan (B)	2037,55	gram
Berat Setelah Keluar Dari Oven (C)	1235,48	gram
Berat Jenis Bulk	2,49	gr/cm <sup>3</sup>
Berat Jenis SSD	2,54	gr/cm <sup>3</sup>
Berat Jenis Semu ( <i>Apparent</i> )	2,62	gr/cm <sup>3</sup>
Penyerapan ( <i>Absorption</i> )	1,88	%



#### **A.10. Pemeriksaan Abrasi dan Keausan**

- I. Waktu Pemeriksaan : 5 April 2018
- II. Bahan : Kerikil
- III. Asal : Kali Clereng
- IV. Lokasi Penelitian : Laboratorium Transportasi,  
Jurusan Teknik Sipil,  
Universitas Atma Jaya,  
Yogyakarta

#### **V. Hasil penelitian**

Gradasi Saringan		Nomor contoh
		I
Lolos	Tertahan	Berat masing-masing agregat
3/4"	1/2"	2500
1/2"	3/8"	2500

Nomor Contoh	I
Berat sebelumnya (A)	5000 gram
Berat sesudah diayak saringan No.12 (B)	3601 gram
Berat sesudah (A) - (B)	1399 gram
Keausan	27,98%



### A.11. Pengujian *Fly Ash*

Measurement Condition

Instrument: EDX-8000    Atmosphere: Vac.    Collimator: 10(mm)    Sample Cup: Mylar

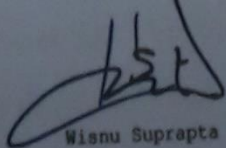
Analyte    TG kV    uA    FI    Acq. (keV)    Anal. (keV)    Time (sec)    DT(%)

Na-U    Rh 50    27-Auto    ----    0 - 40    0.00-40.00    Live- 100    40

Quantitative Result

Analyte	Result	[3-sigma] Proc.-Calc.	Line	Int. (cps/uA)
	44.257 %	[0.343] Quan-FP	SiKa	52.8759
SiO2	21.725 %	[0.331] Quan-FP	AlKa	11.7461
Al2O3	12.264 %	[0.019] Quan-FP	FeKa	1461.8018
Fe2O3	9.199 %	[0.041] Quan-FP	CaKa	167.0568
CaO	8.876 %	[0.948] Quan-FP	MgKa	1.2379
MgO	1.674 %	[0.028] Quan-FP	S Ka	6.9684
S03	0.780 %	[0.013] Quan-FP	K Ka	9.5263
K2O	0.748 %	[0.008] Quan-FP	TiKa	21.8519
TiO2	0.189 %	[0.002] Quan-FP	MnKa	18.3949
MnO	0.106 %	[0.018] Quan-FP	ErLa	6.2146
Er2O3	0.060 %	[0.001] Quan-FP	SrKa	31.3838
SrO	0.051 %	[0.005] Quan-FP	V Ka	2.0242
V2O5	0.017 %	[0.001] Quan-FP	ZnKa	3.6818
ZnO	0.016 %	[0.001] Quan-FP	ZrKa	8.2297
ZrO2	0.014 %	[0.003] Quan-FP	CrKa	0.9716
Cr2O3	0.010 %	[0.002] Quan-FP	PbLb1	1.6676
PbO	0.008 %	[0.002] Quan-FP	NiKa	1.1787
NiO	0.004 %	[0.001] Quan-FP	Y Ka	2.2132
Y2O3				

Operator EDX

  
Wisnu Suprapta



## **B. Perencanaan Adukan Beton (SNI 03-2834-2000)**

### **I. Data bahan**

1. Agregat halus : Kali Progo, Yogyakarta
2. Agregat kasar : Kali Clereng, Yogyakarta
3. Semen : Gresik

### **II. Hitungan *Mix Design***

1. Kuat tekan yang direncanakan ( $f'c$ ) pada umur 28 hari ialah 25 MPa.
2. Menentukan nilai deviasi standar berdasarkan tingkat mutu pengendalian pelaksanaan campuran beton ( $Sd = 2,8$ ).
3. Berdasarkan SNI, nilai margin didapat dari  $1,68 \times 2,8 = 4,705 \sim 5$  MPa.
4. Menetapkan kuat tekan beton rata-rata sesuai dengan SNI.

$$f'cr = f'c + M = 25 + 5 = 30 \text{ MPa}$$

5. Digunakan semen tipe I merk Gresik.
6. Menetapkan jenis agregat
  1. Agregat halus : Pasir alam
  2. Agregat kasar : Batu pecah
7. Menentukan factor air semen, berdasarkan jenis semen yang dipakai dan kuat tekan rata-rata silinder beton yang direncanakan pada umur tertentu. Berdasarkan titik kekuatan tekan beton yang dirancang (dalam hal ini 30 MPa) tarik garis datar hingga memotong kurva garis 28 hari. Dari titik potong ini tarik garis tegak ke bawah hingga

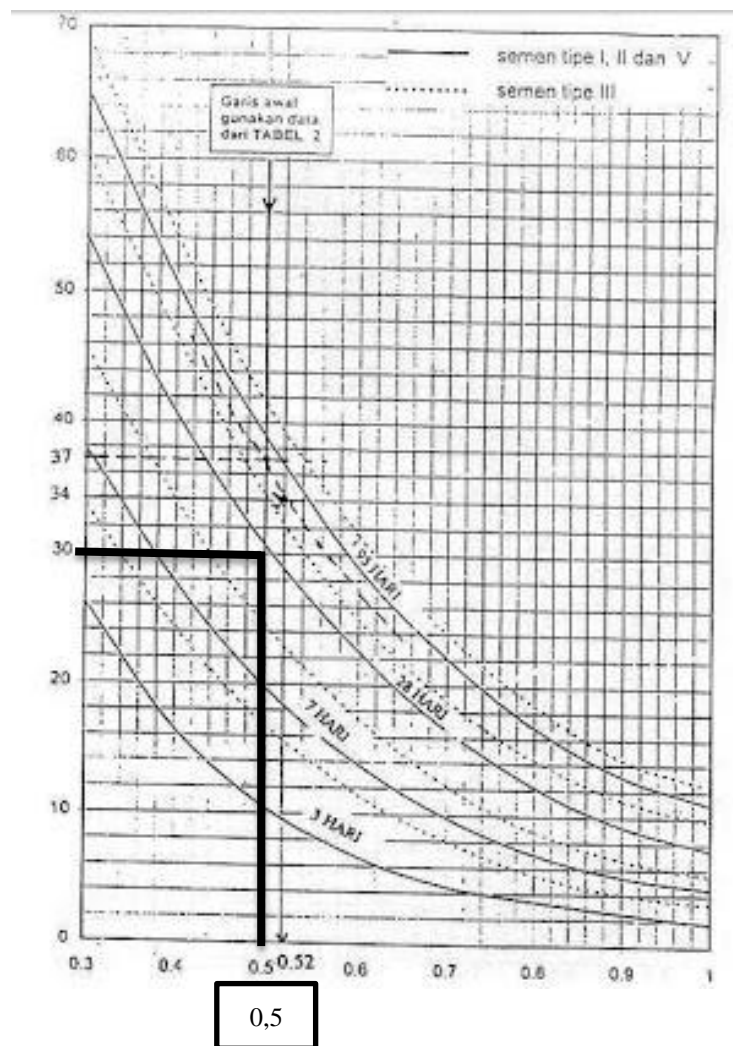


memotong sumbu X dan dibaca faktor air semen yang diperoleh.

Didapatkan sebesar 0,5.

### Hubungan Kuat Tekan Silinder dengan Fas

(Sumber : SNI 03-2834-2000 : Grafik 1)





8. Mencari nilai fas dari tabel

**Persyaratan Jumlah Semen Minimum dan Faktor Air Semen Maksimum Untuk Berbagai Macam Pembetonan dalam Lingkungan Kusus**

(Sumber : SNI 03-2834-2000 : Tabel 4)

Lokasi	Jumlah Semen minimum Per m <sup>3</sup> beton (kg)	Nilai Faktor Air Semen Maksimum
Beton di dalam ruang bangunan :		
a. Keadaan keliling non-korosif	275	0,6
b. Keadaan keliling korosif disebabkan oleh kondensasi atau uap korosif	325	0,52
Beton diluar ruangan bangunan :		
a. tidak terlindung dari hujan dan terik matahari langsung	325	0,60
b. terlindung dari hujan dan terik matahari langsung	275	0,60
Beton masuk kedalam tanah :		
a. mengalami keadaan basah dan kering berganti-ganti	325	0,55
b. mendapat pengaruh sulfat dan alkali dari tanah		Lihat Tabel 5
Beton yang kontinu berhubungan:		
a. Air tawar		
b. Air laut		Lihat Tabel 6

Berdasarkan tabel 4 SNI 03-2834-2000, untuk beton dalam ruang bangunan sekeliling non-korosif fas maksimum 0,6. Dibandingkan dengan poin 8, dipakai fas yang terkecil. Jadi digunakan fas 0,5.

9. Menetapkan nilai *slump*

Digunakan nilai *slump* dengan nilai minimal 6 cm dan maksimal 18 cm

10. Ukuran butir maksimum kerikil 20 mm.

11. Mencari jumlah air yang dibutuhkan untuk tiap m<sup>3</sup> beton.



**Perkiraan Kadar Air Bebas (kg/m<sup>3</sup>) yang Dibutuhkan  
Untuk Beberapa Tingkat Kemudahan  
Pengerjaan Adukan Beton**

Ukuran Agregat Maksimum (mm)	Jenis Batu	<i>Slump</i>			
		0-10	10-30	30-60	60-180
10	Alami	150	180	205	225
	Batu pecah	180	205	230	250
20	Alami	135	160	180	195
	Batu pecah	170	190	210	225
40	Alami	115	140	160	175
	Batu pecah	155	175	190	205

(Sumber : SNI 03-2834-2000 : Tabel 3)

- Ukuran butir maksimum 20 mm.
- Nilai *Slump* 60-180 mm.
- Agregat halus berupa batu tak di pecah, maka  $W_h = 195$
- Agregat kasar berupa batu pecah, maka  $W_k = 225$

$$W = \frac{2}{3}W_h + \frac{1}{3}W_k$$

Dengan :  $W_h$  adalah perkiraan jumlah air untuk agregat halus  $W_k$  adalah perkiraan jumlah air untuk agregat kasar.

$$W = \frac{2}{3} \times 195 + \frac{1}{3} \times 225 = 205 \text{ lt/m}^3$$

- Menghitung berat semen yang diperlukan.
  - Berdasarkan tabel 4 SNI 03-2834-2000, diperoleh semen minimum 275 kg.





b. Berdasarkan  $f_{as} = 0,5$ . Semen per  $m^3$  beton =  $\frac{air}{f_{as}} = \frac{205}{0,5} =$

410 kg.

Berdasarkan data 1 dan 2, dipilih jumlah semen yang terbesar,  
maka diambil jumlah semen = 410 kg.

13. Penyesuaian fas

fas rencana = 0,5

fas mak > fas rencana

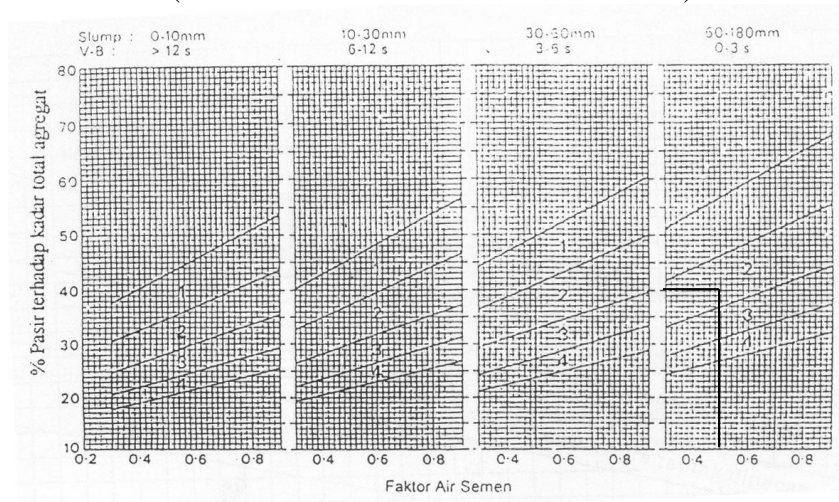
0,6 > 0,5 (Ok)

14. Menghitung perbandingan agregat halus dan kasar

**Persen Pasir Terhadap Kadar Total Agregat yang Dianjurkan**

**Untuk Ukuran Butir Maksimum 20 mm**

(Sumber : SNI 03-2834-2000 : Tabel 13)



a. Ukuran maksimum 20 mm.

b. Nilai *Slump* 60 mm – 180 mm

c.  $f_{as}$  0,5.

d. Jenis gradasi pasir no. 2. Diambil proporsi pasir = 40%.

15. Menghitung berat jenis agregat campuran.



$$\begin{aligned} &= \frac{P}{100} \times bj \text{ agregat halus} + \frac{K}{100} \times bj \text{ agregat kasar} \\ &= \frac{42}{100} \times 2,774 + \frac{58}{100} \times 2,543 \\ &= 2,632 \text{ gr/cm}^3 \end{aligned}$$

Dimana:

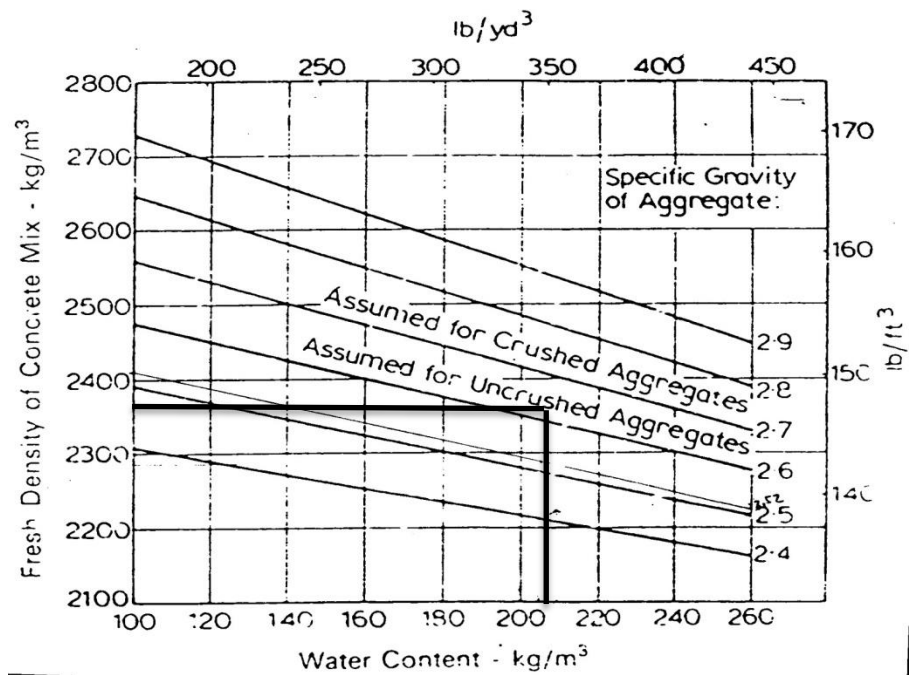
P = % agregat halus terhadap agregat campuran

K = % agregat kasar terhadap agregat campuran

#### 16. Mencari berat jenis beton

##### Perkiraan Berat Isi Beton yang Telah Selesai Didapatkan

(Sumber : SNI 03-2834-2000 : Grafik 16)



di dapat 2350 kg/m<sup>3</sup>.

#### 17. Berat agregat campuran

= berat tiap m<sup>3</sup> – keperluan air dan semen

$$= 2350 - (205 + 410)$$

$$= 1735 \text{ kg/m}^3$$



18. Menghitung berat agregat halus

$$\begin{aligned}\text{Berat agregat halus} &= \% \text{ agregat halus} \times \text{keperluan agregat} \\ &\quad \text{campuran} \\ &= 40\% \times 1735 \\ &= 694 \text{ kg/m}^3\end{aligned}$$

19. Menghitung berat agregat kasar

$$\begin{aligned}\text{Berat agregat kasar} &= \% \text{ agregat kasar} \times \text{keperluan agregat} \\ &\quad \text{campuran} \\ &= 60\% \times 1735 \\ &= 1041 \text{ kg/m}^3\end{aligned}$$

$$\begin{aligned}20. \text{ Volume silinder} &= \frac{1}{4} \times \pi \times D^2 \times T \\ &= \frac{1}{4} \times \pi \times 0,15^2 \times 0,3 \\ &= 0,0053\end{aligned}$$

III. Hasil *Mix Design*

Tabel Proporsi Campuran Adukan per m<sup>3</sup>

Kode	Semen	Pasir	Split	Serat	<i>Fly Ash</i>	<i>Superplasticizer</i>	Air
	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)
BK	410	694	1041	-	-	-	205
BKFA	348,5	694	1041	-	61,5	-	205
BS-0,5% 0M	348,5	694	1041	2,05	61,5	4,1	205
BS-1% 0M	348,5	694	1041	4,1	61,5	4,1	205
BS-0,5% 1,5M	348,5	694	1041	2,05	61,5	4,1	205
BS-1% 1,5M	348,5	694	1041	4,1	61,5	4,1	205
BS-0,5% 1,75M	348,5	694	1041	2,05	61,5	4,1	205
BS-1% 1,75M	348,5	694	1041	4,1	61,5	4,1	205



Tabel Proporsi untuk 1 Siinder

Kode	Semen	Pasir	Split	Serat	Fly Ash	Superplasticizer	Air
	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)
BK	2,174	3,681	5,521	-	-	-	1,087
BKFA	1,848	3,681	5,521	-	0,326	-	1,087
BS-0,5% 0M	1,848	3,681	5,521	0,011	0,326	0,022	1,087
BS-1% 0M	1,848	3,681	5,521	0,022	0,326	0,022	1,087
BS-0,5% 1,5M	1,848	3,681	5,521	0,011	0,326	0,022	1,087
BS-1% 1,5M	1,848	3,681	5,521	0,022	0,326	0,022	1,087
BS-0,5% 1,75M	1,848	3,681	5,521	0,011	0,326	0,022	1,087
BS-1% 1,75M	1,848	3,681	5,521	0,022	0,326	0,022	1,087

Tabel Proporsi per 1 Kali Adukan (6 Silinder)

Kode	Semen	Pasir	Split	Serat	Fly Ash	Superplasticizer	Air
	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)
BK	13,047	22,084	33,126	-	-	-	6,523
BKFA	11,090	22,084	33,126	-	1,957	-	6,523
BS-0,5% 0M	11,090	22,084	33,126	0,065	1,957	0,130	6,523
BS-1% 0M	11,090	22,084	33,126	0,130	1,957	0,130	6,523
BS-0,5% 1,5M	11,090	22,084	33,126	0,065	1,957	0,130	6,523
BS-1% 1,5M	11,090	22,084	33,126	0,130	1,957	0,130	6,523
BS-0,5% 1,75M	11,090	22,084	33,126	0,065	1,957	0,130	6,523
BS-1% 1,75M	11,090	22,084	33,126	0,130	1,957	0,130	6,523

Tabel Proporsi per 1 Kali Adukan (6 Silinder) dengan SF = 1,25

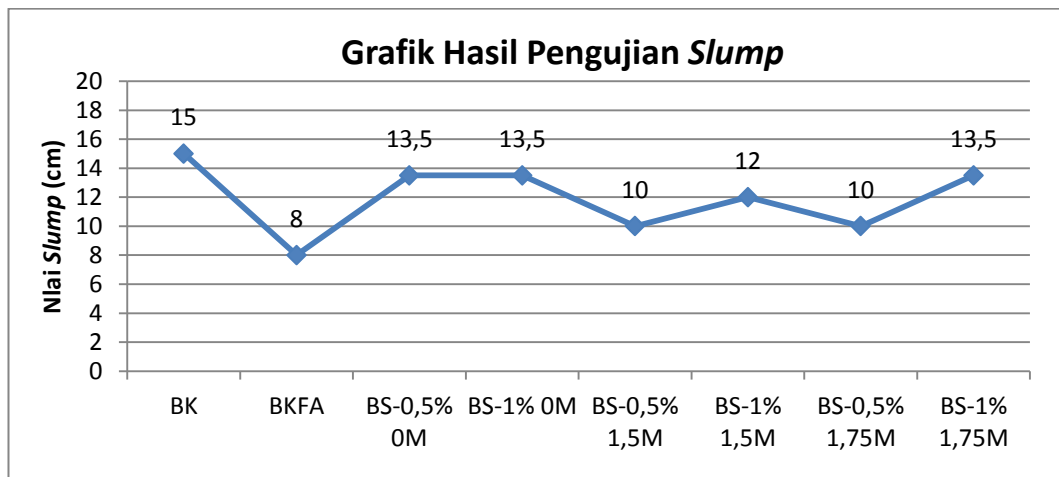
Kode	Semen	Pasir	Split	Serat	Fly Ash	Superplasticizer	Air
	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)	(Kg)
BK	16,308	27,605	41,408	-	-	-	8,154
BKFA	13,862	27,605	41,408	-	2,446	-	8,154
BS-0,5% 0M	13,862	27,605	41,408	0,082	2,446	0,163	8,154
BS-1% 0M	13,862	27,605	41,408	0,163	2,446	0,163	8,154
BS-0,5% 1,5M	13,862	27,605	41,408	0,082	2,446	0,163	8,154
BS-1% 1,5M	13,862	27,605	41,408	0,163	2,446	0,163	8,154
BS-0,5% 1,75M	13,862	27,605	41,408	0,082	2,446	0,163	8,154
BS-1% 1,75M	13,862	27,605	41,408	0,163	2,446	0,163	8,154



## C. Pengujian Beton

### C.1. Pengujian *Slump*

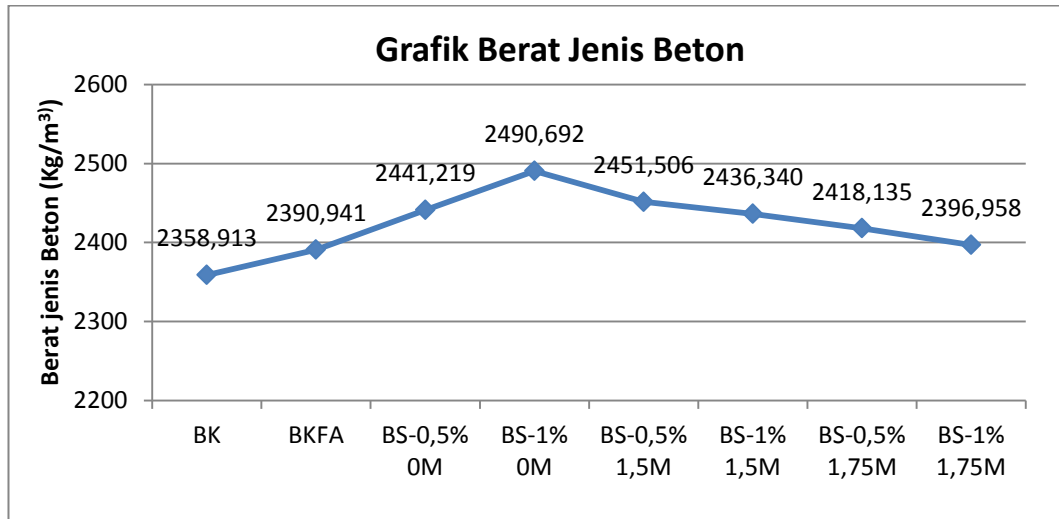
Kode	Nilai <i>Slump</i>
	(cm)
BK	15
BKFA	8
BS-0,5% 0M	13,5
BS-1% 0M	13,5
BS-0,5% 1,5M	10
BS-1% 1,5M	12
BS-0,5% 1,75M	10
BS-1% 1,75M	13,5





### C.2. Pengujian Berat Jenis Beton

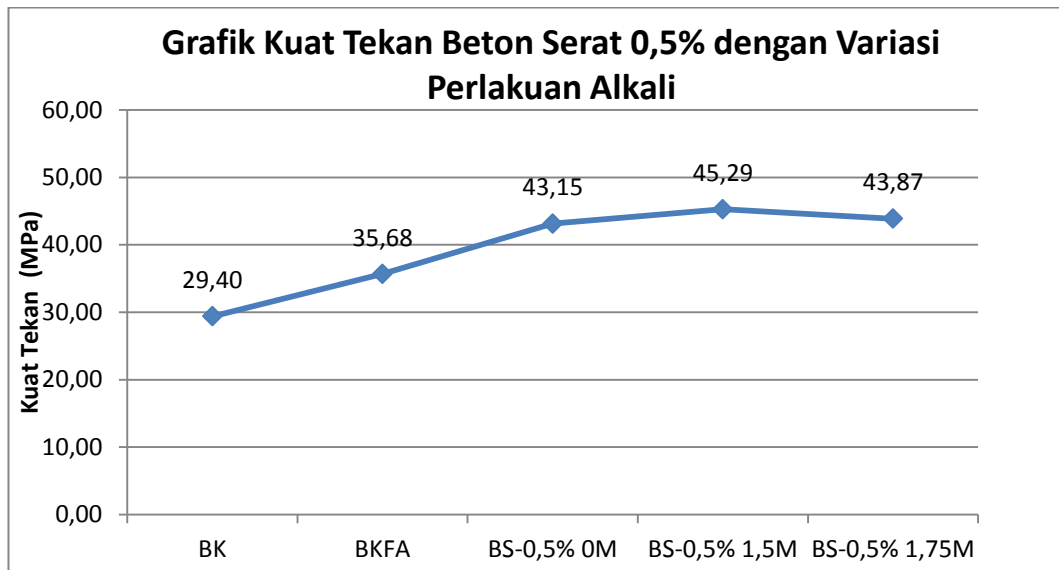
Kode	No.	Berat	Dimensi		Berat Jenis	Berat Volume Rata-rata
		(Kg)	D (cm)	T (cm)	Kg/m <sup>3</sup>	Kg/m <sup>3</sup>
BK	c	12,62	15,063	29,95	2363,762	2358,913
	e	12,54	15,15	30,08	2311,691	
	d	13,54	15,4	30,26	2401,285	
BKFA	e	12,86	15,025	30,3	2392,791	2390,941
	d	12,8	15,04	30,17	2387,120	
	b	12,78	15,025	30,11	2392,911	
BS-0,5% 0M	a	13,84	15,55	29,9	2436,349	2441,219
	b	13,9	15,46	30,3	2442,804	
	d	13,8	15,45	30,1	2444,505	
BS-1% 0M	a	13,1	15,06	30	2450,391	2490,692
	b	13,18	15,25	30,175	2390,363	
	d	13,8	15,06	29,43	2631,323	
BS-0,5% 1,5M	a	12,98	15	29,95	2451,493	2451,506
	b	13,08	15,03	30,08	2449,894	
	f	12,98	15	29,93	2453,131	
BS-1% 1,5M	b	12,98	15,1	29,88	2424,798	2436,340
	e	13,76	15,43	30,23	2433,233	
	f	13,72	15,475	29,75	2450,988	
BS-0,5% 1,75M	a	13,06	15,1	30,575	2384,285	2418,135
	d	13,98	15,53	30,13	2448,502	
	f	12,92	15,08	29,86	2421,617	
BS-1% 1,75M	c	13,7	15,48	30,35	2397,482	2396,958
	e	12,96	15,075	30,18	2404,952	
	f	12,92	15,13	30,075	2388,440	



### C.3. Pengujian Kuat Tekan Beton

#### C.3.1. Kadar Serat Sabut Kelapa 0,5%

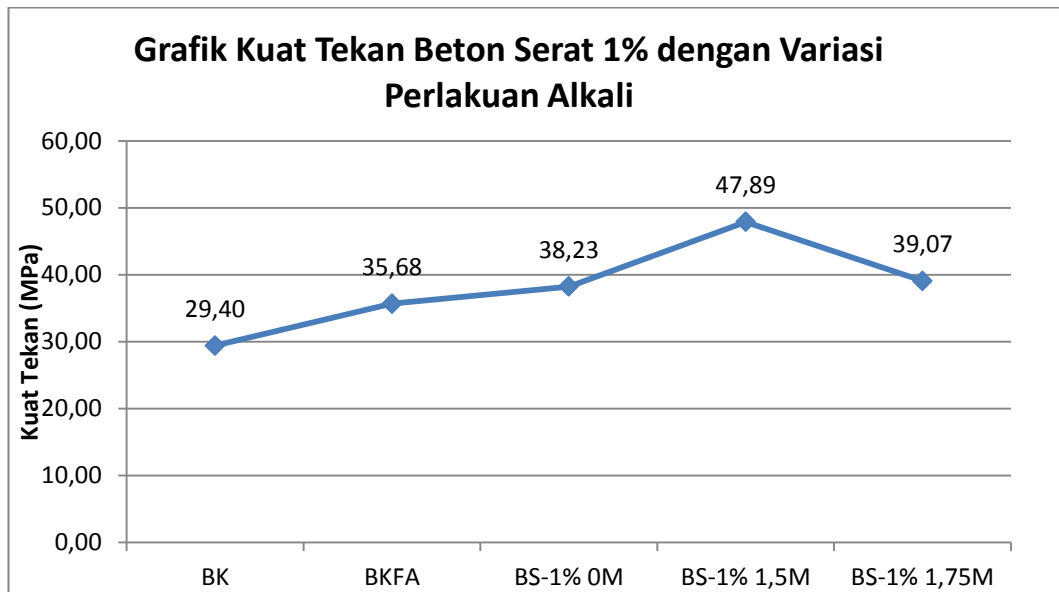
Kode	No.	Dimensi		Beban Max (KN)	Kuat Tekan (MPa)	Kuat Tekan Rerata (MPa)	Kenaikan
		D (cm)	T (cm)				
BK	c	15,0625	29,95	560	31,41	29,40	-
	e	15,15	30,08	550	30,50		
	d	15,4	30,26	490	26,30		
BKFA	e	15,025	30,3	550	31,01	35,68	18%
	d	15,04	30,17	700	39,39		
	b	15,025	30,11	650	36,65		
BS-0,5% 0M	a	15,55	29,9	810	42,63	43,15	35%
	b	15,46	30,3	820	43,66		
	d	15,45	30,1	955	50,92*		
BS-0,5% 1,5M	a	15	29,95	800	45,25	45,29	36%
	b	15,03	30,08	760	42,82		
	f	15	29,93	845	47,80		
BS-0,5% 1,75M	a	15,1	30,575	845	47,17	43,87	33%
	d	15,53	30,13	570	30,08*		
	f	15,08	29,86	725	40,58		



### C.3.2. Kadar Serat Sabut Kelapa 1%

Kode	No.	Dimensi		Beban Max (KN)	Kuat Tekan (MPa)	Kuat Tekan Rerata (MPa)	Kenaikan
		D (cm)	T (cm)				
BK	c	15,0625	29,95	560	31,41	29,40	-
	e	15,15	30,08	550	30,50		
	d	15,4	30,26	490	26,30		
BKFA	e	15,025	30,3	550	31,01	35,68	18%
	d	15,04	30,17	700	39,39		
	b	15,025	30,11	650	36,65		
BS-1% 0M	a	15,06	30	670	37,60	38,23	23%
	b	15,25	30,175	710	38,86		
	d	15,06	29,43	925	51,91*		
BS-1% 1,5M	b	15,1	29,88	850	47,45	47,89	39%
	e	15,43	30,23	955	51,05		
	f	15,475	29,75	850	45,17		
BS-1% 1,75M	c	15,48	30,35	770	40,90	39,07	25%
	e	15,075	30,18	500	28,00*		
	f	15,13	30,075	670	37,25		

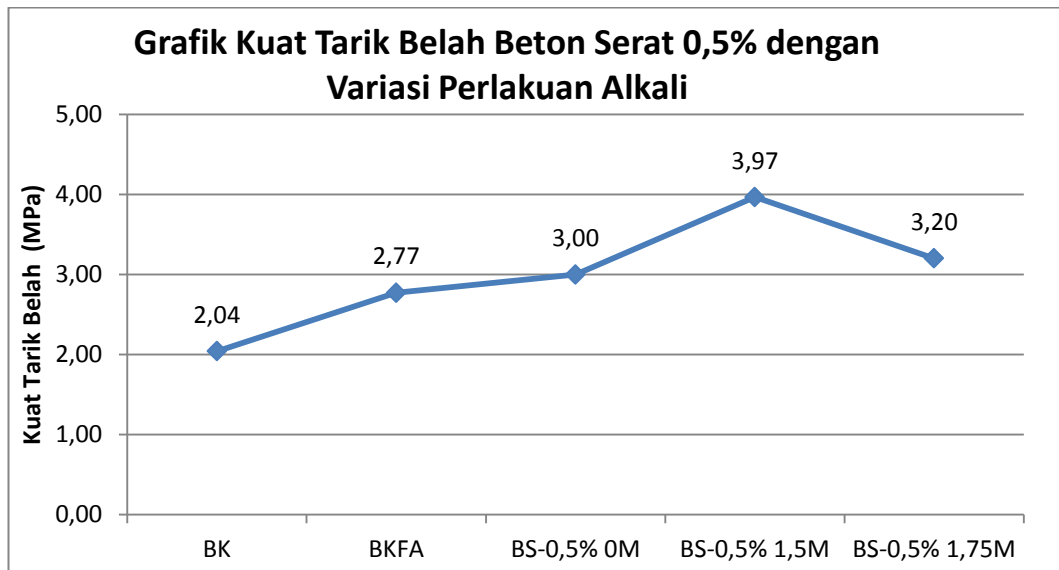




#### C.4. Pengujian Kuat Tarik Belah Beton

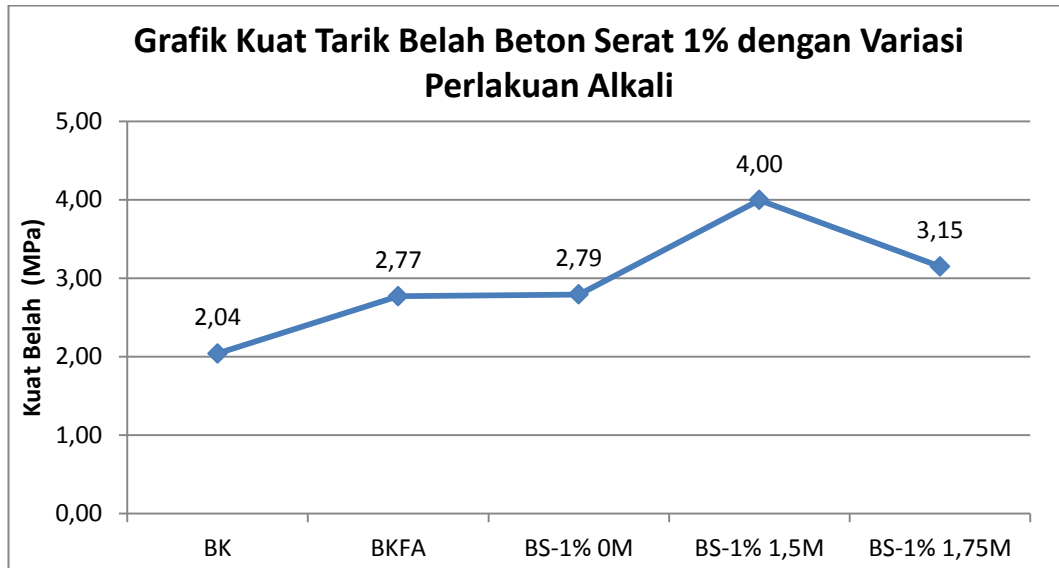
##### C.4.1. Kadar Serat Sabut Kelapa 0,5%

Kode	No.	Dimensi		Beban Max (KN)	Kuat Tarik (MPa)	Kuat Tarik Rerata (MPa)	Kenaikan
		D (cm)	T (cm)				
BK	a	15,15	30	170	2,38	2,04	-
	b	15,5	30,27	130	1,76		
	f	15,2	29,6	140	1,98		
BKFA	a	15,2	30,6	200	2,74	2,77	26%
	c	15,5	29,83	220	3,03		
	f	15,45	30,75	190	2,54		
BS-0,5% 0M	c	15,42	30,34	200	2,72	3,00	32%
	e	15,45	30,18	240	3,28		
	f	15,45	30,26	195	2,65		
BS-0,5% 1,5M	c	15,08	30,09	340	4,77	3,97	49%
	d	15	30,13	260	3,66		
	e	15,18	30,2	250	3,47		
BS-0,5% 1,75M	b	15,18	30,52	200	2,75	3,20	36%
	c	15,545	30,35	200	2,70		
	e	15,16	30,3	300	4,16		



#### C.4.2. Kadar Serat Sabut Kelapa 1%

Kode	No.	Dimensi		Beban Max (KN)	Kuat Tarik (MPa)	Kuat Tarik Rerata (MPa)	Kenaikan
		D (cm)	T (cm)				
BK	a	15,15	30	170	2,38	2,04	-
	b	15,5	30,27	130	1,76		
	f	15,2	29,6	140	1,98		
BKFA	a	15,2	30,6	200	2,74	2,77	26%
	c	15,5	29,83	220	3,03		
	f	15,45	30,75	190	2,54		
BS-1% 0M	c	15,29	30,53	230	3,14	2,79	27%
	e	15,16	30,25	180	2,50		
	f	15,2	30,46	200	2,75		
BS-1% 1,5M	a	15,025	30,045	260	3,67	4,00	49%
	c	15	30,025	310	4,38		
	d	15,025	30,075	280	3,94		
BS-1% 1,75M	a	15,38	30,23	255	3,49	3,15	35%
	b	15,61	30,33	210	2,82		
	d	15,26	30,58	205	2,80		





### C.5. Pengujian Modulus Elastisitas Beton

Kode Beton = BK c

Po = 202,7 mm

Ao = 17826,2 mm<sup>2</sup>

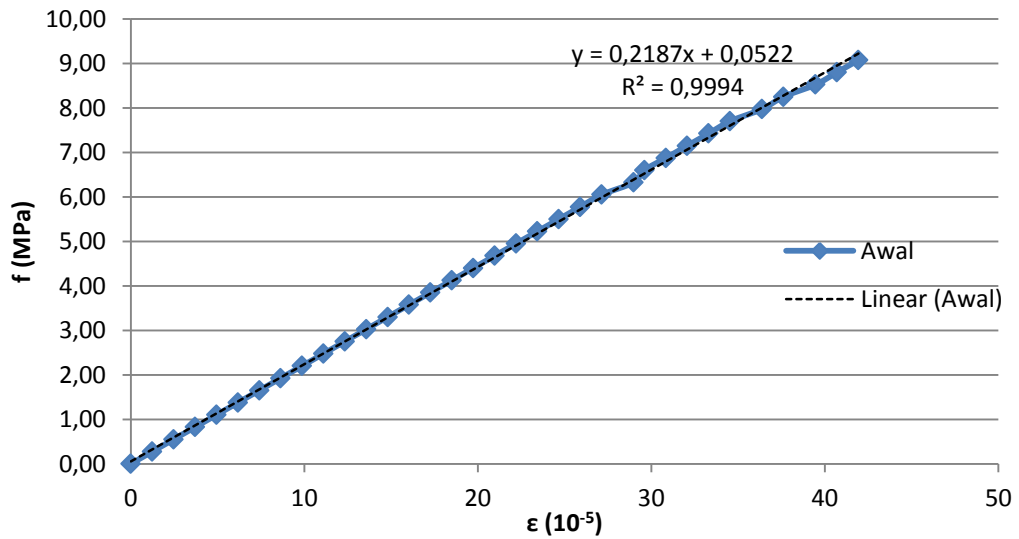
E = 21524,1 MPa

Beban Maks = 16500 Kgf

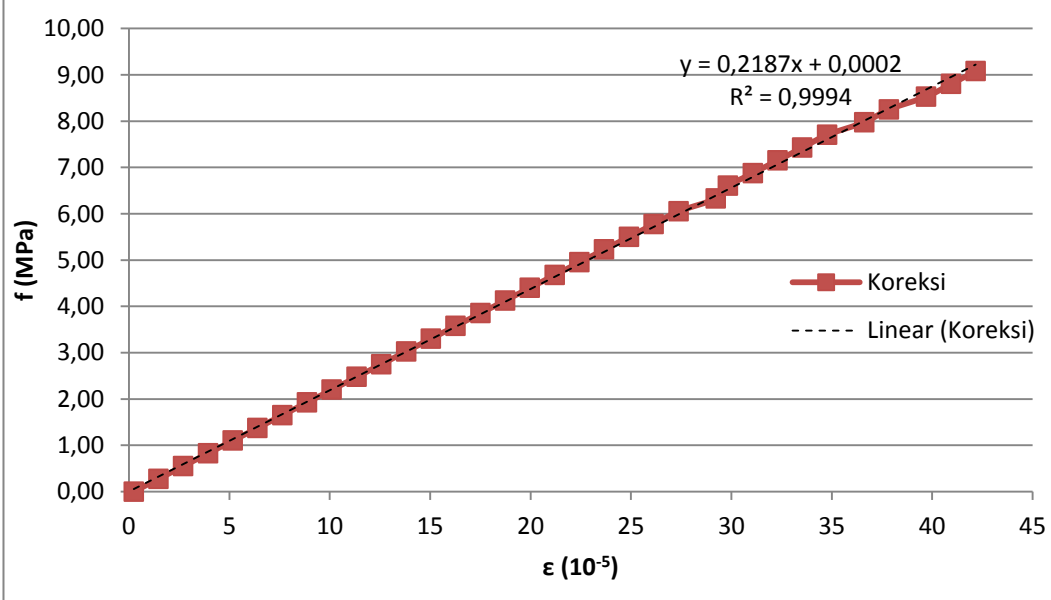
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	0,238
500	4903,4	5	2,5	0,275	1,233	1,471
1000	9806,7	10	5	0,550	2,467	2,705
1500	14710,1	15	7,5	0,825	3,700	3,938
2000	19613,4	20	10	1,100	4,933	5,171
2500	24516,8	25	12,5	1,375	6,167	6,405
3000	29420,1	30	15	1,650	7,400	7,638
3500	34323,5	35	17,5	1,925	8,633	8,871
4000	39226,8	40	20	2,201	9,867	10,105
4500	44130,2	45	22,5	2,476	11,100	11,338
5000	49033,5	50	25	2,751	12,333	12,571
5500	53936,9	55	27,5	3,026	13,567	13,805
6000	58840,2	60	30	3,301	14,800	15,038
6500	63743,6	65	32,5	3,576	16,034	16,272
7000	68646,9	70	35	3,851	17,267	17,505
7500	73550,3	75	37,5	4,126	18,500	18,738
8000	78453,6	80	40	4,401	19,734	19,972
8500	83357,0	85	42,5	4,676	20,967	21,205
9000	88260,3	90	45	4,951	22,200	22,438
9500	93163,7	95	47,5	5,226	23,434	23,672
10000	98067,0	100	50	5,501	24,667	24,905
10500	102970,4	105	52,5	5,776	25,900	26,138
11000	107873,7	110	55	6,051	27,134	27,372
11500	112777,1	117,5	58,75	6,326	28,984	29,222
12000	117680,4	120	60	6,602	29,600	29,838
12500	122583,8	125	62,5	6,877	30,834	31,072
13000	127487,1	130	65	7,152	32,067	32,305
13500	132390,5	135	67,5	7,427	33,300	33,538
14000	137293,8	140	70	7,702	34,534	34,772
14500	142197,2	147,5	73,75	7,977	36,384	36,622
15000	147100,5	152,5	76,25	8,252	37,617	37,855
15500	152003,9	160	80	8,527	39,467	39,705
16000	156907,2	165	82,5	8,802	40,701	40,939
16500	161810,6	170	85	9,077	41,934	42,172



### Grafik Modulus Elastisitas BK c



### Grafik Koreksi Modulus Elastisitas BK c





Kode Beton = BK e

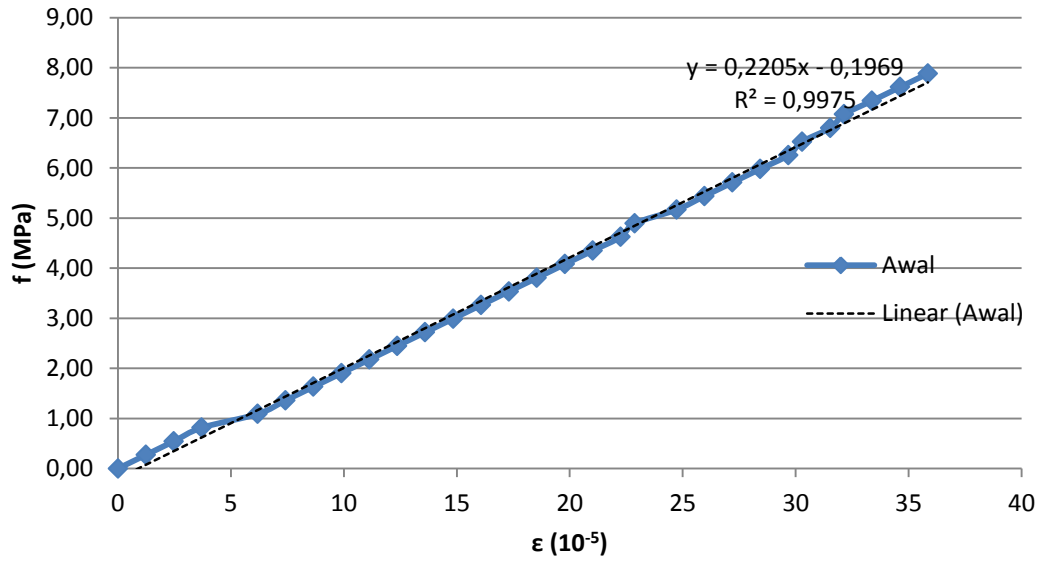
Po = 202,2 mm E = 22552 MPa

Ao = 18033,91 mm<sup>2</sup> Beban Maks = 14500 Kgf

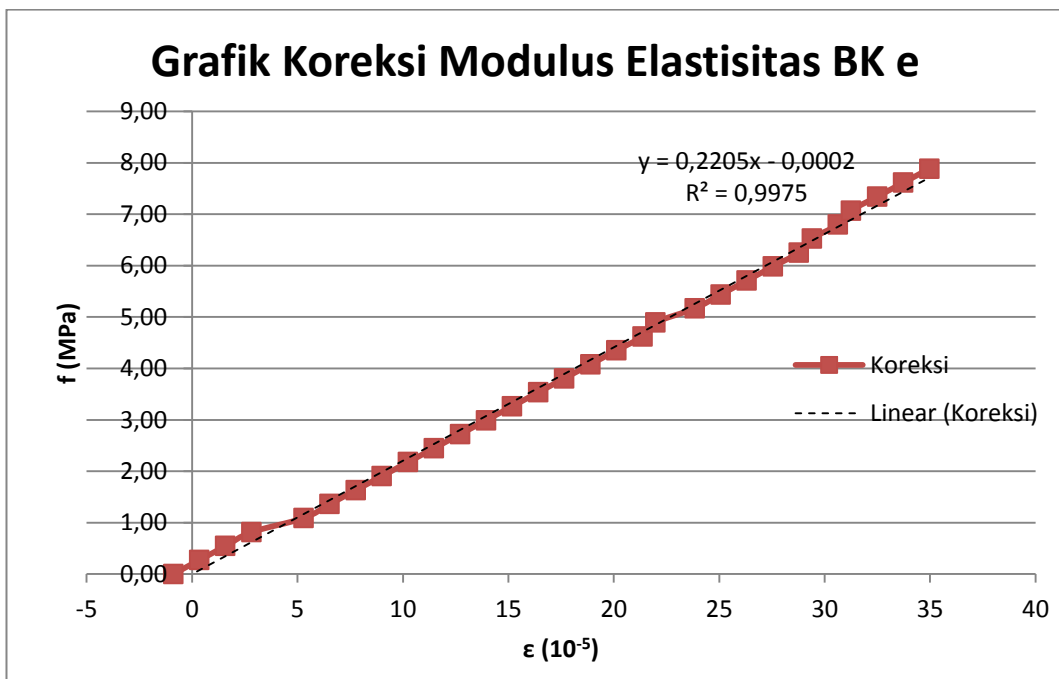
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0,000	-0,892
500	4903,4	5	2,5	0,272	1,236	0,344
1000	9806,7	10	5	0,544	2,473	1,581
1500	14710,1	15	7,5	0,816	3,709	2,817
2000	19613,4	25	12,5	1,088	6,182	5,290
2500	24516,8	30	15	1,359	7,418	6,526
3000	29420,1	35	17,5	1,631	8,655	7,763
3500	34323,5	40	20	1,903	9,891	8,999
4000	39226,8	45	22,5	2,175	11,128	10,236
4500	44130,2	50	25	2,447	12,364	11,472
5000	49033,5	55	27,5	2,719	13,600	12,708
5500	53936,9	60	30	2,991	14,837	13,945
6000	58840,2	65	32,5	3,263	16,073	15,181
6500	63743,6	70	35	3,535	17,310	16,418
7000	68646,9	75	37,5	3,807	18,546	17,654
7500	73550,3	80	40	4,078	19,782	18,890
8000	78453,6	85	42,5	4,350	21,019	20,127
8500	83357,0	90	45	4,622	22,255	21,363
9000	88260,3	92,5	46,25	4,894	22,873	21,981
9500	93163,7	100	50	5,166	24,728	23,836
10000	98067,0	105	52,5	5,438	25,964	25,072
10500	102970,4	110	55	5,710	27,201	26,309
11000	107873,7	115	57,5	5,982	28,437	27,545
11500	112777,1	120	60	6,254	29,674	28,782
12000	117680,4	122,5	61,25	6,526	30,292	29,400
12500	122583,8	127,5	63,75	6,797	31,528	30,636
13000	127487,1	130	65	7,069	32,146	31,254
13500	132390,5	135	67,5	7,341	33,383	32,491
14000	137293,8	140	70	7,613	34,619	33,727
14500	142197,2	145	72,5	7,885	35,856	34,964



### Grafik Modulus Elastisitas BK e



### Grafik Koreksi Modulus Elastisitas BK e





Kode Beton = BK d

Po = 202,7 mm

Ao = 18634 mm<sup>2</sup>

E = 19544,33 MPa

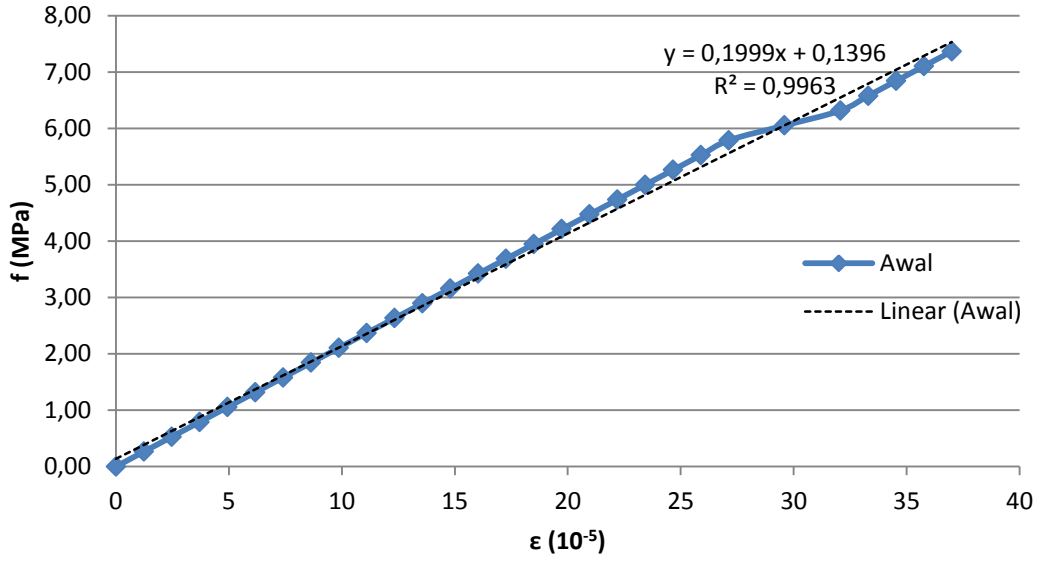
Beban Maks = 14000 Kgf

Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0,000	0,698
500	4903,4	5	2,5	0,263	1,233	1,931
1000	9806,7	10	5	0,526	2,467	3,165
1500	14710,1	15	7,5	0,789	3,700	4,398
2000	19613,4	20	10	1,053	4,933	5,631
2500	24516,8	25	12,5	1,316	6,167	6,865
3000	29420,1	30	15	1,579	7,400	8,098
3500	34323,5	35	17,5	1,842	8,633	9,331
4000	39226,8	40	20	2,105	9,867	10,565
4500	44130,2	45	22,5	2,368	11,100	11,798
5000	49033,5	50	25	2,631	12,333	13,031
5500	53936,9	55	27,5	2,895	13,567	14,265
6000	58840,2	60	30	3,158	14,800	15,498
6500	63743,6	65	32,5	3,421	16,034	16,732
7000	68646,9	70	35	3,684	17,267	17,965
7500	73550,3	75	37,5	3,947	18,500	19,198
8000	78453,6	80	40	4,210	19,734	20,432
8500	83357,0	85	42,5	4,473	20,967	21,665
9000	88260,3	90	45	4,737	22,200	22,898
9500	93163,7	95	47,5	5,000	23,434	24,132
10000	98067,0	100	50	5,263	24,667	25,365
10500	102970,4	105	52,5	5,526	25,900	26,598
11000	107873,7	110	55	5,789	27,134	27,832
11500	112777,1	120	60	6,052	29,600	30,298
12000	117680,4	130	65	6,315	32,067	32,765
12500	122583,8	135	67,5	6,578	33,300	33,998
13000	127487,1	140	70	6,842	34,534	35,232
13500	132390,5	145	72,5	7,105	35,767	36,465
14000	137293,8	150	75	7,368	37,000	37,698

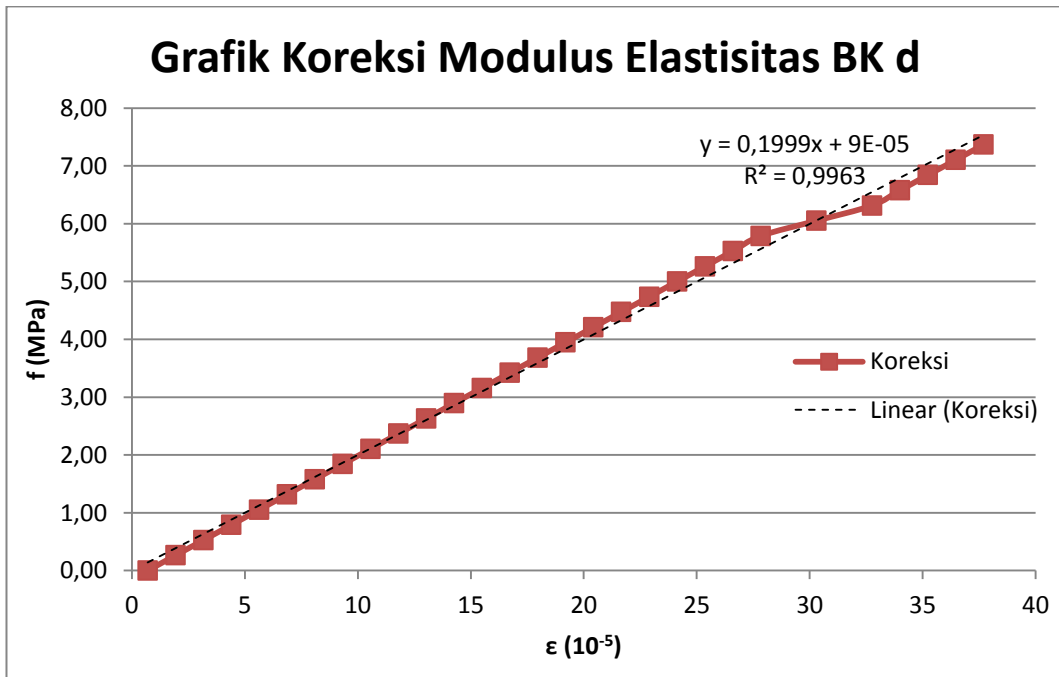




### Grafik Modulus Elastisitas BK d



### Grafik Koreksi Modulus Elastisitas BK d





Kode Beton = BKFA e

Po = 200,7 mm

Ao = 7737,55 mm<sup>2</sup>

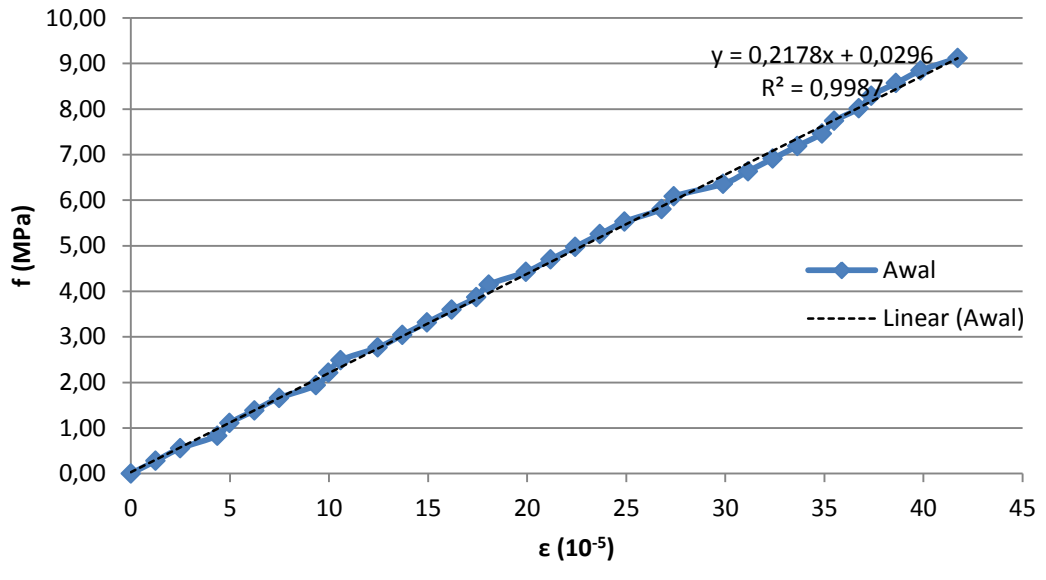
E = 21790,79 MPa

Beban Maks = 16500 Kgf

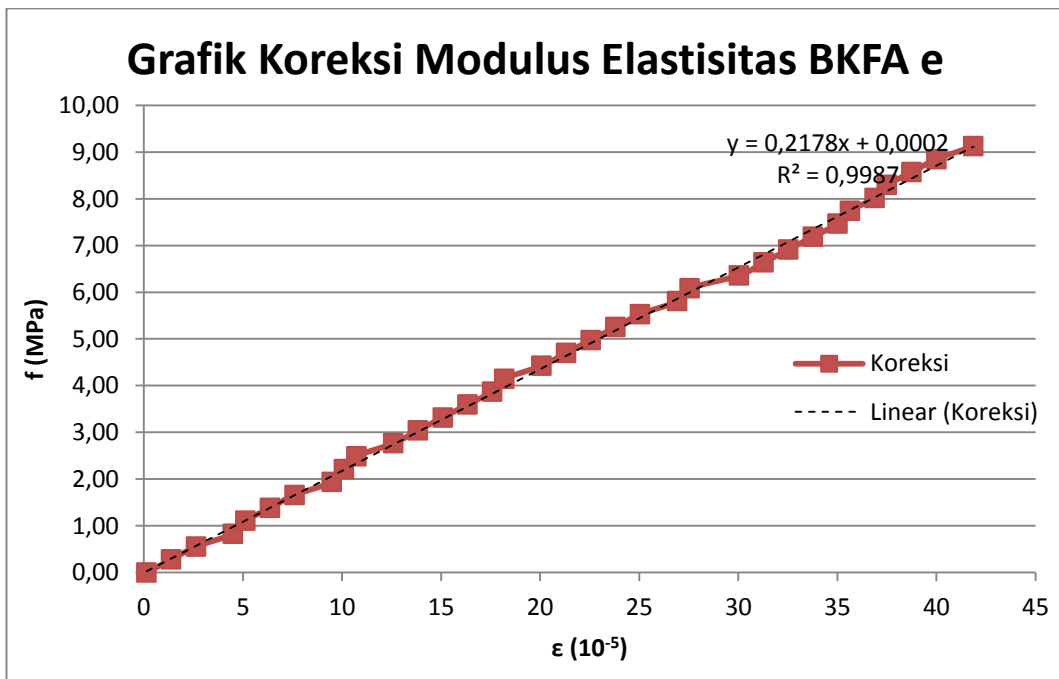
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	0,135
500	4903,4	5	2,5	0,276	1,246	1,381
1000	9806,7	10	5	0,553	2,491	2,626
1500	14710,1	17,5	8,75	0,829	4,360	4,495
2000	19613,4	20	10	1,106	4,983	5,118
2500	24516,8	25	12,5	1,382	6,228	6,363
3000	29420,1	30	15	1,659	7,474	7,609
3500	34323,5	37,5	18,75	1,935	9,342	9,477
4000	39226,8	40	20	2,212	9,965	10,100
4500	44130,2	42,5	21,25	2,488	10,588	10,723
5000	49033,5	50	25	2,764	12,456	12,591
5500	53936,9	55	27,5	3,041	13,702	13,837
6000	58840,2	60	30	3,317	14,948	15,083
6500	63743,6	65	32,5	3,594	16,193	16,328
7000	68646,9	70	35	3,870	17,439	17,574
7500	73550,3	72,5	36,25	4,147	18,062	18,197
8000	78453,6	80	40	4,423	19,930	20,065
8500	83357,0	85	42,5	4,699	21,176	21,311
9000	88260,3	90	45	4,976	22,422	22,557
9500	93163,7	95	47,5	5,252	23,667	23,802
10000	98067,0	100	50	5,529	24,913	25,048
10500	102970,4	107,5	53,75	5,805	26,781	26,916
11000	107873,7	110	55	6,082	27,404	27,539
11500	112777,1	120	60	6,358	29,895	30,030
12000	117680,4	125	62,5	6,635	31,141	31,276
12500	122583,8	130	65	6,911	32,387	32,522
13000	127487,1	135	67,5	7,187	33,632	33,767
13500	132390,5	140	70	7,464	34,878	35,013
14000	137293,8	142,5	71,25	7,740	35,501	35,636
14500	142197,2	147,5	73,75	8,017	36,746	36,881
15000	147100,5	150	75	8,293	37,369	37,504
15500	152003,9	155	77,5	8,570	38,615	38,750
16000	156907,2	160	80	8,846	39,860	39,995
16500	161810,6	167,5	83,75	9,122	41,729	41,864



### Grafik Modulus Elastisitas BKFA e



### Grafik Koreksi Modulus Elastisitas BKFA e





Kode Beton = BKFA d

Po = 201,9 mm

Ao = 17772,98 mm<sup>2</sup>

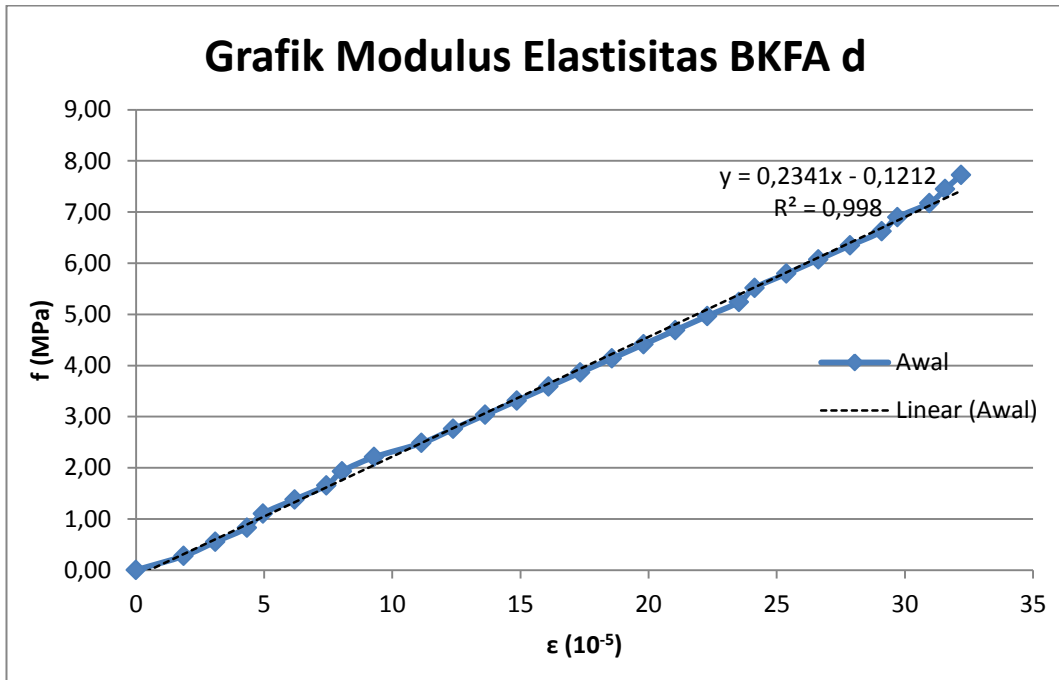
E = 23614,646 MPa

Beban Maks = 14000 Kgf

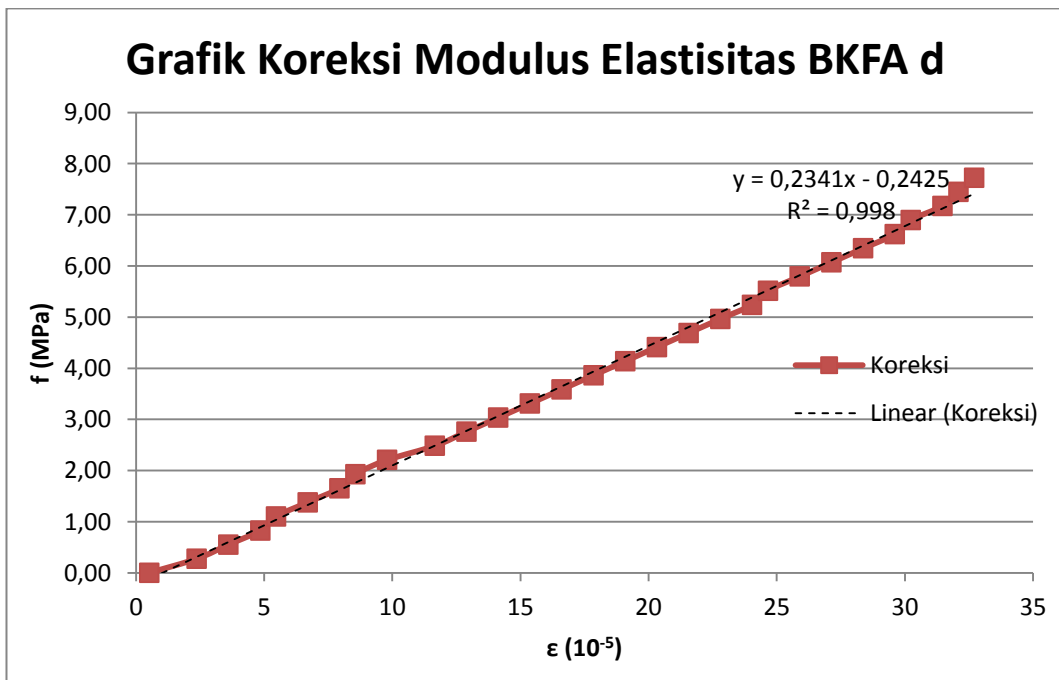
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0	0,000	0
500	4903,4	0,75	7,5	3,75	0,276	1,857
1000	9806,7	1,25	12,5	6,25	0,552	3,096
1500	14710,1	1,75	17,5	8,75	0,828	4,334
2000	19613,4	2	20	10	1,104	4,953
2500	24516,8	2,5	25	12,5	1,379	6,191
3000	29420,1	3	30	15	1,655	7,429
3500	34323,5	3,25	32,5	16,25	1,931	8,049
4000	39226,8	3,75	37,5	18,75	2,207	9,287
4500	44130,2	4,5	45	22,5	2,483	11,144
5000	49033,5	5	50	25	2,759	12,382
5500	53936,9	5,5	55	27,5	3,035	13,621
6000	58840,2	6	60	30	3,311	14,859
6500	63743,6	6,5	65	32,5	3,587	16,097
7000	68646,9	7	70	35	3,862	17,335
7500	73550,3	7,5	75	37,5	4,138	18,574
8000	78453,6	8	80	40	4,414	19,812
8500	83357,0	8,5	85	42,5	4,690	21,050
9000	88260,3	9	90	45	4,966	22,288
9500	93163,7	9,5	95	47,5	5,242	23,526
10000	98067,0	9,75	97,5	48,75	5,518	24,146
10500	102970,4	10,25	102,5	51,25	5,794	25,384
11000	107873,7	10,75	107,5	53,75	6,070	26,622
11500	112777,1	11,25	112,5	56,25	6,345	27,860
12000	117680,4	11,75	117,5	58,75	6,621	29,099
12500	122583,8	12	120	60	6,897	29,718
13000	127487,1	12,5	125	62,5	7,173	30,956
13500	132390,5	12,75	127,5	63,75	7,449	31,575
14000	137293,8	13	130	65	7,725	32,194



### Grafik Modulus Elastisitas BKFA d



### Grafik Koreksi Modulus Elastisitas BKFA d





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Kode Beton = BKFA b

Po = 201,9 mm

E = 25421,63 MPa

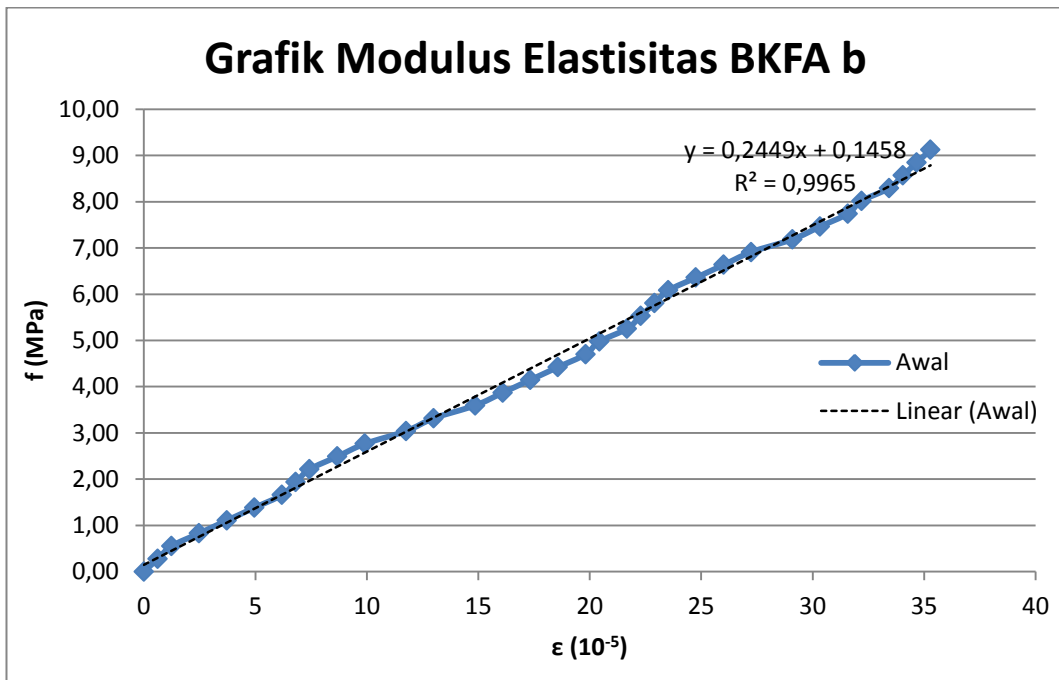
Ao = 17737,55 mm<sup>2</sup>

Beban Maks = 16500 Kgf

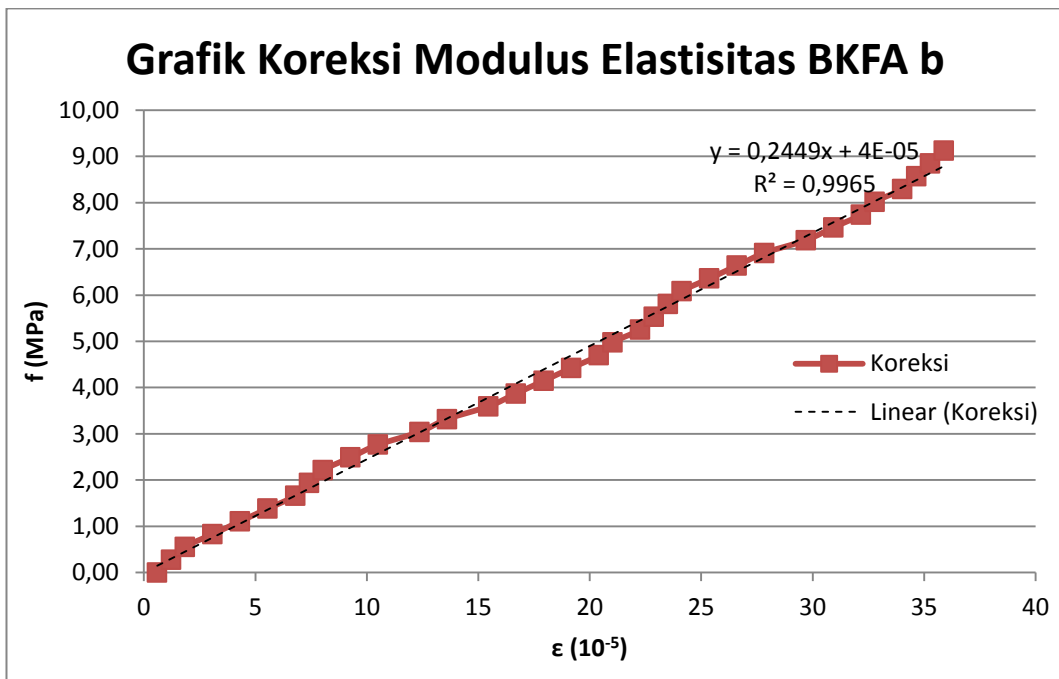
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0	0,000	0
500	4903,4	0,25	2,5	1,25	0,276	0,619
1000	9806,7	0,5	5	2,5	0,553	1,238
1500	14710,1	1	10	5	0,829	2,476
2000	19613,4	1,5	15	7,5	1,106	3,715
2500	24516,8	2	20	10	1,382	4,953
3000	29420,1	2,5	25	12,5	1,659	6,191
3500	34323,5	2,75	27,5	13,75	1,935	6,810
4000	39226,8	3	30	15	2,212	7,429
4500	44130,2	3,5	35	17,5	2,488	8,668
5000	49033,5	4	40	20	2,764	9,906
5500	53936,9	4,75	47,5	23,75	3,041	11,763
6000	58840,2	5,25	52,5	26,25	3,317	13,001
6500	63743,6	6	60	30	3,594	14,859
7000	68646,9	6,5	65	32,5	3,870	16,097
7500	73550,3	7	70	35	4,147	17,335
8000	78453,6	7,5	75	37,5	4,423	18,574
8500	83357,0	8	80	40	4,699	19,812
9000	88260,3	8,25	82,5	41,25	4,976	20,431
9500	93163,7	8,75	87,5	43,75	5,252	21,669
10000	98067,0	9	90	45	5,529	22,288
10500	102970,4	9,25	92,5	46,25	5,805	22,907
11000	107873,7	9,5	95	47,5	6,082	23,526
11500	112777,1	10	100	50	6,358	24,765
12000	117680,4	10,5	105	52,5	6,635	26,003
12500	122583,8	11	110	55	6,911	27,241
13000	127487,1	11,75	117,5	58,75	7,187	29,099
13500	132390,5	12,25	122,5	61,25	7,464	30,337
14000	137293,8	12,75	127,5	63,75	7,740	31,575
14500	142197,2	13	130	65	8,017	32,194
15000	147100,5	13,5	135	67,5	8,293	33,432
15500	152003,9	13,75	137,5	68,75	8,570	34,052
16000	156907,2	14	140	70	8,846	34,671
16500	161810,6	14,25	142,5	71,25	9,122	35,290



### Grafik Modulus Elastisitas BKFA b



### Grafik Koreksi Modulus Elastisitas BKFA b





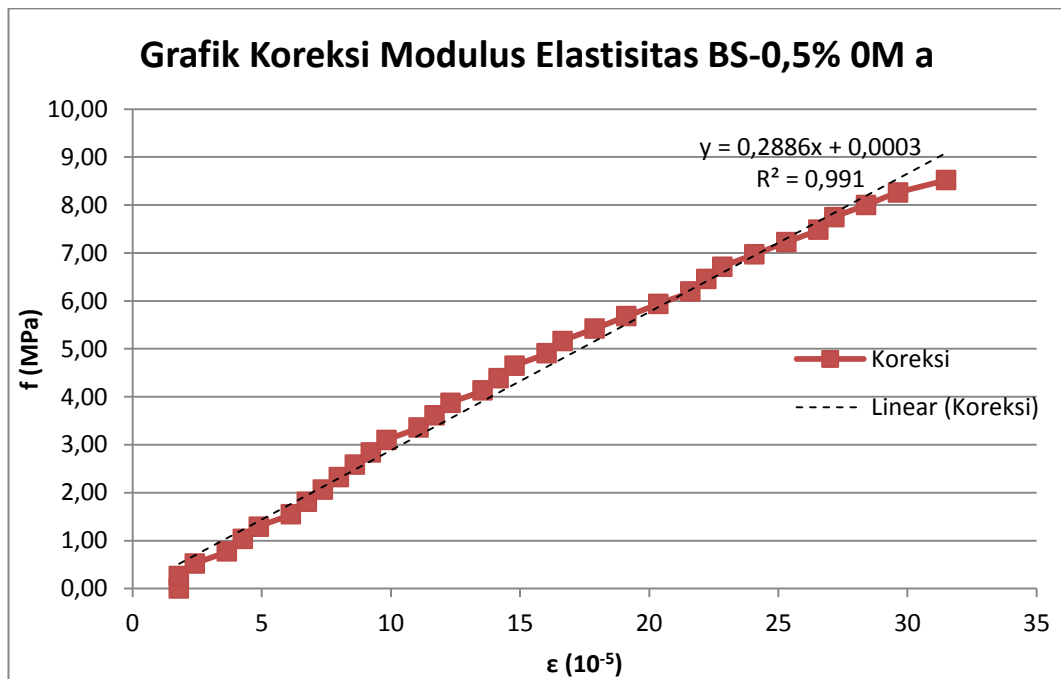
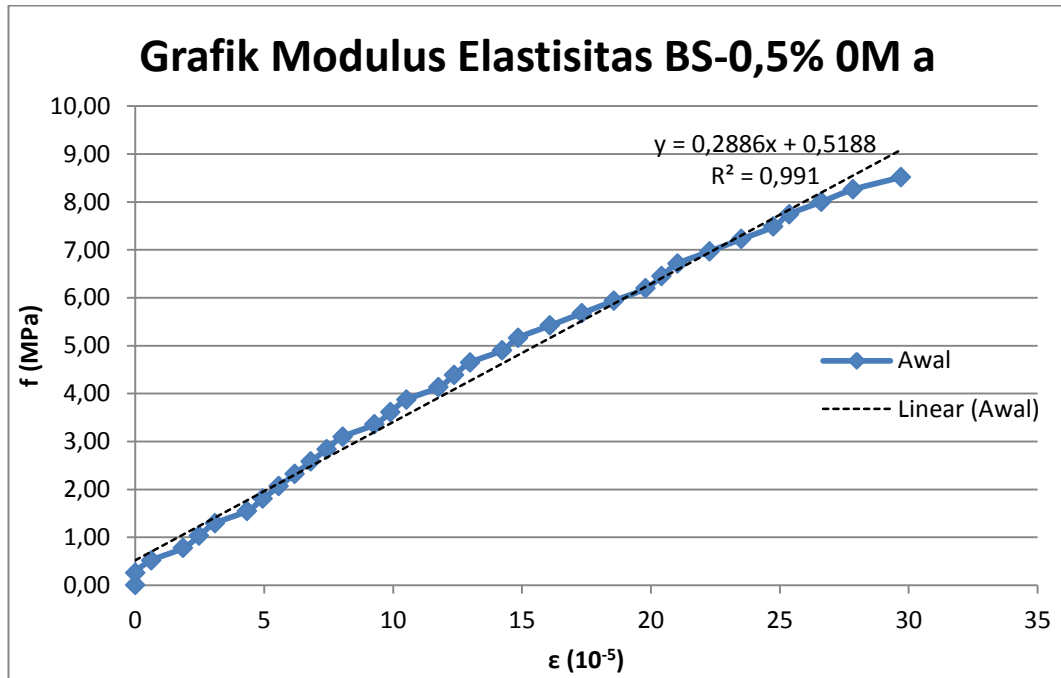
Kode Beton = BS-0,5% 0M a

Po = 202 mm E = 27037,794 MPa

Ao = 18998,768 mm<sup>2</sup> Beban Maks = 16500 Kgf

Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	1,797
500	4903,4	0	0	0,258	0,000	1,797
1000	9806,7	2,5	1,25	0,516	0,619	2,416
1500	14710,1	7,5	3,75	0,774	1,856	3,653
2000	19613,4	10	5	1,032	2,475	4,272
2500	24516,8	12,5	6,25	1,290	3,094	4,891
3000	29420,1	17,5	8,75	1,549	4,332	6,129
3500	34323,5	20	10	1,807	4,950	6,747
4000	39226,8	22,5	11,25	2,065	5,569	7,366
4500	44130,2	25	12,5	2,323	6,188	7,985
5000	49033,5	27,5	13,75	2,581	6,807	8,604
5500	53936,9	30	15	2,839	7,426	9,223
6000	58840,2	32,5	16,25	3,097	8,045	9,842
6500	63743,6	37,5	18,75	3,355	9,282	11,079
7000	68646,9	40	20	3,613	9,901	11,698
7500	73550,3	42,5	21,25	3,871	10,520	12,317
8000	78453,6	47,5	23,75	4,129	11,757	13,554
8500	83357,0	50	25	4,387	12,376	14,173
9000	88260,3	52,5	26,25	4,646	12,995	14,792
9500	93163,7	57,5	28,75	4,904	14,233	16,030
10000	98067,0	60	30	5,162	14,851	16,648
10500	102970,4	65	32,5	5,420	16,089	17,886
11000	107873,7	70	35	5,678	17,327	19,124
11500	112777,1	75	37,5	5,936	18,564	20,361
12000	117680,4	80	40	6,194	19,802	21,599
12500	122583,8	82,5	41,25	6,452	20,421	22,218
13000	127487,1	85	42,5	6,710	21,040	22,837
13500	132390,5	90	45	6,968	22,277	24,074
14000	137293,8	95	47,5	7,226	23,515	25,312
14500	142197,2	100	50	7,485	24,752	26,549
15000	147100,5	102,5	51,25	7,743	25,371	27,168
15500	152003,9	107,5	53,75	8,001	26,609	28,406
16000	156907,2	112,5	56,25	8,259	27,847	29,644
16500	161810,6	120	60	8,517	29,703	31,500







Kode Beton = BS-0,5% 0M b

Po = 201,7 mm

E = 24120,196 MPa

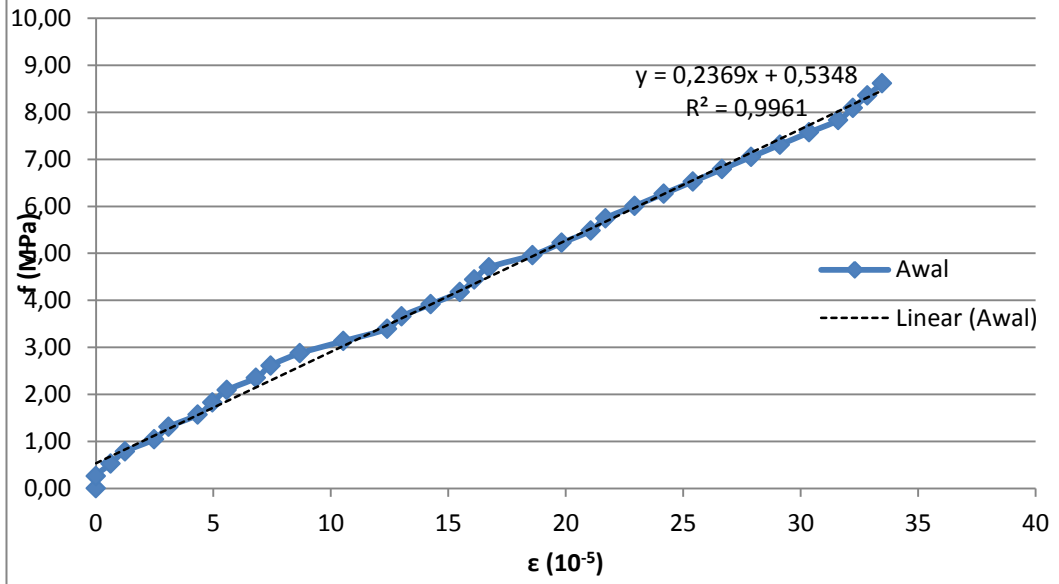
Ao = 18779,483 mm<sup>2</sup>

Beban Maks = 16500 Kgf

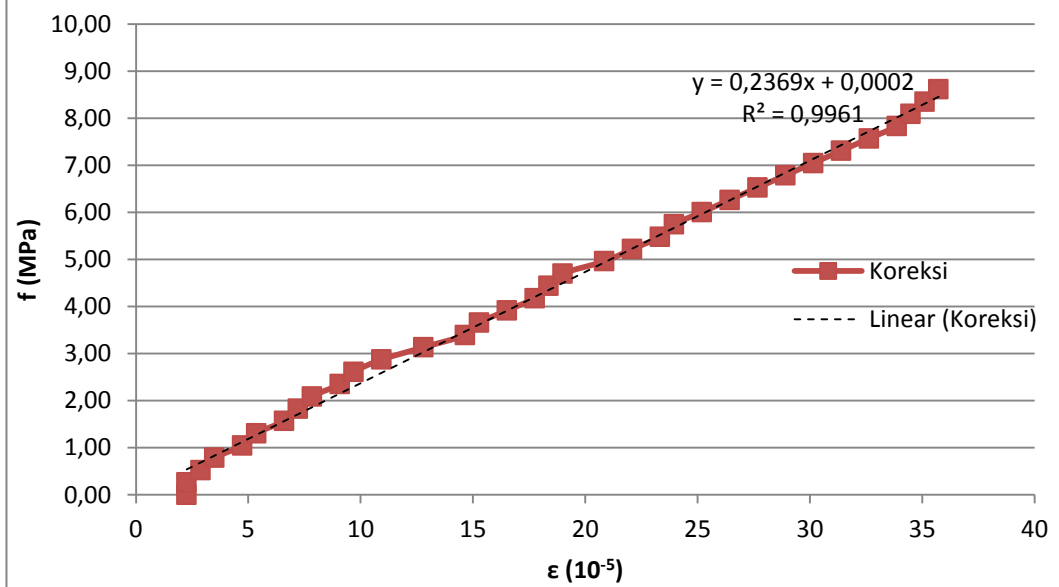
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	2,257
500	4903,4	0	0	0,261	0,000	2,257
1000	9806,7	2,5	1,25	0,522	0,620	2,877
1500	14710,1	5	2,5	0,783	1,239	3,496
2000	19613,4	10	5	1,044	2,479	4,736
2500	24516,8	12,5	6,25	1,306	3,099	5,356
3000	29420,1	17,5	8,75	1,567	4,338	6,595
3500	34323,5	20	10	1,828	4,958	7,215
4000	39226,8	22,5	11,25	2,089	5,578	7,835
4500	44130,2	27,5	13,75	2,350	6,817	9,074
5000	49033,5	30	15	2,611	7,437	9,694
5500	53936,9	35	17,5	2,872	8,676	10,933
6000	58840,2	42,5	21,25	3,133	10,535	12,792
6500	63743,6	50	25	3,394	12,395	14,652
7000	68646,9	52,5	26,25	3,655	13,014	15,271
7500	73550,3	57,5	28,75	3,917	14,254	16,511
8000	78453,6	62,5	31,25	4,178	15,493	17,750
8500	83357,0	65	32,5	4,439	16,113	18,370
9000	88260,3	67,5	33,75	4,700	16,733	18,990
9500	93163,7	75	37,5	4,961	18,592	20,849
10000	98067,0	80	40	5,222	19,831	22,088
10500	102970,4	85	42,5	5,483	21,071	23,328
11000	107873,7	87,5	43,75	5,744	21,691	23,948
11500	112777,1	92,5	46,25	6,005	22,930	25,187
12000	117680,4	97,5	48,75	6,266	24,170	26,427
12500	122583,8	102,5	51,25	6,528	25,409	27,666
13000	127487,1	107,5	53,75	6,789	26,648	28,905
13500	132390,5	112,5	56,25	7,050	27,888	30,145
14000	137293,8	117,5	58,75	7,311	29,127	31,384
14500	142197,2	122,5	61,25	7,572	30,367	32,624
15000	147100,5	127,5	63,75	7,833	31,606	33,863
15500	152003,9	130	65	8,094	32,226	34,483
16000	156907,2	132,5	66,25	8,355	32,846	35,103
16500	161810,6	135	67,5	8,616	33,466	35,723



### Grafik Modulus Elastisitas BS-0,5% 0M b



### Grafik Koreksi Modulus Elastisitas BS-0,5% 0M b





Kode Beton = BS-0,5% 0M d

Po = 202,7 mm

E = 27210,615 MPa

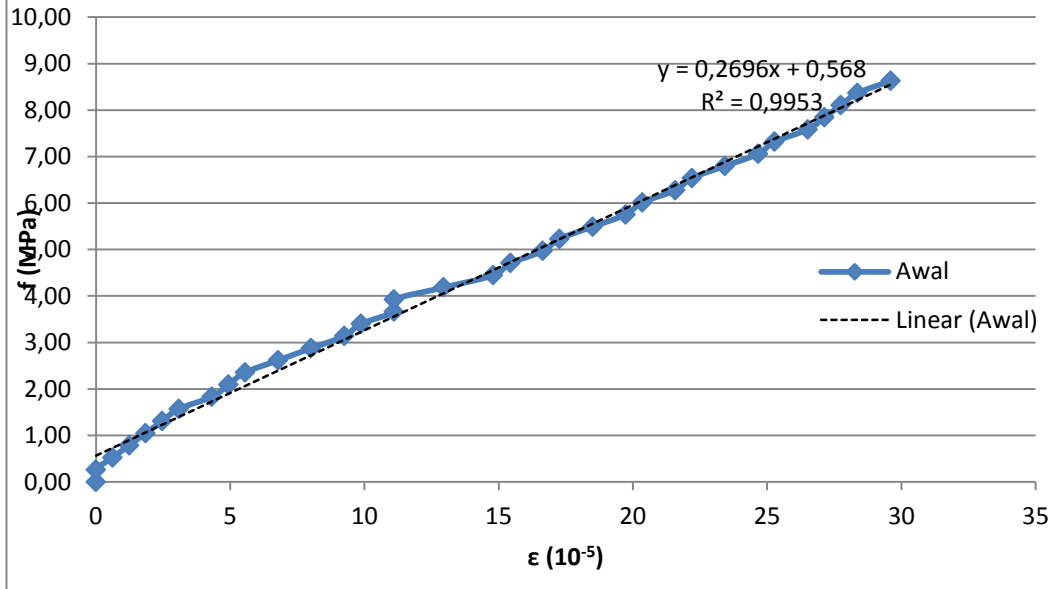
Ao = 18755,196 mm<sup>2</sup>

Beban Maks = 16500 Kgf

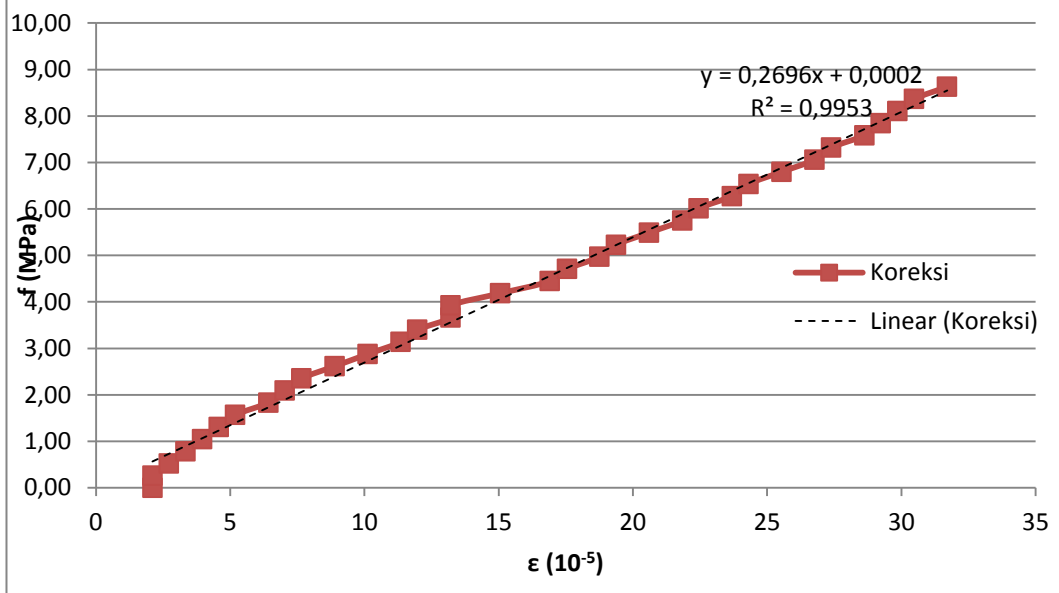
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0	0,000	0
500	4903,4	0	0	0	0,261	0,000
1000	9806,7	0,25	2,5	1,25	0,523	0,617
1500	14710,1	0,5	5	2,5	0,784	1,233
2000	19613,4	0,75	7,5	3,75	1,046	1,850
2500	24516,8	1	10	5	1,307	2,467
3000	29420,1	1,25	12,5	6,25	1,569	3,083
3500	34323,5	1,75	17,5	8,75	1,830	4,317
4000	39226,8	2	20	10	2,092	4,933
4500	44130,2	2,25	22,5	11,25	2,353	5,550
5000	49033,5	2,75	27,5	13,75	2,614	6,783
5500	53936,9	3,25	32,5	16,25	2,876	8,017
6000	58840,2	3,75	37,5	18,75	3,137	9,250
6500	63743,6	4	40	20	3,399	9,867
7000	68646,9	4,5	45	22,5	3,660	11,100
7500	73550,3	4,5	45	22,5	3,922	11,100
8000	78453,6	5,25	52,5	26,25	4,183	12,950
8500	83357,0	6	60	30	4,444	14,800
9000	88260,3	6,26	62,6	31,3	4,706	15,442
9500	93163,7	6,75	67,5	33,75	4,967	16,650
10000	98067,0	7	70	35	5,229	17,267
10500	102970,4	7,5	75	37,5	5,490	18,500
11000	107873,7	8	80	40	5,752	19,734
11500	112777,1	8,25	82,5	41,25	6,013	20,350
12000	117680,4	8,75	87,5	43,75	6,275	21,584
12500	122583,8	9	90	45	6,536	22,200
13000	127487,1	9,5	95	47,5	6,797	23,434
13500	132390,5	10	100	50	7,059	24,667
14000	137293,8	10,25	102,5	51,25	7,320	25,284
14500	142197,2	10,75	107,5	53,75	7,582	26,517
15000	147100,5	11	110	55	7,843	27,134
15500	152003,9	11,25	112,5	56,25	8,105	27,750
16000	156907,2	11,5	115	57,5	8,366	28,367
16500	161810,6	12	120	60	8,628	29,600



### Grafik Modulus Elastisitas BS-0,5% 0M d



### Grafik Koreksi Modulus Elastisitas BS-0,5% 0M d





Kode Beton = BS-1% 0M a

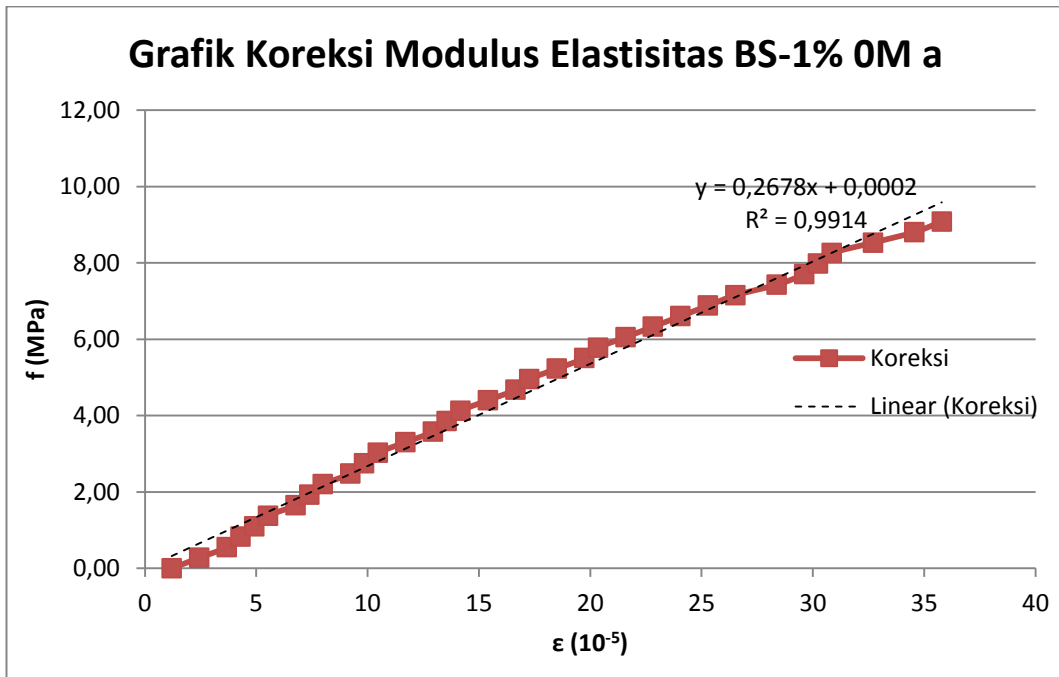
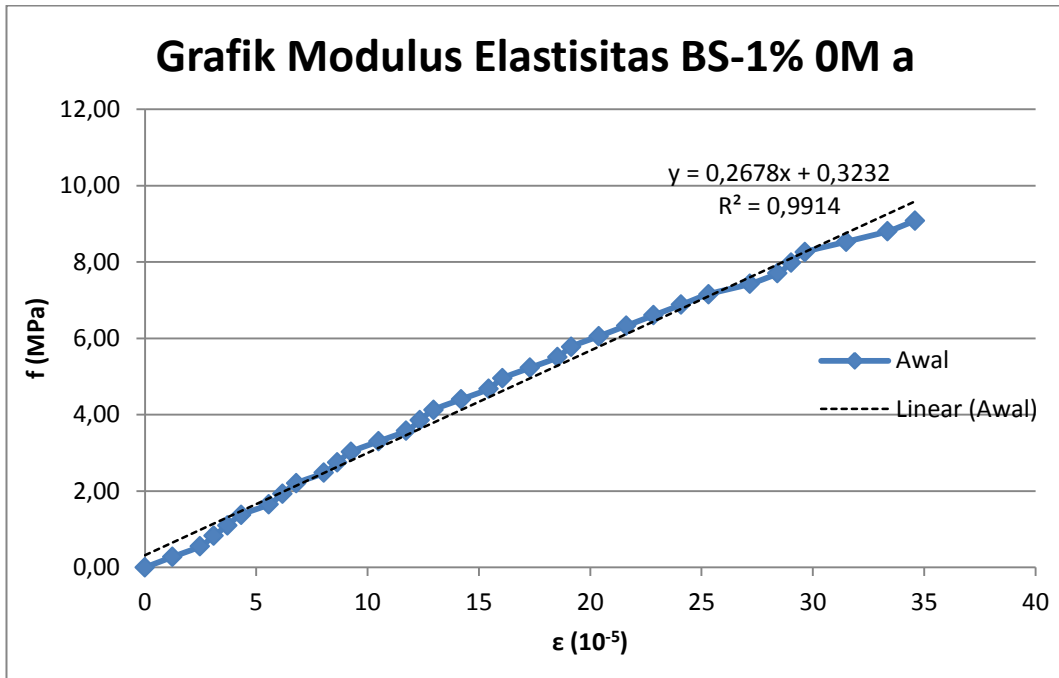
Po = 202,4 mm

E = 25369,894 MPa

Ao = 17820,283 mm<sup>2</sup>

Beban Maks = 16500 Kgf

Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	1,206
500	4903,4	5	2,5	0,275	1,235	2,441
1000	9806,7	10	5	0,550	2,470	3,676
1500	14710,1	12,5	6,25	0,825	3,088	4,294
2000	19613,4	15	7,5	1,101	3,706	4,912
2500	24516,8	17,5	8,75	1,376	4,323	5,529
3000	29420,1	22,5	11,25	1,651	5,558	6,764
3500	34323,5	25	12,5	1,926	6,176	7,382
4000	39226,8	27,5	13,75	2,201	6,793	7,999
4500	44130,2	32,5	16,25	2,476	8,029	9,235
5000	49033,5	35	17,5	2,752	8,646	9,852
5500	53936,9	37,5	18,75	3,027	9,264	10,470
6000	58840,2	42,5	21,25	3,302	10,499	11,705
6500	63743,6	47,5	23,75	3,577	11,734	12,940
7000	68646,9	50	25	3,852	12,352	13,558
7500	73550,3	52,5	26,25	4,127	12,969	14,175
8000	78453,6	57,5	28,75	4,402	14,205	15,411
8500	83357,0	62,5	31,25	4,678	15,440	16,646
9000	88260,3	65	32,5	4,953	16,057	17,263
9500	93163,7	70	35	5,228	17,292	18,498
10000	98067,0	75	37,5	5,503	18,528	19,734
10500	102970,4	77,5	38,75	5,778	19,145	20,351
11000	107873,7	82,5	41,25	6,053	20,380	21,586
11500	112777,1	87,5	43,75	6,329	21,616	22,822
12000	117680,4	92,5	46,25	6,604	22,851	24,057
12500	122583,8	97,5	48,75	6,879	24,086	25,292
13000	127487,1	102,5	51,25	7,154	25,321	26,527
13500	132390,5	110	55	7,429	27,174	28,380
14000	137293,8	115	57,5	7,704	28,409	29,615
14500	142197,2	117,5	58,75	7,980	29,027	30,233
15000	147100,5	120	60	8,255	29,644	30,850
15500	152003,9	127,5	63,75	8,530	31,497	32,703
16000	156907,2	135	67,5	8,805	33,350	34,556
16500	161810,6	140	70	9,080	34,585	35,791





Kode Beton = BS-1% 0M b

Po = 202 mm

E = 26928,059 MPa

Ao = 18272,768 mm<sup>2</sup>

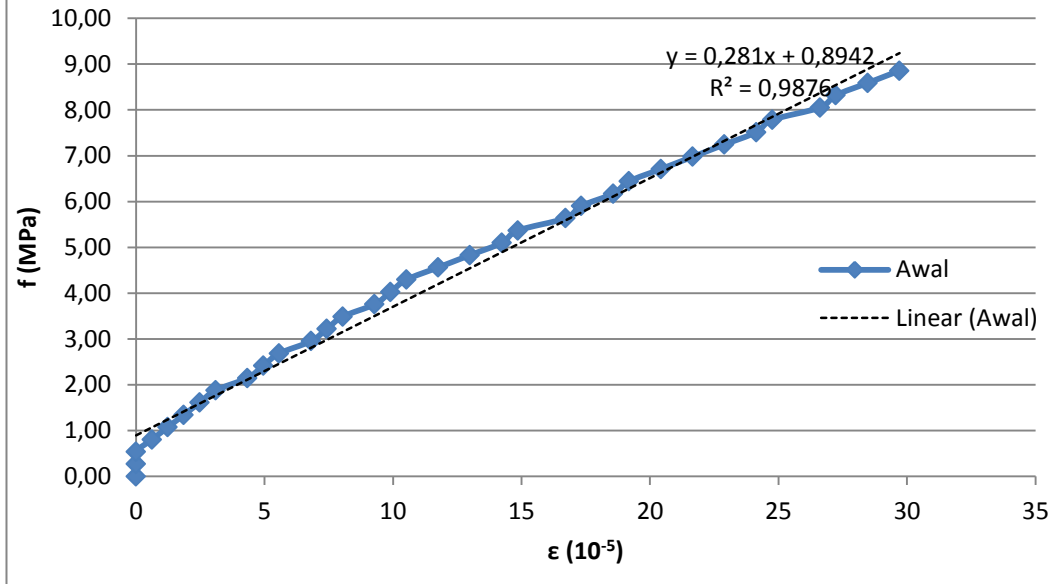
Beban Maks = 16500 Kgf

Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	3,182
500	4903,4	0	0	0,268	0,000	3,182
1000	9806,7	0	0	0,537	0,000	3,182
1500	14710,1	2,5	1,25	0,805	0,619	3,801
2000	19613,4	5	2,5	1,073	1,238	4,420
2500	24516,8	7,5	3,75	1,342	1,856	5,038
3000	29420,1	10	5	1,610	2,475	5,657
3500	34323,5	12,5	6,25	1,878	3,094	6,276
4000	39226,8	17,5	8,75	2,147	4,332	7,514
4500	44130,2	20	10	2,415	4,950	8,132
5000	49033,5	22,5	11,25	2,683	5,569	8,751
5500	53936,9	27,5	13,75	2,952	6,807	9,989
6000	58840,2	30	15	3,220	7,426	10,608
6500	63743,6	32,5	16,25	3,488	8,045	11,227
7000	68646,9	37,5	18,75	3,757	9,282	12,464
7500	73550,3	40	20	4,025	9,901	13,083
8000	78453,6	42,5	21,25	4,293	10,520	13,702
8500	83357,0	47,5	23,75	4,562	11,757	14,939
9000	88260,3	52,5	26,25	4,830	12,995	16,177
9500	93163,7	57,5	28,75	5,098	14,233	17,415
10000	98067,0	60	30	5,367	14,851	18,033
10500	102970,4	67,5	33,75	5,635	16,708	19,890
11000	107873,7	70	35	5,904	17,327	20,509
11500	112777,1	75	37,5	6,172	18,564	21,746
12000	117680,4	77,5	38,75	6,440	19,183	22,365
12500	122583,8	82,5	41,25	6,709	20,421	23,603
13000	127487,1	87,5	43,75	6,977	21,658	24,840
13500	132390,5	92,5	46,25	7,245	22,896	26,078
14000	137293,8	97,5	48,75	7,514	24,134	27,316
14500	142197,2	100	50	7,782	24,752	27,934
15000	147100,5	107,5	53,75	8,050	26,609	29,791
15500	152003,9	110	55	8,319	27,228	30,410
16000	156907,2	115	57,5	8,587	28,465	31,647
16500	161810,6	120	60	8,855	29,703	32,885

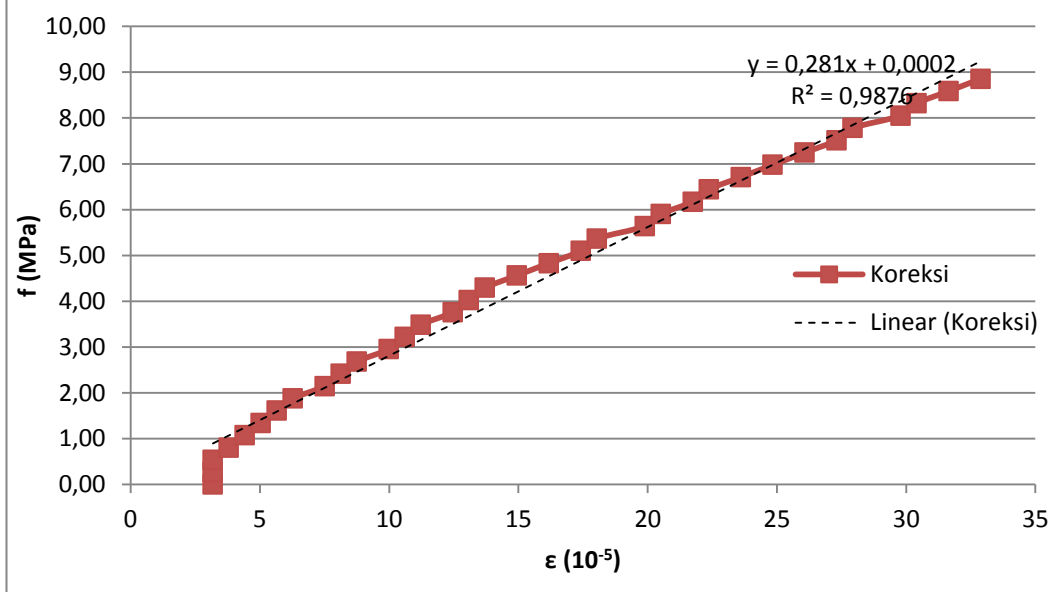




### Grafik Modulus Elastisitas BS-1% 0M b



### Grafik Koreksi Modulus Elastisitas BS-1% 0M b



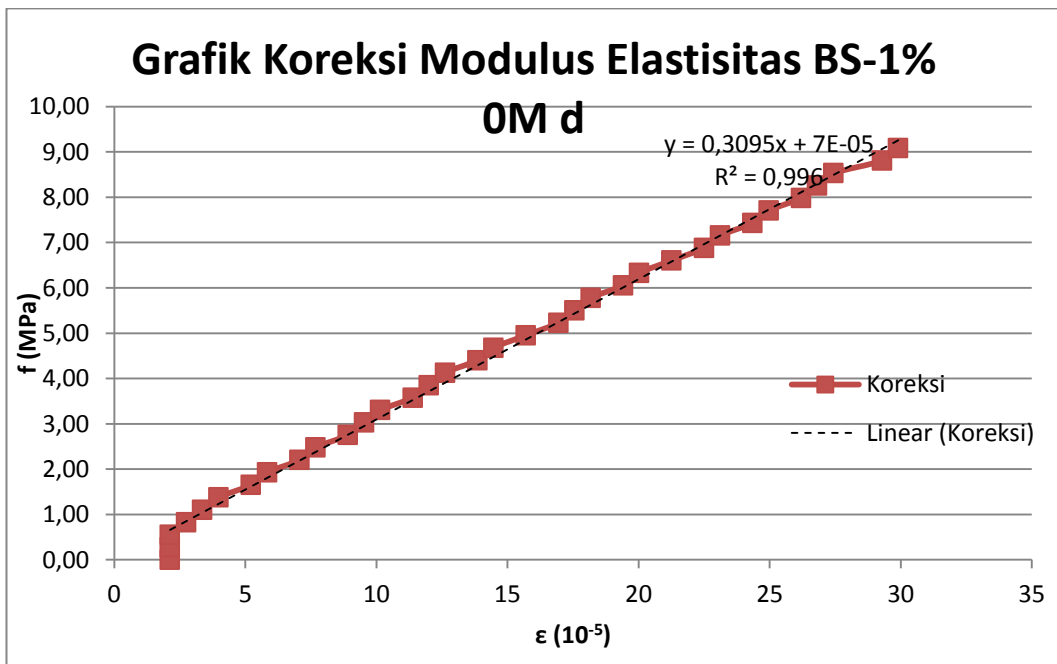
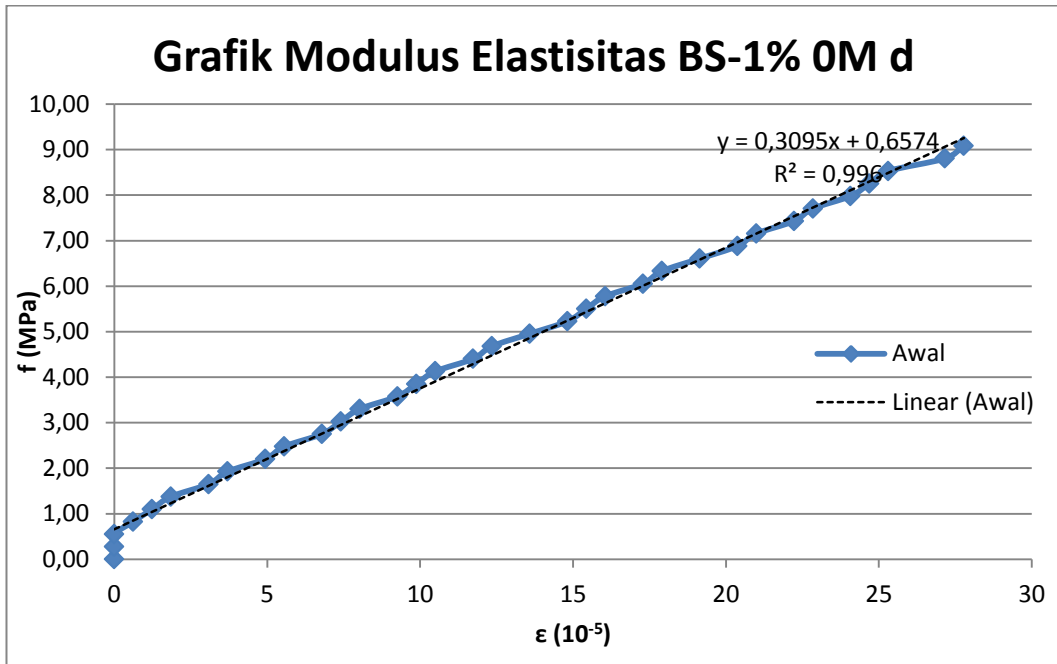


Kode Beton = BS-1% 0M d

Po = 202,5 mm E = 30366,534 MPa

Ao = 17820,283 mm<sup>2</sup> Beban Maks = 16500 Kgf

Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	2,124
500	4903,4	0	0	0,275	0,000	2,124
1000	9806,7	0	0	0,550	0,000	2,124
1500	14710,1	2,5	1,25	0,825	0,617	2,741
2000	19613,4	5	2,5	1,101	1,235	3,359
2500	24516,8	7,5	3,75	1,376	1,852	3,976
3000	29420,1	12,5	6,25	1,651	3,086	5,210
3500	34323,5	15	7,5	1,926	3,704	5,828
4000	39226,8	20	10	2,201	4,938	7,062
4500	44130,2	22,5	11,25	2,476	5,556	7,680
5000	49033,5	27,5	13,75	2,752	6,790	8,914
5500	53936,9	30	15	3,027	7,407	9,531
6000	58840,2	32,5	16,25	3,302	8,025	10,149
6500	63743,6	37,5	18,75	3,577	9,259	11,383
7000	68646,9	40	20	3,852	9,877	12,001
7500	73550,3	42,5	21,25	4,127	10,494	12,618
8000	78453,6	47,5	23,75	4,402	11,728	13,852
8500	83357,0	50	25	4,678	12,346	14,470
9000	88260,3	55	27,5	4,953	13,580	15,704
9500	93163,7	60	30	5,228	14,815	16,939
10000	98067,0	62,5	31,25	5,503	15,432	17,556
10500	102970,4	65	32,5	5,778	16,049	18,173
11000	107873,7	70	35	6,053	17,284	19,408
11500	112777,1	72,5	36,25	6,329	17,901	20,025
12000	117680,4	77,5	38,75	6,604	19,136	21,260
12500	122583,8	82,5	41,25	6,879	20,370	22,494
13000	127487,1	85	42,5	7,154	20,988	23,112
13500	132390,5	90	45	7,429	22,222	24,346
14000	137293,8	92,5	46,25	7,704	22,840	24,964
14500	142197,2	97,5	48,75	7,980	24,074	26,198
15000	147100,5	100	50	8,255	24,691	26,815
15500	152003,9	102,5	51,25	8,530	25,309	27,433
16000	156907,2	110	55	8,805	27,160	29,284
16500	161810,6	112,5	56,25	9,080	27,778	29,902





Kode Beton = BS-0,5% 1,5M a

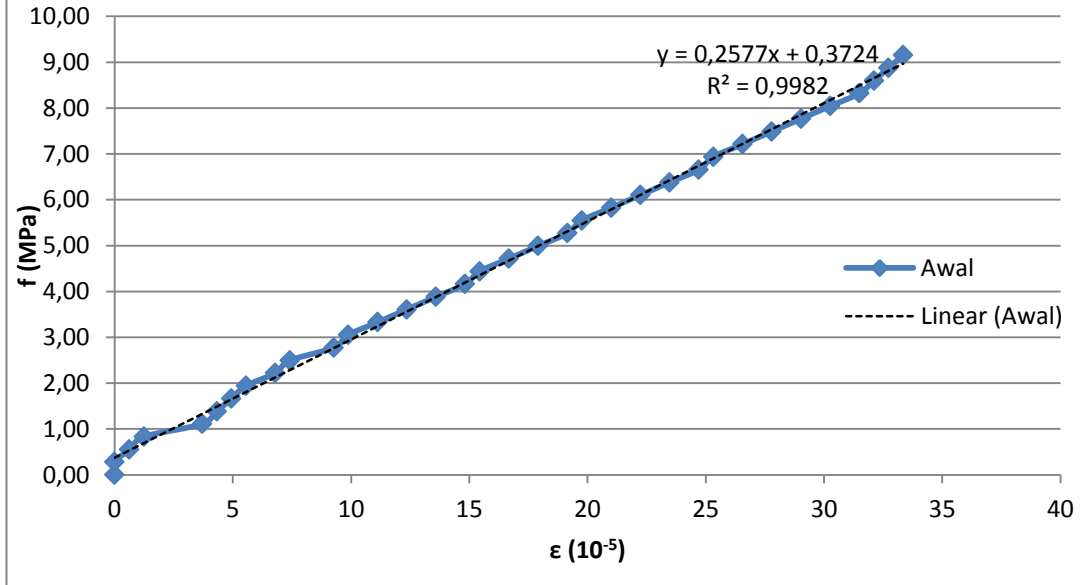
Po = 202,4 mm E = 26305,423 MPa

Ao = 17678,571 mm<sup>2</sup> Beban Maks = 16500 Kgf

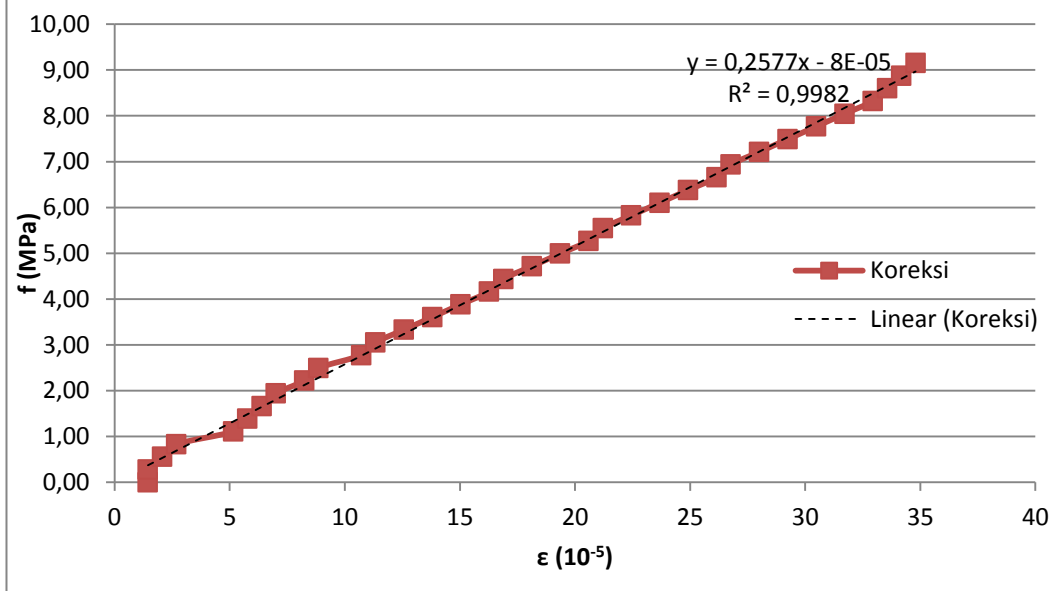
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	1,445
500	4903,4	0	0	0,277	0,000	1,445
1000	9806,7	2,5	1,25	0,555	0,618	2,063
1500	14710,1	5	2,5	0,832	1,235	2,680
2000	19613,4	15	7,5	1,109	3,706	5,151
2500	24516,8	17,5	8,75	1,387	4,323	5,768
3000	29420,1	20	10	1,664	4,941	6,386
3500	34323,5	22,5	11,25	1,942	5,558	7,003
4000	39226,8	27,5	13,75	2,219	6,793	8,238
4500	44130,2	30	15	2,496	7,411	8,856
5000	49033,5	37,5	18,75	2,774	9,264	10,709
5500	53936,9	40	20	3,051	9,881	11,326
6000	58840,2	45	22,5	3,328	11,117	12,562
6500	63743,6	50	25	3,606	12,352	13,797
7000	68646,9	55	27,5	3,883	13,587	15,032
7500	73550,3	60	30	4,160	14,822	16,267
8000	78453,6	62,5	31,25	4,438	15,440	16,885
8500	83357,0	67,5	33,75	4,715	16,675	18,120
9000	88260,3	72,5	36,25	4,993	17,910	19,355
9500	93163,7	77,5	38,75	5,270	19,145	20,590
10000	98067,0	80	40	5,547	19,763	21,208
10500	102970,4	85	42,5	5,825	20,998	22,443
11000	107873,7	90	45	6,102	22,233	23,678
11500	112777,1	95	47,5	6,379	23,468	24,913
12000	117680,4	100	50	6,657	24,704	26,149
12500	122583,8	102,5	51,25	6,934	25,321	26,766
13000	127487,1	107,5	53,75	7,211	26,556	28,001
13500	132390,5	112,5	56,25	7,489	27,792	29,237
14000	137293,8	117,5	58,75	7,766	29,027	30,472
14500	142197,2	122,5	61,25	8,043	30,262	31,707
15000	147100,5	127,5	63,75	8,321	31,497	32,942
15500	152003,9	130	65	8,598	32,115	33,560
16000	156907,2	132,5	66,25	8,876	32,732	34,177
16500	161810,6	135	67,5	9,153	33,350	34,795



### Grafik Modulus Elastisitas BS-0,5% 1,5M a



### Grafik Koreksi Modulus Elastisitas BS-0,5% 1,5M a



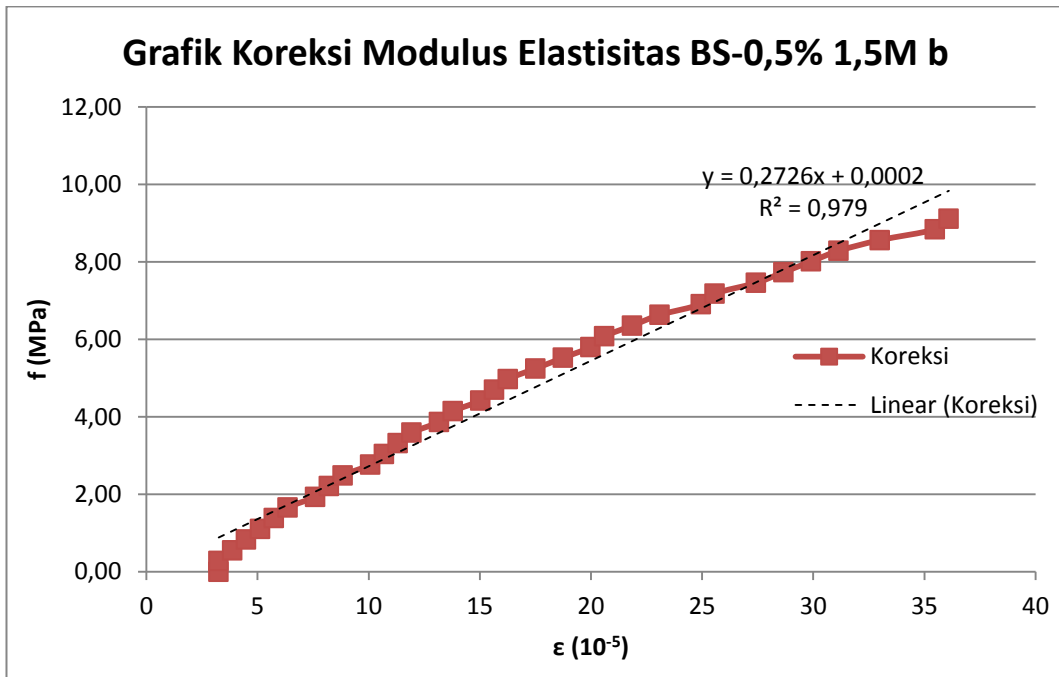
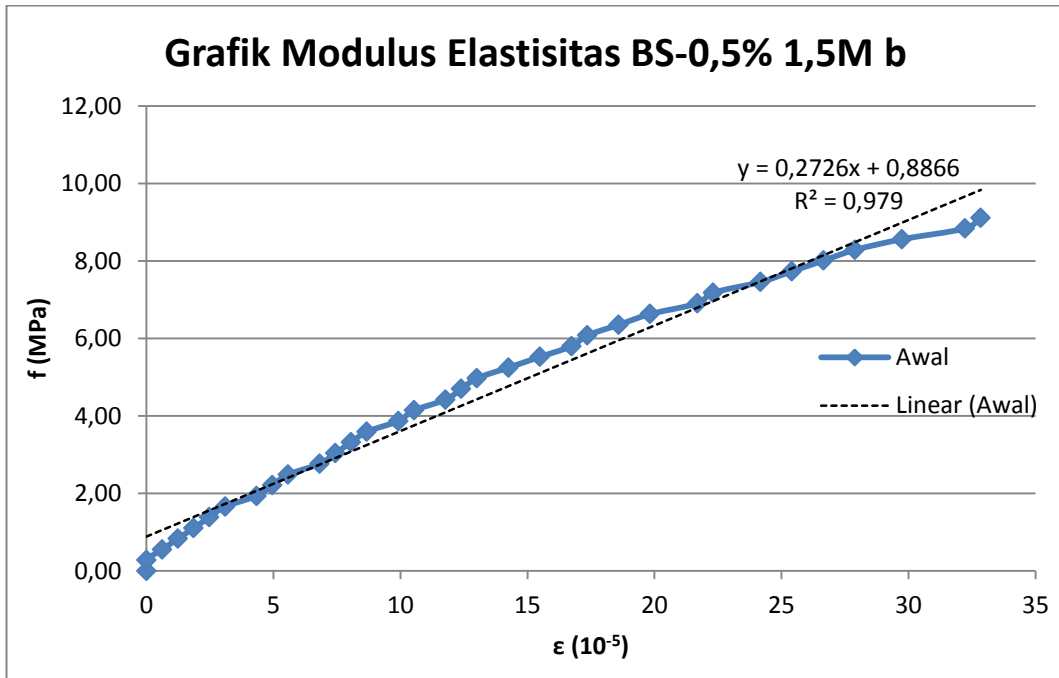


Kode Beton = BS-0,5% 1,5M b

Po = 201,7 mm E = 25254,767 MPa

Ao = 17749,356 mm<sup>2</sup> Beban Maks = 16500 Kgf

Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0,000	3,252
500	4903,4	0	0	0,276	0,000	3,252
1000	9806,7	2,5	1,25	0,553	0,620	3,872
1500	14710,1	5	2,5	0,829	1,239	4,491
2000	19613,4	7,5	3,75	1,105	1,859	5,111
2500	24516,8	10	5	1,381	2,479	5,731
3000	29420,1	12,5	6,25	1,658	3,099	6,351
3500	34323,5	17,5	8,75	1,934	4,338	7,590
4000	39226,8	20	10	2,210	4,958	8,210
4500	44130,2	22,5	11,25	2,486	5,578	8,830
5000	49033,5	27,5	13,75	2,763	6,817	10,069
5500	53936,9	30	15	3,039	7,437	10,689
6000	58840,2	32,5	16,25	3,315	8,057	11,309
6500	63743,6	35	17,5	3,591	8,676	11,928
7000	68646,9	40	20	3,868	9,916	13,168
7500	73550,3	42,5	21,25	4,144	10,535	13,787
8000	78453,6	47,5	23,75	4,420	11,775	15,027
8500	83357,0	50	25	4,696	12,395	15,647
9000	88260,3	52,5	26,25	4,973	13,014	16,266
9500	93163,7	57,5	28,75	5,249	14,254	17,506
10000	98067,0	62,5	31,25	5,525	15,493	18,745
10500	102970,4	67,5	33,75	5,801	16,733	19,985
11000	107873,7	70	35	6,078	17,353	20,605
11500	112777,1	75	37,5	6,354	18,592	21,844
12000	117680,4	80	40	6,630	19,831	23,083
12500	122583,8	87,5	43,75	6,906	21,691	24,943
13000	127487,1	90	45	7,183	22,310	25,562
13500	132390,5	97,5	48,75	7,459	24,170	27,422
14000	137293,8	102,5	51,25	7,735	25,409	28,661
14500	142197,2	107,5	53,75	8,011	26,648	29,900
15000	147100,5	112,5	56,25	8,288	27,888	31,140
15500	152003,9	120	60	8,564	29,747	32,999
16000	156907,2	130	65	8,840	32,226	35,478
16500	161810,6	132,5	66,25	9,116	32,846	36,098





Kode Beton = BS-0,5% 1,5M f

Po = 202,2 mm E = 28424,343 MPa

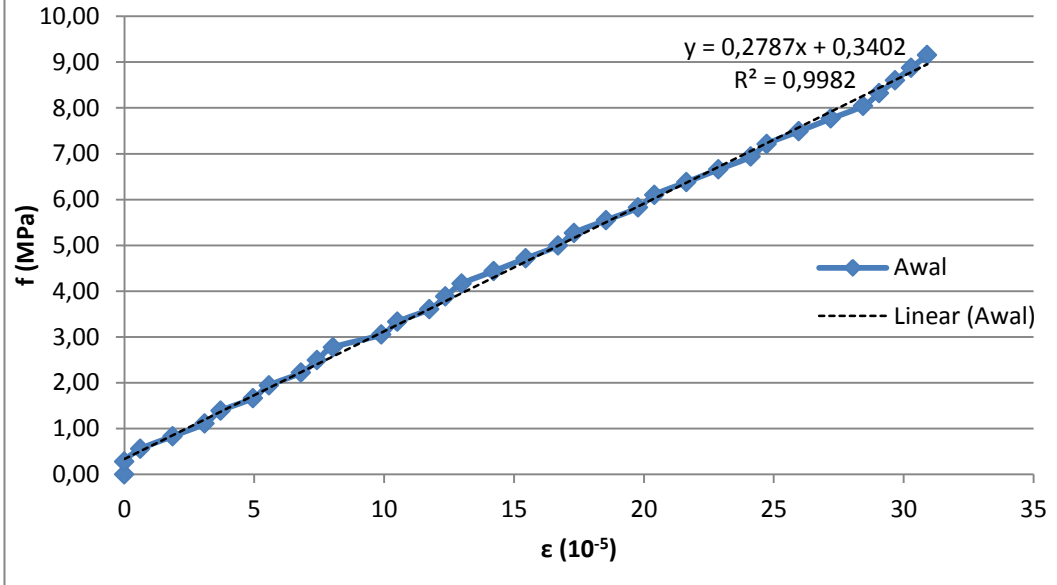
Ao = 17678,571 mm<sup>2</sup> Beban Maks = 16500 Kgf

Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0,000	1,291
500	4903,4	0	0	0,277	0,000	1,291
1000	9806,7	2,5	1,25	0,555	0,618	1,909
1500	14710,1	7,5	3,75	0,832	1,855	3,146
2000	19613,4	12,5	6,25	1,109	3,091	4,382
2500	24516,8	15	7,5	1,387	3,709	5,000
3000	29420,1	20	10	1,664	4,946	6,237
3500	34323,5	22,5	11,25	1,942	5,564	6,855
4000	39226,8	27,5	13,75	2,219	6,800	8,091
4500	44130,2	30	15	2,496	7,418	8,709
5000	49033,5	32,5	16,25	2,774	8,037	9,328
5500	53936,9	40	20	3,051	9,891	11,182
6000	58840,2	42,5	21,25	3,328	10,509	11,800
6500	63743,6	47,5	23,75	3,606	11,746	13,037
7000	68646,9	50	25	3,883	12,364	13,655
7500	73550,3	52,5	26,25	4,160	12,982	14,273
8000	78453,6	57,5	28,75	4,438	14,219	15,510
8500	83357,0	62,5	31,25	4,715	15,455	16,746
9000	88260,3	67,5	33,75	4,993	16,691	17,982
9500	93163,7	70	35	5,270	17,310	18,601
10000	98067,0	75	37,5	5,547	18,546	19,837
10500	102970,4	80	40	5,825	19,782	21,073
11000	107873,7	82,5	41,25	6,102	20,401	21,692
11500	112777,1	87,5	43,75	6,379	21,637	22,928
12000	117680,4	92,5	46,25	6,657	22,873	24,164
12500	122583,8	97,5	48,75	6,934	24,110	25,401
13000	127487,1	100	50	7,211	24,728	26,019
13500	132390,5	105	52,5	7,489	25,964	27,255
14000	137293,8	110	55	7,766	27,201	28,492
14500	142197,2	115	57,5	8,043	28,437	29,728
15000	147100,5	117,5	58,75	8,321	29,055	30,346
15500	152003,9	120	60	8,598	29,674	30,965
16000	156907,2	122,5	61,25	8,876	30,292	31,583
16500	161810,6	125	62,5	9,153	30,910	32,201

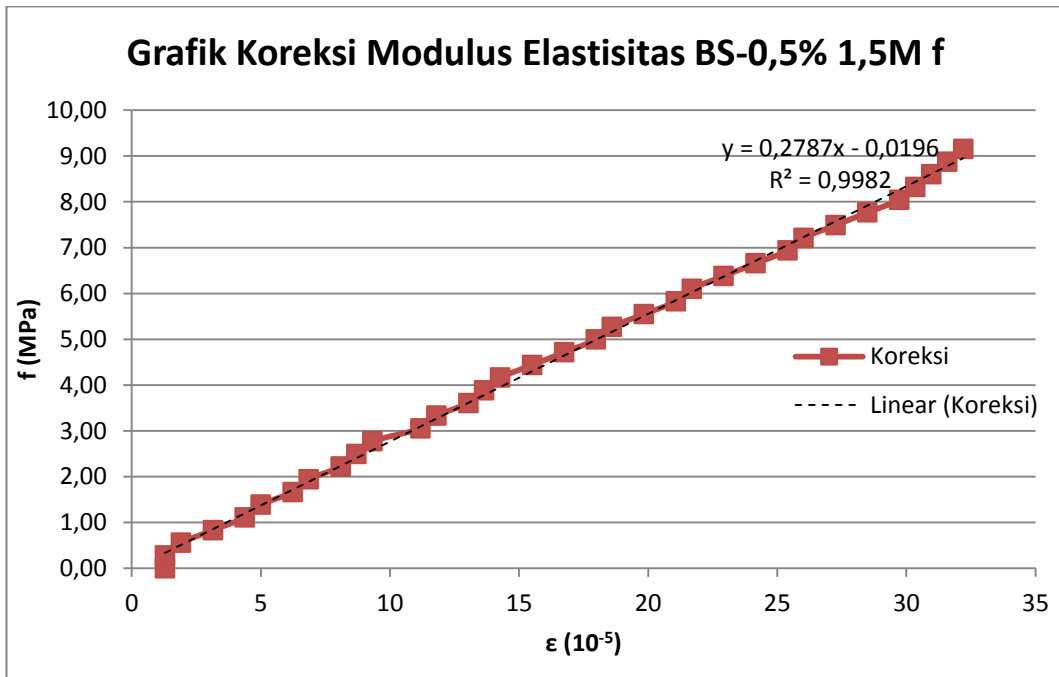




### Grafik Modulus Elastisitas BS-0,5% 1,5M f



### Grafik Koreksi Modulus Elastisitas BS-0,5% 1,5M f





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Kode Beton = BS-1% 1,5M b

Po = 201,7mm

E = 25888,415 MPa

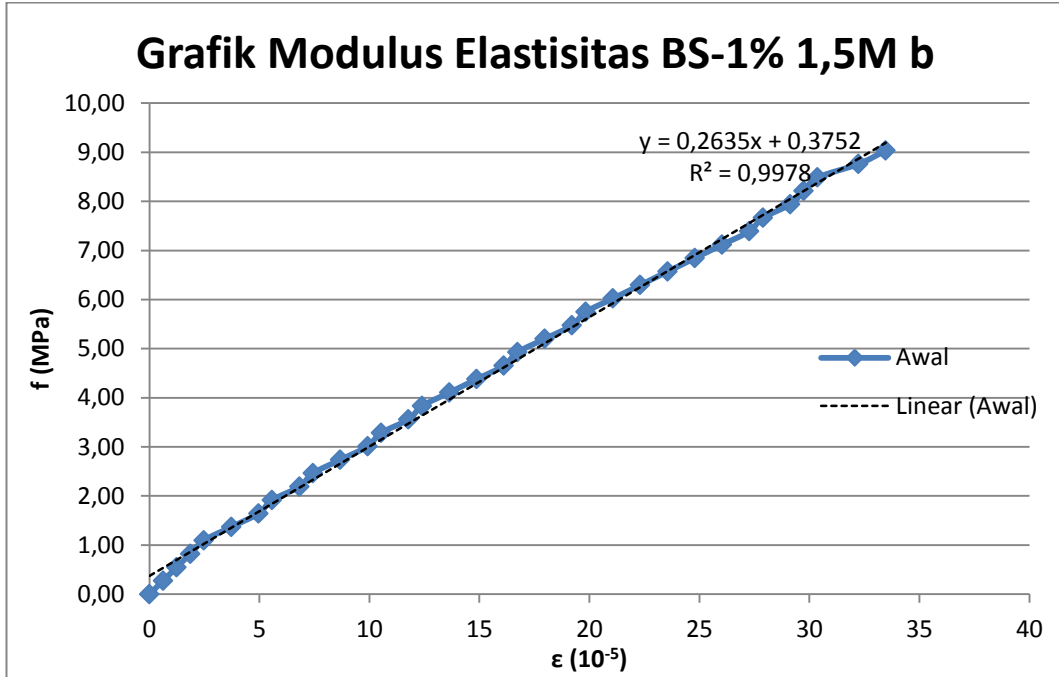
Ao = 17915,071 mm<sup>2</sup>

Beban Maks = 16500 Kgf

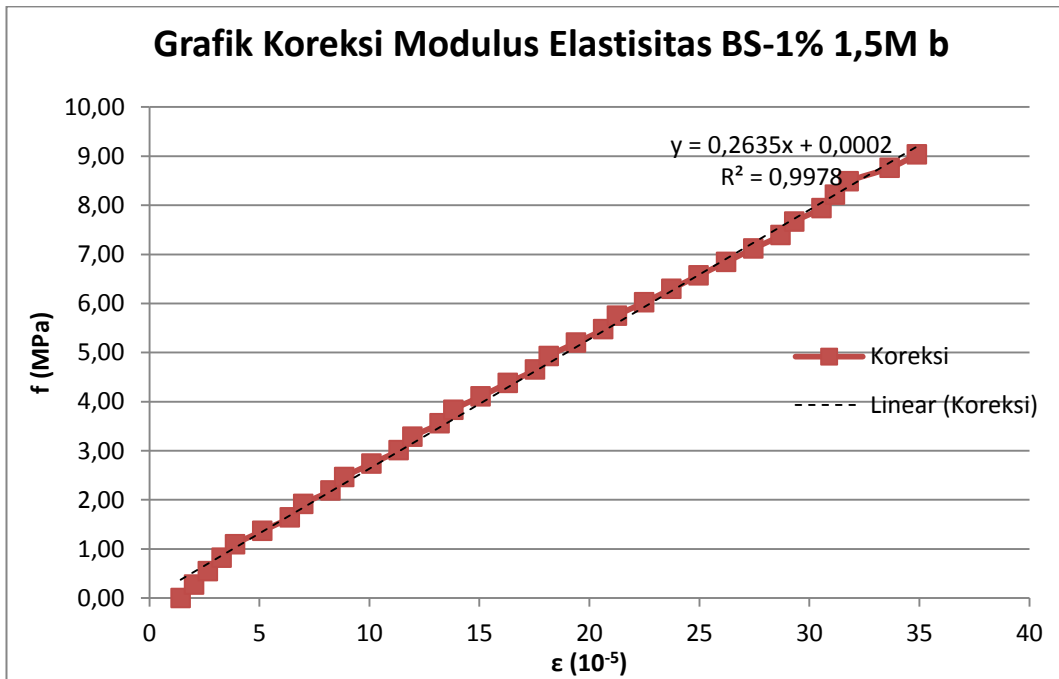
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	1,423
500	4903,4	2,5	1,25	0,274	0,620	2,043
1000	9806,7	5	2,5	0,547	1,239	2,662
1500	14710,1	7,5	3,75	0,821	1,859	3,282
2000	19613,4	10	5	1,095	2,479	3,902
2500	24516,8	15	7,5	1,368	3,718	5,141
3000	29420,1	20	10	1,642	4,958	6,381
3500	34323,5	22,5	11,25	1,916	5,578	7,001
4000	39226,8	27,5	13,75	2,190	6,817	8,240
4500	44130,2	30	15	2,463	7,437	8,860
5000	49033,5	35	17,5	2,737	8,676	10,099
5500	53936,9	40	20	3,011	9,916	11,339
6000	58840,2	42,5	21,25	3,284	10,535	11,958
6500	63743,6	47,5	23,75	3,558	11,775	13,198
7000	68646,9	50	25	3,832	12,395	13,818
7500	73550,3	55	27,5	4,105	13,634	15,057
8000	78453,6	60	30	4,379	14,874	16,297
8500	83357,0	65	32,5	4,653	16,113	17,536
9000	88260,3	67,5	33,75	4,927	16,733	18,156
9500	93163,7	72,5	36,25	5,200	17,972	19,395
10000	98067,0	77,5	38,75	5,474	19,212	20,635
10500	102970,4	80	40	5,748	19,831	21,254
11000	107873,7	85	42,5	6,021	21,071	22,494
11500	112777,1	90	45	6,295	22,310	23,733
12000	117680,4	95	47,5	6,569	23,550	24,973
12500	122583,8	100	50	6,842	24,789	26,212
13000	127487,1	105	52,5	7,116	26,029	27,452
13500	132390,5	110	55	7,390	27,268	28,691
14000	137293,8	112,5	56,25	7,664	27,888	29,311
14500	142197,2	117,5	58,75	7,937	29,127	30,550
15000	147100,5	120	60	8,211	29,747	31,170
15500	152003,9	122,5	61,25	8,485	30,367	31,790
16000	156907,2	130	65	8,758	32,226	33,649
16500	161810,6	135	67,5	9,032	33,466	34,889



### Grafik Modulus Elastisitas BS-1% 1,5M b



### Grafik Koreksi Modulus Elastisitas BS-1% 1,5M b





Kode Beton = BS-1% 1,5M e

Po = 202,2 mm

E = 29957,358 MPa

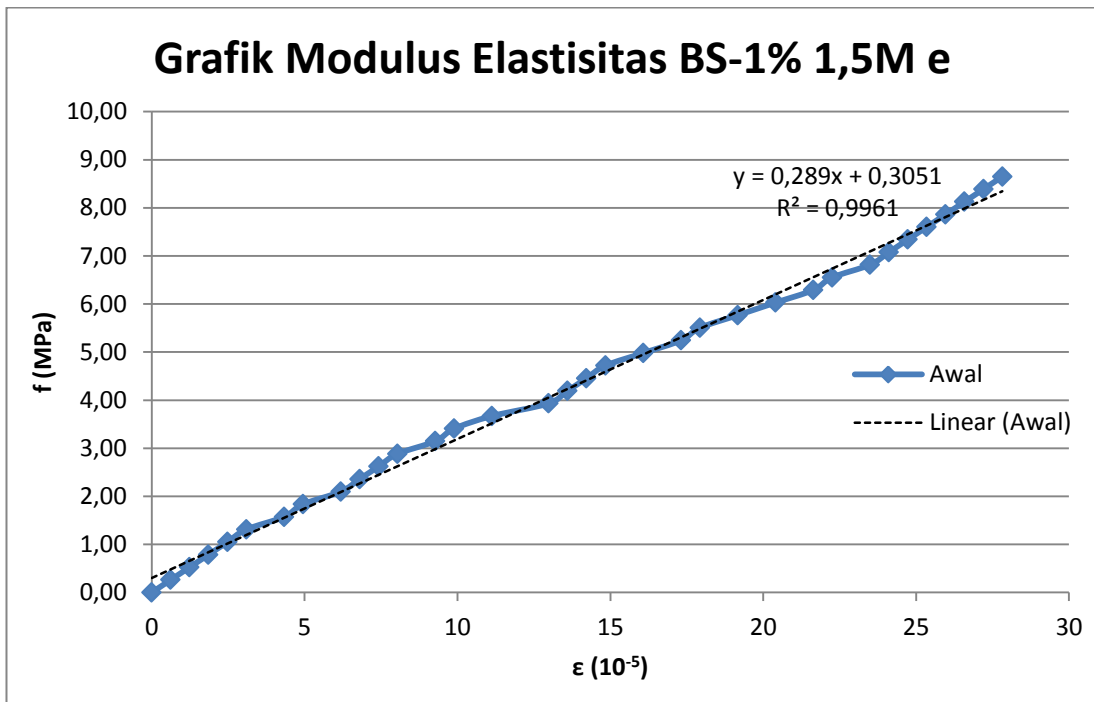
Ao = 18706,671 mm<sup>2</sup>

Beban Maks = 16500 Kgf

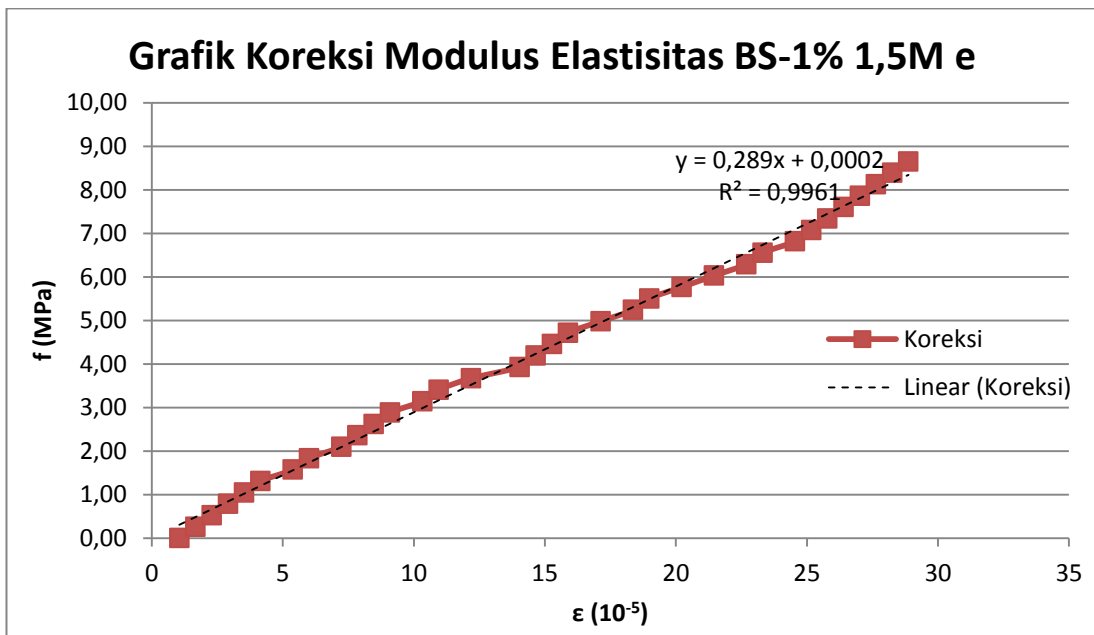
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	1,055
500	4903,4	2,5	1,25	0,262	0,618	1,673
1000	9806,7	5	2,5	0,524	1,236	2,291
1500	14710,1	7,5	3,75	0,786	1,855	2,910
2000	19613,4	10	5	1,048	2,473	3,528
2500	24516,8	12,5	6,25	1,311	3,091	4,146
3000	29420,1	17,5	8,75	1,573	4,327	5,382
3500	34323,5	20	10	1,835	4,946	6,001
4000	39226,8	25	12,5	2,097	6,182	7,237
4500	44130,2	27,5	13,75	2,359	6,800	7,855
5000	49033,5	30	15	2,621	7,418	8,473
5500	53936,9	32,5	16,25	2,883	8,037	9,092
6000	58840,2	37,5	18,75	3,145	9,273	10,328
6500	63743,6	40	20	3,408	9,891	10,946
7000	68646,9	45	22,5	3,670	11,128	12,183
7500	73550,3	52,5	26,25	3,932	12,982	14,037
8000	78453,6	55	27,5	4,194	13,600	14,655
8500	83357,0	57,5	28,75	4,456	14,219	15,274
9000	88260,3	60	30	4,718	14,837	15,892
9500	93163,7	65	32,5	4,980	16,073	17,128
10000	98067,0	70	35	5,242	17,310	18,365
10500	102970,4	72,5	36,25	5,504	17,928	18,983
11000	107873,7	77,5	38,75	5,767	19,164	20,219
11500	112777,1	82,5	41,25	6,029	20,401	21,456
12000	117680,4	87,5	43,75	6,291	21,637	22,692
12500	122583,8	90	45	6,553	22,255	23,310
13000	127487,1	95	47,5	6,815	23,492	24,547
13500	132390,5	97,5	48,75	7,077	24,110	25,165
14000	137293,8	100	50	7,339	24,728	25,783
14500	142197,2	102,5	51,25	7,601	25,346	26,401
15000	147100,5	105	52,5	7,864	25,964	27,019
15500	152003,9	107,5	53,75	8,126	26,583	27,638
16000	156907,2	110	55	8,388	27,201	28,256
16500	161810,6	112,5	56,25	8,650	27,819	28,874



### Grafik Modulus Elastisitas BS-1% 1,5M e



### Grafik Koreksi Modulus Elastisitas BS-1% 1,5M e





Kode Beton = BS-1% 1,5M f

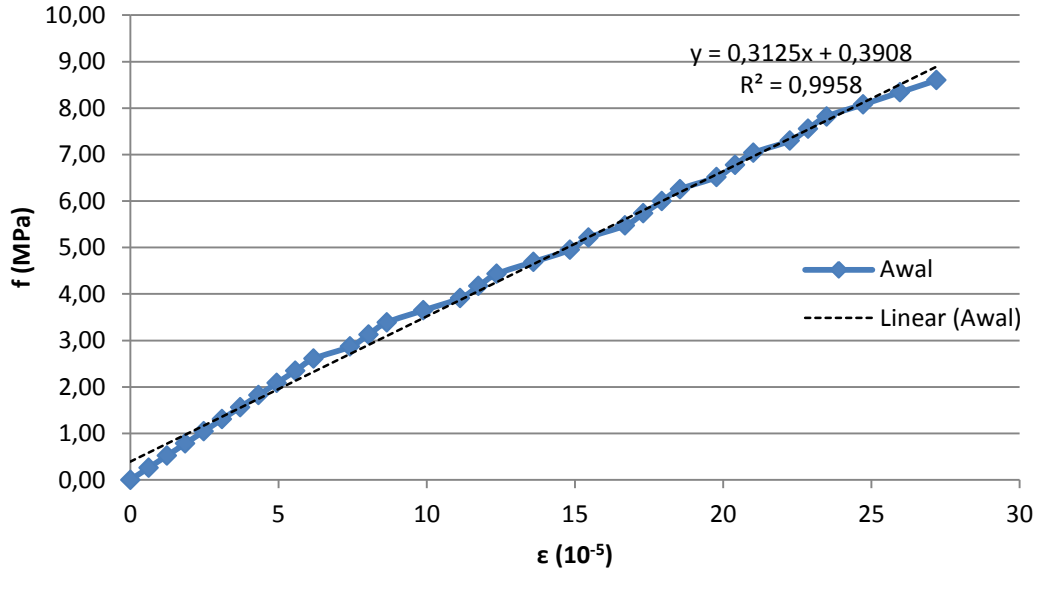
Po = 202,2 mm E = 30226,407 MPa

Ao = 18815,942 mm<sup>2</sup> Beban Maks = 16500 Kgf

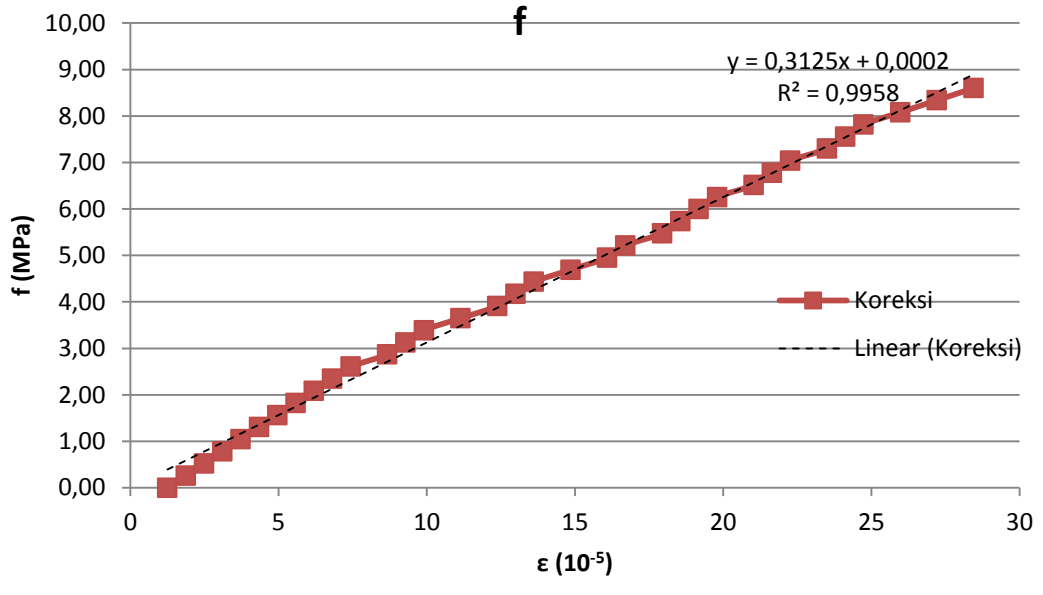
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	1,250
500	4903,4	2,5	1,25	0,261	0,618	1,868
1000	9806,7	5	2,5	0,521	1,236	2,486
1500	14710,1	7,5	3,75	0,782	1,855	3,105
2000	19613,4	10	5	1,042	2,473	3,723
2500	24516,8	12,5	6,25	1,303	3,091	4,341
3000	29420,1	15	7,5	1,564	3,709	4,959
3500	34323,5	17,5	8,75	1,824	4,327	5,577
4000	39226,8	20	10	2,085	4,946	6,196
4500	44130,2	22,5	11,25	2,345	5,564	6,814
5000	49033,5	25	12,5	2,606	6,182	7,432
5500	53936,9	30	15	2,867	7,418	8,668
6000	58840,2	32,5	16,25	3,127	8,037	9,287
6500	63743,6	35	17,5	3,388	8,655	9,905
7000	68646,9	40	20	3,648	9,891	11,141
7500	73550,3	45	22,5	3,909	11,128	12,378
8000	78453,6	47,5	23,75	4,170	11,746	12,996
8500	83357,0	50	25	4,430	12,364	13,614
9000	88260,3	55	27,5	4,691	13,600	14,850
9500	93163,7	60	30	4,951	14,837	16,087
10000	98067,0	62,5	31,25	5,212	15,455	16,705
10500	102970,4	67,5	33,75	5,473	16,691	17,941
11000	107873,7	70	35	5,733	17,310	18,560
11500	112777,1	72,5	36,25	5,994	17,928	19,178
12000	117680,4	75	37,5	6,254	18,546	19,796
12500	122583,8	80	40	6,515	19,782	21,032
13000	127487,1	82,5	41,25	6,775	20,401	21,651
13500	132390,5	85	42,5	7,036	21,019	22,269
14000	137293,8	90	45	7,297	22,255	23,505
14500	142197,2	92,5	46,25	7,557	22,873	24,123
15000	147100,5	95	47,5	7,818	23,492	24,742
15500	152003,9	100	50	8,078	24,728	25,978
16000	156907,2	105	52,5	8,339	25,964	27,214
16500	161810,6	110	55	8,600	27,201	28,451



### Grafik Modulus Elastisitas BS-1% 1,5M f



### Grafik Koreksi Modulus Elastisitas BS-1% 1,5M





Kode Beton = BS-0,5% 1,75M a

Po = 202,7 mm

E = 25541,478 MPa

Ao = 17915,071 mm<sup>2</sup>

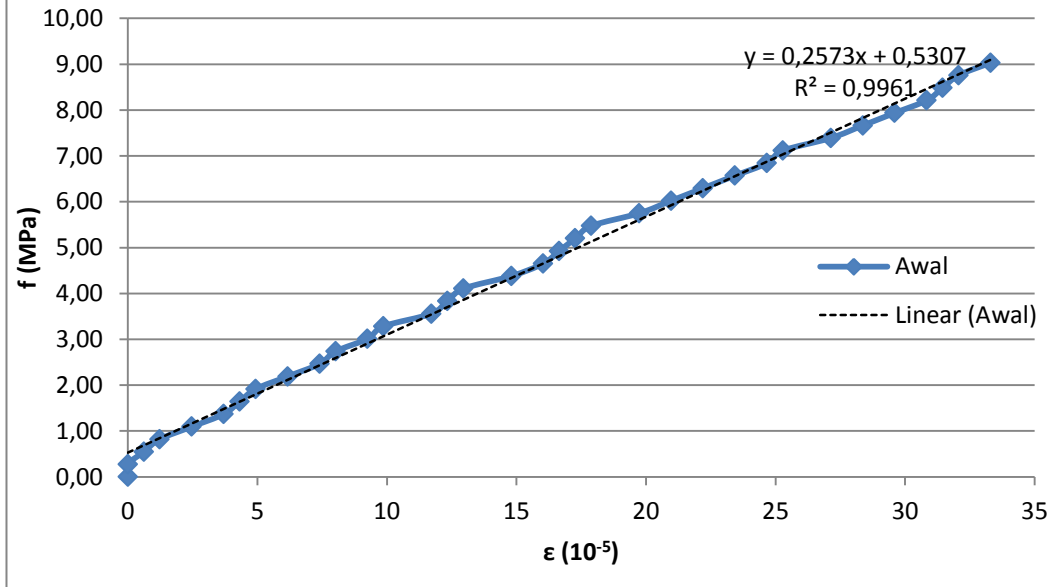
Beban Maks = 16500 Kgf

Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	2,062
500	4903,4	0	0	0,274	0,000	2,062
1000	9806,7	2,5	1,25	0,547	0,617	2,679
1500	14710,1	5	2,5	0,821	1,233	3,295
2000	19613,4	10	5	1,095	2,467	4,529
2500	24516,8	15	7,5	1,368	3,700	5,762
3000	29420,1	17,5	8,75	1,642	4,317	6,379
3500	34323,5	20	10	1,916	4,933	6,995
4000	39226,8	25	12,5	2,190	6,167	8,229
4500	44130,2	30	15	2,463	7,400	9,462
5000	49033,5	32,5	16,25	2,737	8,017	10,079
5500	53936,9	37,5	18,75	3,011	9,250	11,312
6000	58840,2	40	20	3,284	9,867	11,929
6500	63743,6	47,5	23,75	3,558	11,717	13,779
7000	68646,9	50	25	3,832	12,333	14,395
7500	73550,3	52,5	26,25	4,105	12,950	15,012
8000	78453,6	60	30	4,379	14,800	16,862
8500	83357,0	65	32,5	4,653	16,034	18,096
9000	88260,3	67,5	33,75	4,927	16,650	18,712
9500	93163,7	70	35	5,200	17,267	19,329
10000	98067,0	72,5	36,25	5,474	17,884	19,946
10500	102970,4	80	40	5,748	19,734	21,796
11000	107873,7	85	42,5	6,021	20,967	23,029
11500	112777,1	90	45	6,295	22,200	24,262
12000	117680,4	95	47,5	6,569	23,434	25,496
12500	122583,8	100	50	6,842	24,667	26,729
13000	127487,1	102,5	51,25	7,116	25,284	27,346
13500	132390,5	110	55	7,390	27,134	29,196
14000	137293,8	115	57,5	7,664	28,367	30,429
14500	142197,2	120	60	7,937	29,600	31,662
15000	147100,5	125	62,5	8,211	30,834	32,896
15500	152003,9	127,5	63,75	8,485	31,450	33,512
16000	156907,2	130	65	8,758	32,067	34,129
16500	161810,6	135	67,5	9,032	33,300	35,362

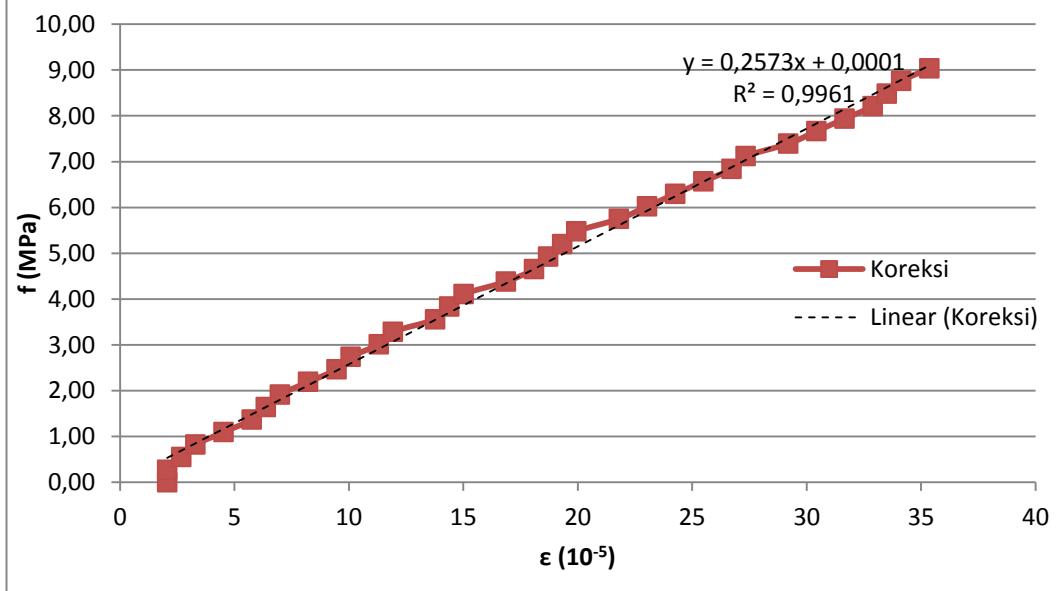




**Grafik Modulus Elastisitas BS-0,5% 1,75M a**



**Grafik Koreksi Modulus Elastisitas BS-0,5% 1,75M a**





Kode Beton = BS-0,5% 1,75M d

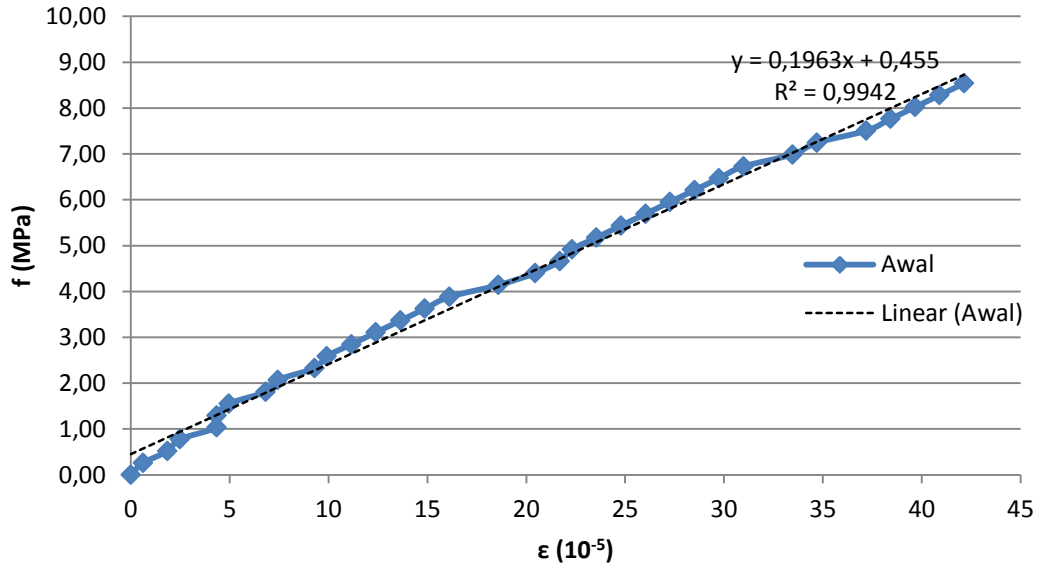
Po = 202,1 mm E = 19209,231 MPa

Ao = 18949,928 mm<sup>2</sup> Beban Maks = 16500 Kgf

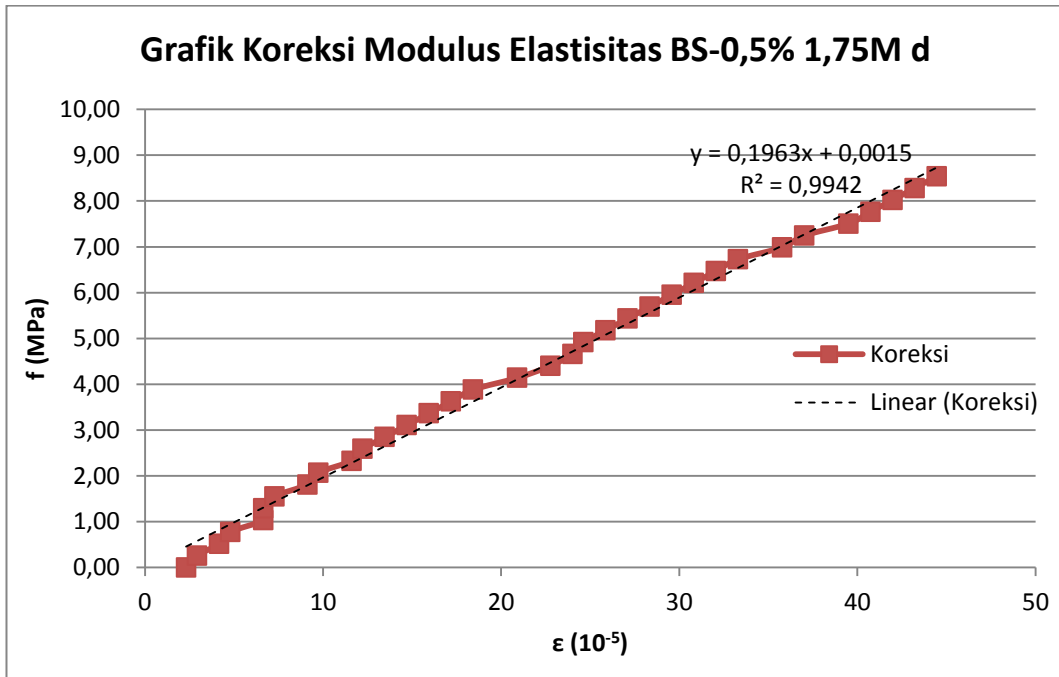
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	2,31
500	4903,4	2,5	1,25	0,259	0,620	2,930
1000	9806,7	7,5	3,75	0,518	1,859	4,169
1500	14710,1	10	5	0,776	2,479	4,789
2000	19613,4	17,5	8,75	1,035	4,338	6,648
2500	24516,8	17,5	8,75	1,294	4,338	6,648
3000	29420,1	20	10	1,553	4,958	7,268
3500	34323,5	27,5	13,75	1,811	6,817	9,127
4000	39226,8	30	15	2,070	7,437	9,747
4500	44130,2	37,5	18,75	2,329	9,296	11,606
5000	49033,5	40	20	2,588	9,916	12,226
5500	53936,9	45	22,5	2,846	11,155	13,465
6000	58840,2	50	25	3,105	12,395	14,705
6500	63743,6	55	27,5	3,364	13,634	15,944
7000	68646,9	60	30	3,623	14,874	17,184
7500	73550,3	65	32,5	3,881	16,113	18,423
8000	78453,6	75	37,5	4,140	18,592	20,902
8500	83357,0	82,5	41,25	4,399	20,451	22,761
9000	88260,3	87,5	43,75	4,658	21,691	24,001
9500	93163,7	90	45	4,916	22,310	24,620
10000	98067,0	95	47,5	5,175	23,550	25,860
10500	102970,4	100	50	5,434	24,789	27,099
11000	107873,7	105	52,5	5,693	26,029	28,339
11500	112777,1	110	55	5,951	27,268	29,578
12000	117680,4	115	57,5	6,210	28,508	30,818
12500	122583,8	120	60	6,469	29,747	32,057
13000	127487,1	125	62,5	6,728	30,987	33,297
13500	132390,5	135	67,5	6,986	33,466	35,776
14000	137293,8	140	70	7,245	34,705	37,015
14500	142197,2	150	75	7,504	37,184	39,494
15000	147100,5	155	77,5	7,763	38,423	40,733
15500	152003,9	160	80	8,021	39,663	41,973
16000	156907,2	165	82,5	8,280	40,902	43,212
16500	161810,6	170	85	8,539	42,142	44,452



**Garfik Modulus Elastisitas BS-0,5% 1,75M d**



**Grafik Koreksi Modulus Elastisitas BS-0,5% 1,75M d**



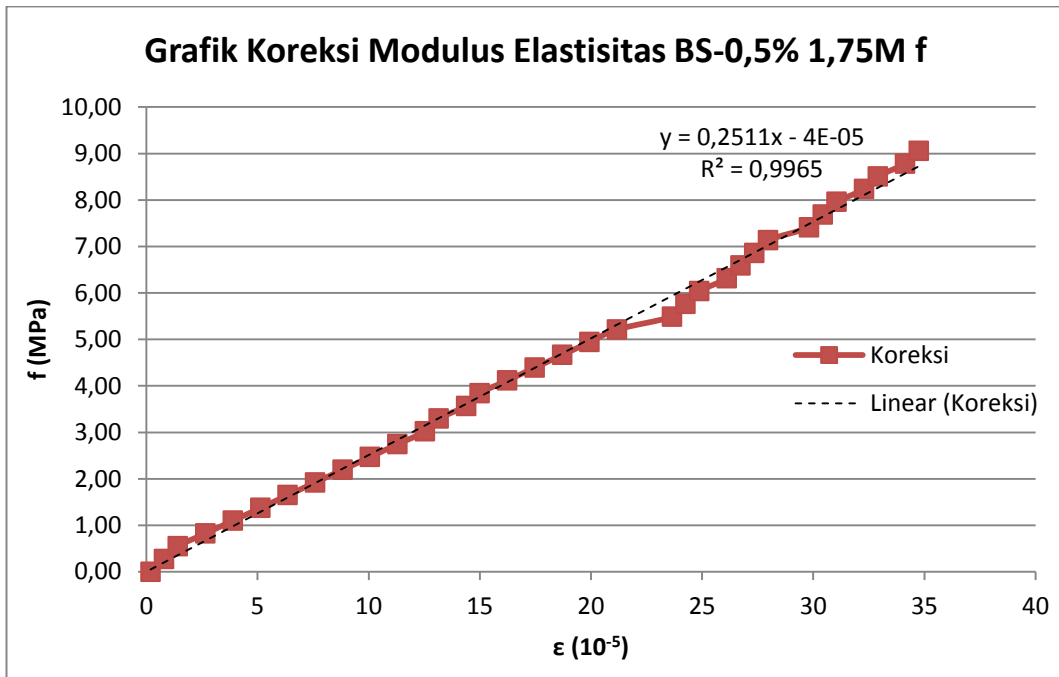
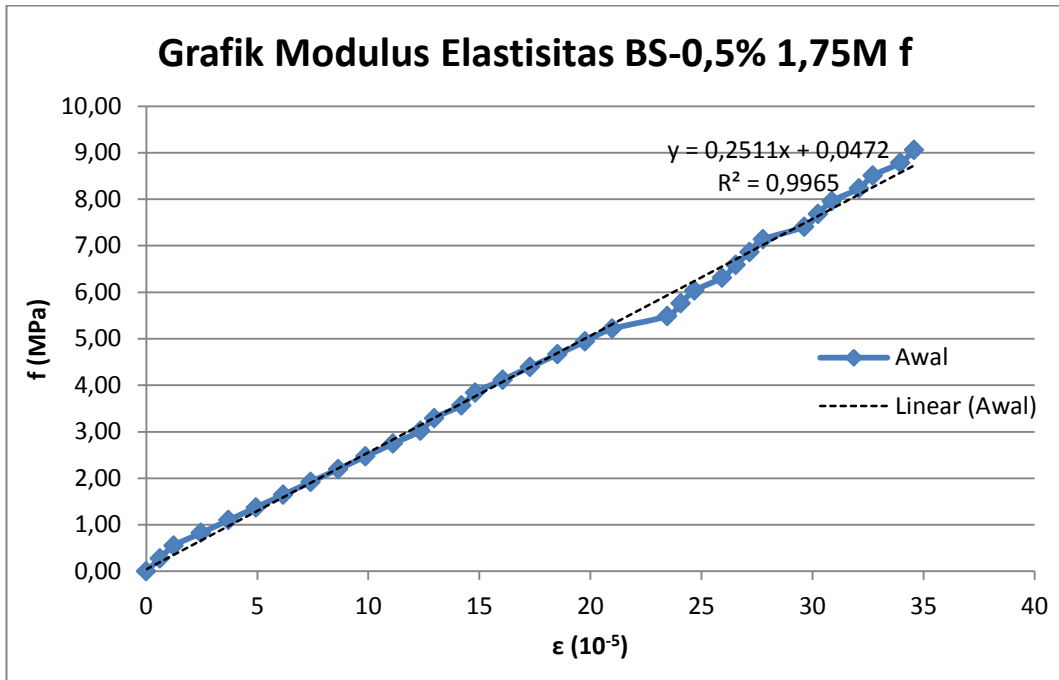


Kode Beton = BS-0,5% 1,75M f

Po = 202,5 mm E = 26056,192 MPa

Ao = 17867,646 mm<sup>2</sup> Beban Maks = 16500 Kgf

Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	0,188
500	4903,4	2,5	1,25	0,274	0,617	0,805
1000	9806,7	5	2,5	0,549	1,235	1,423
1500	14710,1	10	5	0,823	2,469	2,657
2000	19613,4	15	7,5	1,098	3,704	3,892
2500	24516,8	20	10	1,372	4,938	5,126
3000	29420,1	25	12,5	1,647	6,173	6,361
3500	34323,5	30	15	1,921	7,407	7,595
4000	39226,8	35	17,5	2,195	8,642	8,830
4500	44130,2	40	20	2,470	9,877	10,065
5000	49033,5	45	22,5	2,744	11,111	11,299
5500	53936,9	50	25	3,019	12,346	12,534
6000	58840,2	52,5	26,25	3,293	12,963	13,151
6500	63743,6	57,5	28,75	3,568	14,198	14,386
7000	68646,9	60	30	3,842	14,815	15,003
7500	73550,3	65	32,5	4,116	16,049	16,237
8000	78453,6	70	35	4,391	17,284	17,472
8500	83357,0	75	37,5	4,665	18,519	18,707
9000	88260,3	80	40	4,940	19,753	19,941
9500	93163,7	85	42,5	5,214	20,988	21,176
10000	98067,0	95	47,5	5,489	23,457	23,645
10500	102970,4	97,5	48,75	5,763	24,074	24,262
11000	107873,7	100	50	6,037	24,691	24,879
11500	112777,1	105	52,5	6,312	25,926	26,114
12000	117680,4	107,5	53,75	6,586	26,543	26,731
12500	122583,8	110	55	6,861	27,160	27,348
13000	127487,1	112,5	56,25	7,135	27,778	27,966
13500	132390,5	120	60	7,410	29,630	29,818
14000	137293,8	122,5	61,25	7,684	30,247	30,435
14500	142197,2	125	62,5	7,958	30,864	31,052
15000	147100,5	130	65	8,233	32,099	32,287
15500	152003,9	132,5	66,25	8,507	32,716	32,904
16000	156907,2	137,5	68,75	8,782	33,951	34,139
16500	161810,6	140	70	9,056	34,568	34,756





Kode Beton = BS-1% 1,75M c

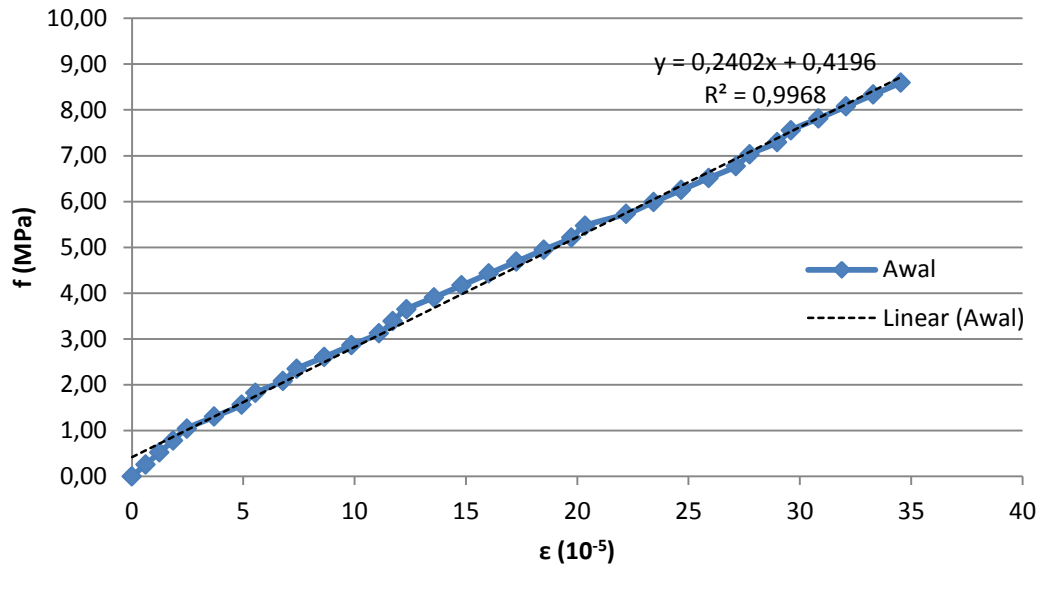
Po = 202,7 mm E = 23688,385 MPa

Ao = 18828,103 mm<sup>2</sup> Beban Maks = 16500 Kgf

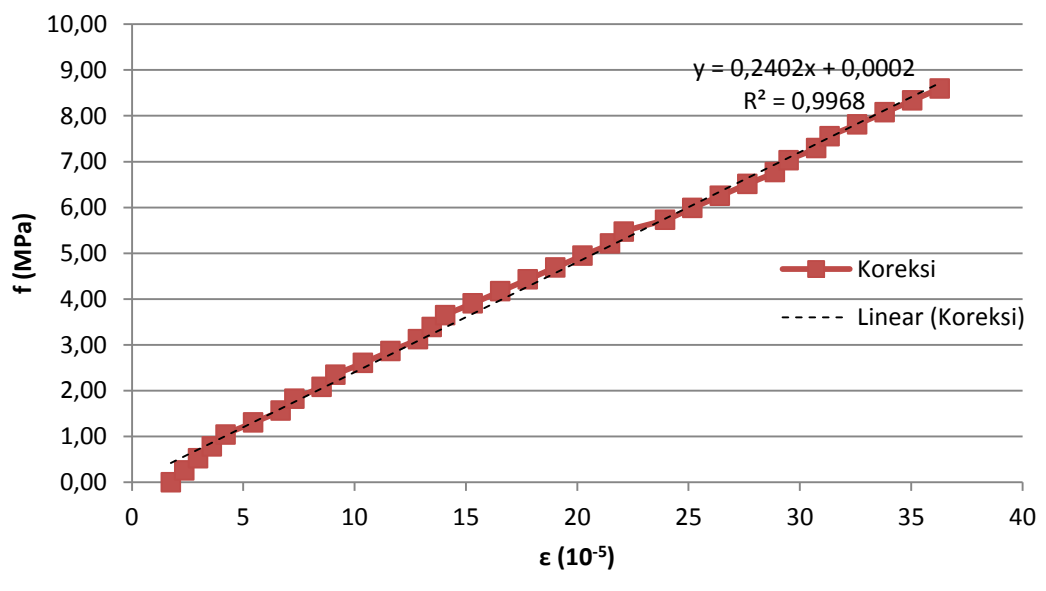
Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0	1,746
500	4903,4	2,5	1,25	0,260	0,617	2,363
1000	9806,7	5	2,5	0,521	1,233	2,979
1500	14710,1	7,5	3,75	0,781	1,850	3,596
2000	19613,4	10	5	1,042	2,467	4,213
2500	24516,8	15	7,5	1,302	3,700	5,446
3000	29420,1	20	10	1,563	4,933	6,679
3500	34323,5	22,5	11,25	1,823	5,550	7,296
4000	39226,8	27,5	13,75	2,083	6,783	8,529
4500	44130,2	30	15	2,344	7,400	9,146
5000	49033,5	35	17,5	2,604	8,633	10,379
5500	53936,9	40	20	2,865	9,867	11,613
6000	58840,2	45	22,5	3,125	11,100	12,846
6500	63743,6	47,5	23,75	3,386	11,717	13,463
7000	68646,9	50	25	3,646	12,333	14,079
7500	73550,3	55	27,5	3,906	13,567	15,313
8000	78453,6	60	30	4,167	14,800	16,546
8500	83357,0	65	32,5	4,427	16,034	17,780
9000	88260,3	70	35	4,688	17,267	19,013
9500	93163,7	75	37,5	4,948	18,500	20,246
10000	98067,0	80	40	5,209	19,734	21,480
10500	102970,4	82,5	41,25	5,469	20,350	22,096
11000	107873,7	90	45	5,729	22,200	23,946
11500	112777,1	95	47,5	5,990	23,434	25,180
12000	117680,4	100	50	6,250	24,667	26,413
12500	122583,8	105	52,5	6,511	25,900	27,646
13000	127487,1	110	55	6,771	27,134	28,880
13500	132390,5	112,5	56,25	7,032	27,750	29,496
14000	137293,8	117,5	58,75	7,292	28,984	30,730
14500	142197,2	120	60	7,552	29,600	31,346
15000	147100,5	125	62,5	7,813	30,834	32,580
15500	152003,9	130	65	8,073	32,067	33,813
16000	156907,2	135	67,5	8,334	33,300	35,046
16500	161810,6	140	70	8,594	34,534	36,280



### Grafik Modulus Elastisitas BS-1% 1,75M c



### Grafik Koreksi Modulus Elastisitas BS-1% 1,75M c





Kode Beton = BS-1% 1,75M e

Po = 201,5 mm E = 30667,25 MPa

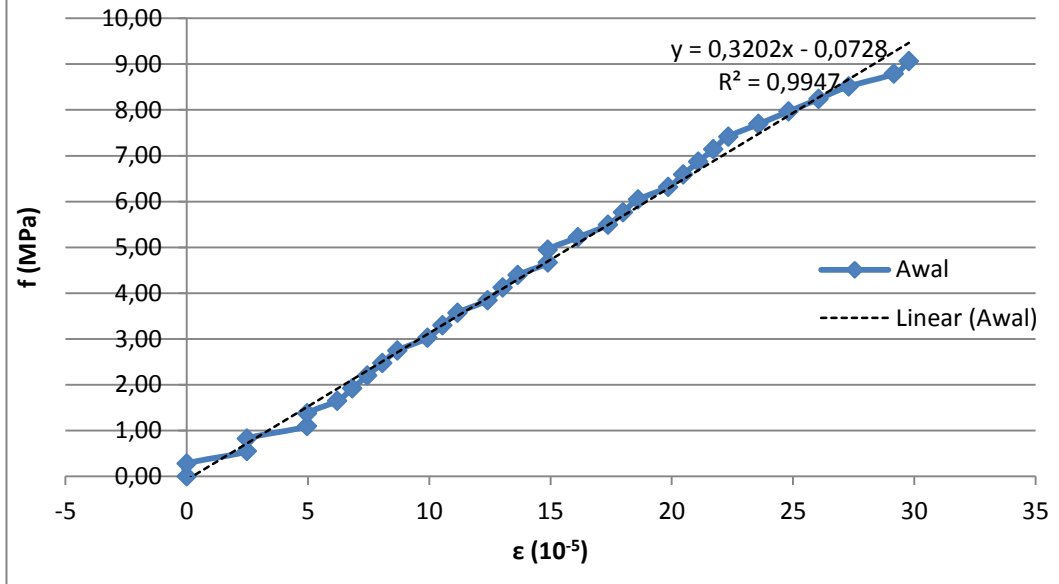
Ao = 17855,799 mm<sup>2</sup> Beban Maks = 16500 Kgf

Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0,000	-0,227
500	4903,4	0	0	0,275	0,000	-0,227
1000	9806,7	10	5	0,549	2,481	2,254
1500	14710,1	10	5	0,824	2,481	2,254
2000	19613,4	20	10	1,098	4,963	4,736
2500	24516,8	20	10	1,373	4,963	4,736
3000	29420,1	25	12,5	1,648	6,203	5,976
3500	34323,5	27,5	13,75	1,922	6,824	6,597
4000	39226,8	30	15	2,197	7,444	7,217
4500	44130,2	32,5	16,25	2,471	8,065	7,838
5000	49033,5	35	17,5	2,746	8,685	8,458
5500	53936,9	40	20	3,021	9,926	9,699
6000	58840,2	42,5	21,25	3,295	10,546	10,319
6500	63743,6	45	22,5	3,570	11,166	10,939
7000	68646,9	50	25	3,845	12,407	12,180
7500	73550,3	52,5	26,25	4,119	13,027	12,800
8000	78453,6	55	27,5	4,394	13,648	13,421
8500	83357,0	60	30	4,668	14,888	14,661
9000	88260,3	60	30	4,943	14,888	14,661
9500	93163,7	65	32,5	5,218	16,129	15,902
10000	98067,0	70	35	5,492	17,370	17,143
10500	102970,4	72,5	36,25	5,767	17,990	17,763
11000	107873,7	75	37,5	6,041	18,610	18,383
11500	112777,1	80	40	6,316	19,851	19,624
12000	117680,4	82,5	41,25	6,591	20,471	20,244
12500	122583,8	85	42,5	6,865	21,092	20,865
13000	127487,1	87,5	43,75	7,140	21,712	21,485
13500	132390,5	90	45	7,414	22,333	22,106
14000	137293,8	95	47,5	7,689	23,573	23,346
14500	142197,2	100	50	7,964	24,814	24,587
15000	147100,5	105	52,5	8,238	26,055	25,828
15500	152003,9	110	55	8,513	27,295	27,068
16000	156907,2	117,5	58,75	8,787	29,156	28,929
16500	161810,6	120	60	9,062	29,777	29,550

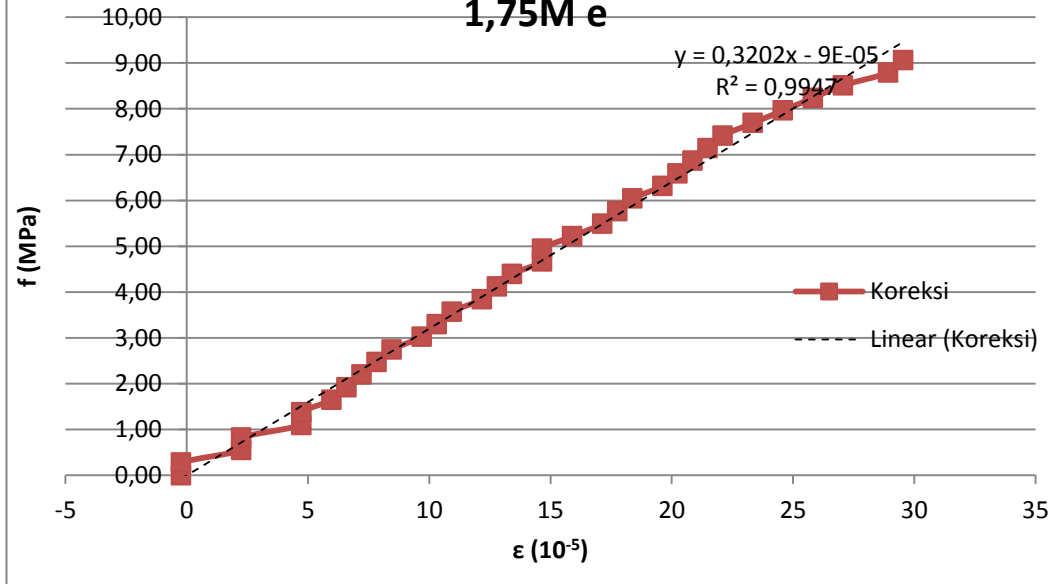




### Grafik Modulus Elastisitas BS-1% 1,75M e



### Grafik Koreksi Modulus Elastisitas BS-1% 1,75M e



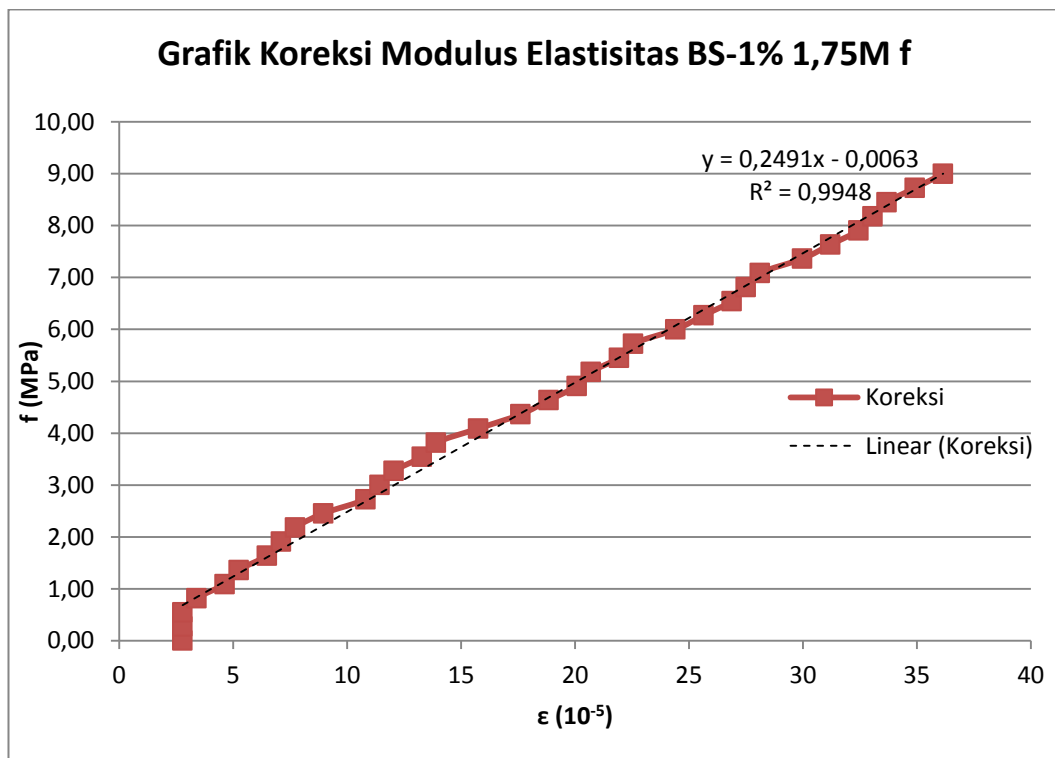
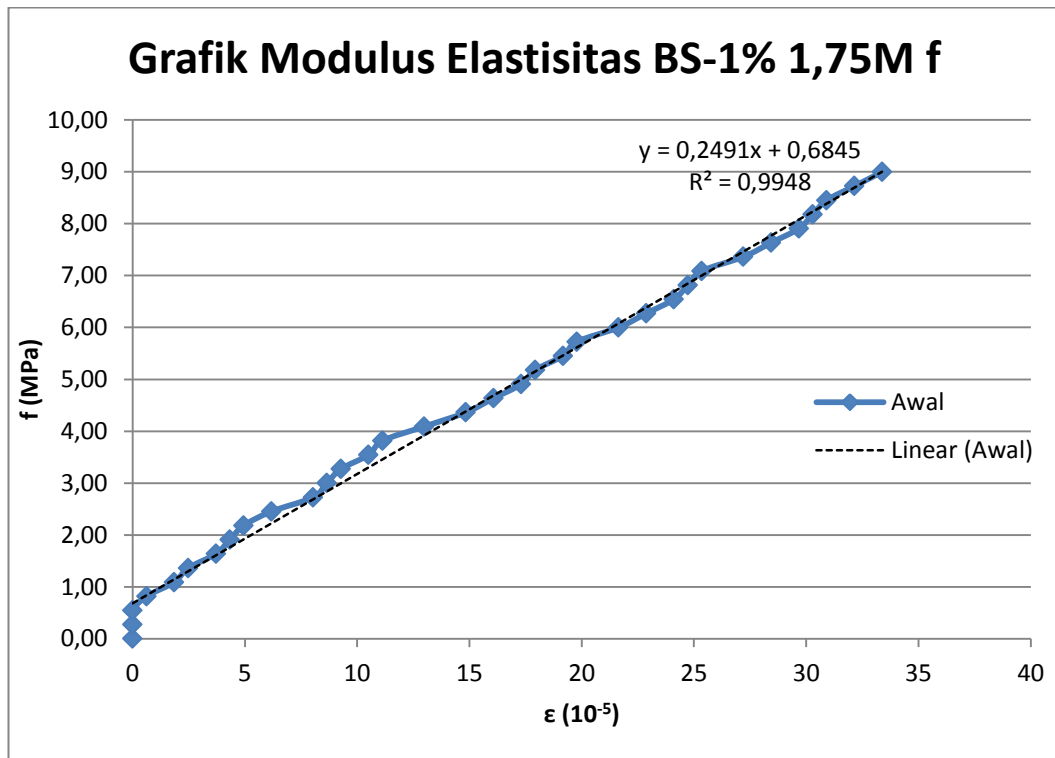


Kode Beton = BS-1% 1,75M e

Po = 202,2 mm E = 24882,069 MPa

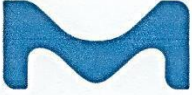
Ao = 17986,328 mm<sup>2</sup> Beban Maks = 16500 Kgf

Beban		Strainometer	Strainometer	Tegangan	Regangan	ε . Koreksi
Kgf	N	(ΔP)	(ΔP/2)	(MPa)	ε (10 <sup>-5</sup> )	ε (10 <sup>-5</sup> )
0	0	0	0	0,000	0,000	2,773
500	4903,4	0	0	0,273	0,000	2,773
1000	9806,7	0	0	0,545	0,000	2,773
1500	14710,1	2,5	1,25	0,818	0,618	3,391
2000	19613,4	7,5	3,75	1,090	1,855	4,628
2500	24516,8	10	5	1,363	2,473	5,246
3000	29420,1	15	7,5	1,636	3,709	6,482
3500	34323,5	17,5	8,75	1,908	4,327	7,100
4000	39226,8	20	10	2,181	4,946	7,719
4500	44130,2	25	12,5	2,454	6,182	8,955
5000	49033,5	32,5	16,25	2,726	8,037	10,810
5500	53936,9	35	17,5	2,999	8,655	11,428
6000	58840,2	37,5	18,75	3,271	9,273	12,046
6500	63743,6	42,5	21,25	3,544	10,509	13,282
7000	68646,9	45	22,5	3,817	11,128	13,901
7500	73550,3	52,5	26,25	4,089	12,982	15,755
8000	78453,6	60	30	4,362	14,837	17,610
8500	83357,0	65	32,5	4,634	16,073	18,846
9000	88260,3	70	35	4,907	17,310	20,083
9500	93163,7	72,5	36,25	5,180	17,928	20,701
10000	98067,0	77,5	38,75	5,452	19,164	21,937
10500	102970,4	80	40	5,725	19,782	22,555
11000	107873,7	87,5	43,75	5,998	21,637	24,410
11500	112777,1	92,5	46,25	6,270	22,873	25,646
12000	117680,4	97,5	48,75	6,543	24,110	26,883
12500	122583,8	100	50	6,815	24,728	27,501
13000	127487,1	102,5	51,25	7,088	25,346	28,119
13500	132390,5	110	55	7,361	27,201	29,974
14000	137293,8	115	57,5	7,633	28,437	31,210
14500	142197,2	120	60	7,906	29,674	32,447
15000	147100,5	122,5	61,25	8,178	30,292	33,065
15500	152003,9	125	62,5	8,451	30,910	33,683
16000	156907,2	130	65	8,724	32,146	34,919
16500	161810,6	135	67,5	8,996	33,383	36,156





D. *Certificate of Analysis* oleh Merck



## Certificate of Analysis

1.06498.1000 Sodium hydroxide pellets for analysis EMSURE® ISO  
Batch B1339998

	Spec. Values		Batch Values	
Assay (acidimetric, NaOH)	≥ 99.0	%	99.1	%
Carbonate (as Na <sub>2</sub> CO <sub>3</sub> )	≤ 1.0	%	0.3	%
Chloride (Cl)	≤ 0.0005	%	≤ 0.0005	%
Phosphate (PO <sub>4</sub> )	≤ 0.0005	%	≤ 0.0005	%
Silicate (SiO <sub>2</sub> )	≤ 0.001	%	≤ 0.001	%
Sulphate (SO <sub>4</sub> )	≤ 0.0005	%	≤ 0.0005	%
Total nitrogen (N)	≤ 0.0003	%	≤ 0.0003	%
Heavy metals (as Pb)	≤ 0.0005	%	≤ 0.0005	%
Al (Aluminium)	≤ 0.0005	%	≤ 0.0005	%
As (Arsenic)	≤ 0.0001	%	≤ 0.0001	%
Ca (Calcium)	≤ 0.0005	%	≤ 0.0005	%
Cu (Copper)	≤ 0.0002	%	≤ 0.0002	%
Fe (Iron)	≤ 0.0005	%	≤ 0.0005	%
K (Potassium)	≤ 0.05	%	0.008	%
Mg (Magnesium)	≤ 0.0005	%	≤ 0.0005	%
Ni (Nickel)	≤ 0.00025	%	≤ 0.00025	%
Pb (Lead)	≤ 0.0005	%	≤ 0.0005	%
Zn (Zinc)	≤ 0.001	%	≤ 0.001	%

Date of release (DD.MM.YYYY) 02.08.2016  
Minimum shelf life (DD.MM.YYYY) 31.07.2019

Dr. Andreas Lang  
Responsible laboratory manager quality control

This document has been produced electronically and is valid without a signature.

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SALSA Version 452761 /990000357239/ Date: 04.08.2016

Page 1 of 1



## E. Product Data Sheet SIKKA Viscocrete 1003



# PRODUCT DATA SHEET

## Sika® ViscoCrete®-1003

CONCRETE ADMIXTURE FOR HIGH FLOW / SELF-COMPACTING CONCRETE

### DESCRIPTION

Sika® Viscocrete®-1003 is a third generation superplasticiser for concrete and mortar. It is particularly developed for the production of high flow concrete with exceptional flow retention properties and significant reduction in bleeding and segregation.

### USES

Sika® Viscocrete®-1003 facilitates extreme water reduction, excellent flowability with optimal cohesion and strong self-compacting behaviour.

Sika® Viscocrete®-1003 is used for the following types of concrete:

- High flow concrete
- Self-compacting concrete (S.C.C.)
- Concrete with very high water reduction (up to 30 %)
- High strength concrete
- Concrete in hot weather and with extended transportation and workability requirements etc.

The combination of high water reduction, excellent flowability and high early strength provides clear benefits in the above mentioned applications.

### CHARACTERISTICS / ADVANTAGES

Sika® Viscocrete®-1003 acts by surface adsorption on the cement particles producing a sterical separation effect. Concrete produced with Sika® Viscocrete®-1003 exhibits the following properties:

- Excellent flowability (resulting in highly reduced placing and compacting efforts)
- Strong self-compacting behaviour
- Extremely high water reduction (resulting in high density and strengths)
- Improved shrinkage and creep behaviour
- Increased carbonation resistance of the concrete
- Improved finish
- Reduce tendency to bleeding and segregation

Sika® Viscocrete®-1003 does not contain chlorides or other ingredients which promotes steel corrosion. Therefore, it may be used without restriction for reinforced and pre-stressed concrete construction.

Sika® Viscocrete®-1003 gives the concrete extended workability and depending on the mix design and the quality of materials used, self-compacting properties can be maintained for more than 1 hour at 30°C.

### PRODUCT INFORMATION

<b>Chemical Base</b>	Aqueous solution of modified polycarboxylate copolymers
<b>Packaging</b>	200 liters drums and bulk deliveries
<b>Appearance / Colour</b>	Brownish
<b>Shelf Life</b>	12 months from the date of production when stored in original unopened packaging
<b>Storage Conditions</b>	in a cool, dry place
<b>Density</b>	1.065 ± 0.01 kg/l

PRODUCT DATA SHEET  
Sika® ViscoCrete®-1003  
August 2016  
021301011000001463



#### Specific Advice

With the use of Sika® Viscocrete®-1003, concrete of the highest quality is produced. The standard rules of good concreting practice (production as well as placing) must also be observed with Sika® Viscocrete®-1003 concrete.

Fresh concrete must be cured properly.

#### APPLICATION INFORMATION

<b>Recommended Dosage</b>	For soft plastic concrete	0.2 - 0.6% by weight of binder
	For flowing and self compacting concrete (S.C.C.)	0.6 - 1.6% by weight of binder

#### Compatibility

Sika® Viscocrete®-1003 may be combined with the following products:

- Plastiment VZ
- SikaFume
- SikaAER
- Sika Control

Pre-trials are recommended if combinations with the above products are required.

Please consult our Technical Service Department.

To produce flowing and/or self-compacting concrete, special concrete mix design is required. Pre-trials are mandatory. Please consult our Technical Service Department.

#### APPLICATION INSTRUCTIONS

##### DISPENSING

Sika® Viscocrete®-1003 is added to the gauging water or simultaneously poured with it into the concrete mixer. For optimum utilisation of its high water reduction property, we recommend thorough mixing at a minimal wet mixing time of 60 seconds.

The addition of the remaining gauging water (to fine tune concrete consistency) may only be started after two-thirds of the wet mixing time, to avoid surplus water in the concrete.

##### BASIS OF PRODUCT DATA

All technical data stated in this Product Data Sheet are based on laboratory tests. Actual measured data may vary due to circumstances beyond our control.

#### LOCAL RESTRICTIONS

Please note that as a result of specific local regulations the declared data and recommended uses for this product may vary from country to country. Please consult the local Product Data Sheet for the exact product data and uses

#### ECOLOGY, HEALTH AND SAFETY

For information and advice on the safe handling, storage and disposal of chemical products, users shall refer to the most recent Safety Data Sheet (SDS) containing physical, ecological, toxicological and other safety-related data.





#### LEGAL NOTES

The information, and, in particular, the recommendations relating to the application and end-use of Sika products, are given in good faith based on Sika's current knowledge and experience of the products when properly stored, handled and applied under normal conditions in accordance with Sika's recommendations. In practice, the differences in materials, substrates and actual site conditions are such that no warranty in respect of merchantability or of fitness for a particular purpose, nor any liability arising out of any legal relationship whatsoever, can be inferred either from this information, or from any written recommendations, or from any other advice offered. The user of the product must test the product's suitability for the intended application and purpose. Sika reserves the right to change the properties of its products. The proprietary rights of third parties must be observed. All orders are accepted subject to our current terms of sale and delivery. Users must always refer to the most recent issue of the local Product Data Sheet for the product concerned, copies of which will be supplied on request.

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PRODUCT DATA SHEET  
Sika® ViscoCrete®-1003  
August 2016  
021301011000001463

3 / 3



## F. Dokumentasi Penelitian



Pengujian Berat Jenis Agregat Halus



Pengujian Ayakan Agregat Halus dan Kasar



Pembuatan Larutan NaOH



Pembuatan SSD Agregat Halus



Mengeringkan Serabut Kelapa Setelah direndam Larutan NaOH



Proses Mixing





Proses *Curing* Beton



Proses *Capping* Beton



Pengukuran Nilai *Slump*



Pengujian Modulus Elastisitas



Pengujian Kuat Tarik Belah



Pengujian Kuat Tekan