

BAB V

KESIMPULAN DAN SARAN

5.1 KESIMPULAN

Berdasarkan perhitungan dan analisis yang telah dilakukan menunjukkan bahwa kapasitas tangki eksisting tidak mencukupi untuk tekanan dan *flow rate* maksimal, yaitu sebesar 1050 psig dan 350 MMSCFD. Alternatif solusi yang mungkin untuk dilaksanakan di lapangan adalah dengan mengganti tangki lama dengan tangki yang didesain sesuai kapasitas. Struktur eksisting tidak mampu menanggung beban tangki baru, sehingga diperlukan rekonstruksi struktur tersebut.

5.2 SARAN

1. Dalam pembuatan tangki lebih lanjut, perlu dikaji lebih dalam lagi mengenai tekanan maksimum dan *flow rate* maksimum gas yang akan dilewatkan pada tangki tersebut.
2. Perlu adanya *Operating Manual*, agar saat gas dialirkan dalam kondisi tekanan yang berbeda-beda, tangki masih dapat bekerja secara optimum.
3. Perlu dilakukan pengukuran secara detail di lapangan apabila akan dilakukan rekonstruksi. Selain itu juga perlu dilakukan beberapa tinjauan agar solusi dari masalah tersebut efektif dan efisien.

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*Calculation Sheet for Slug Catcher Foundation (Bojonegara Station) (SJ2-F-52-CA-
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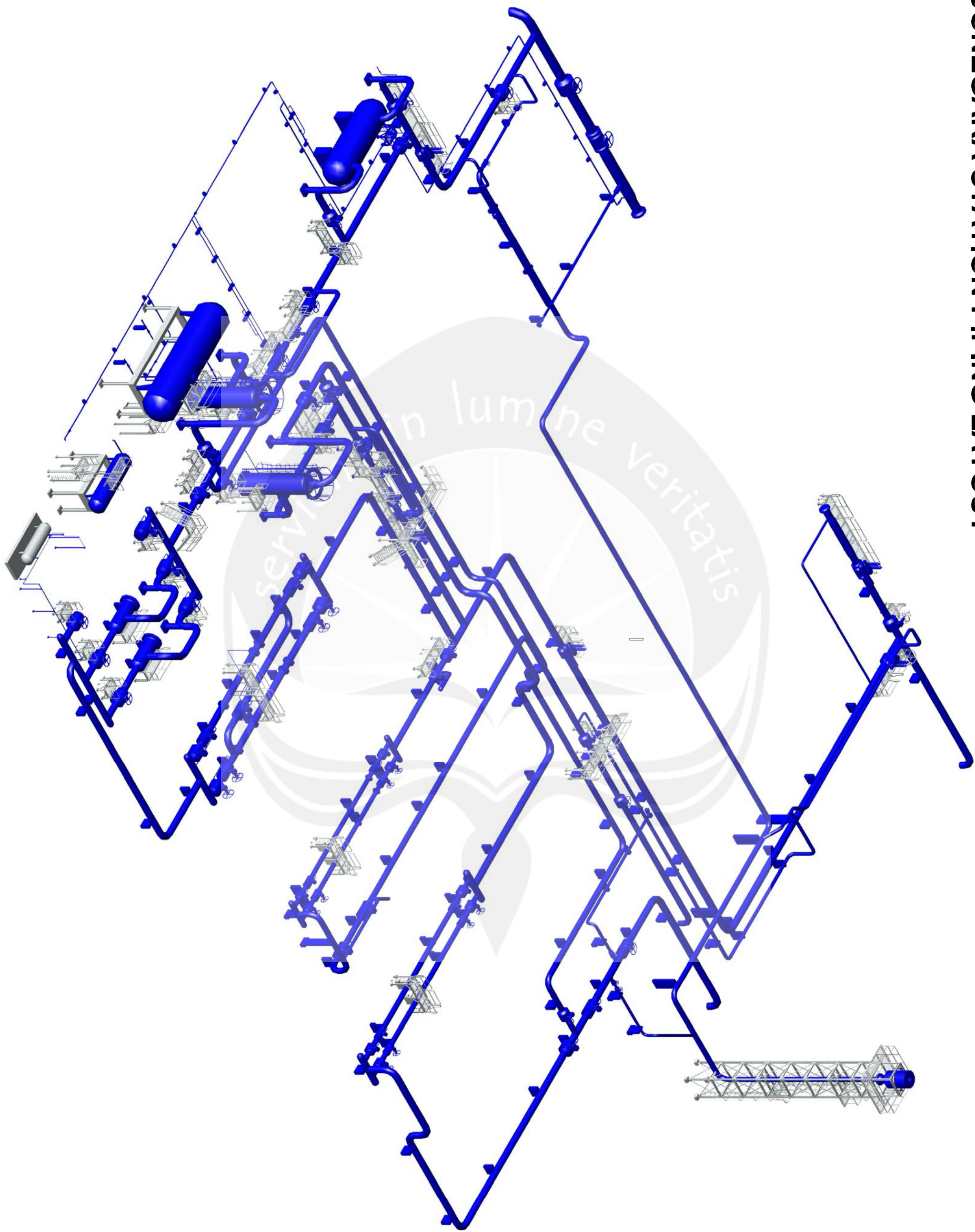
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indonesia.net/index.php?option=com_docman&task=doc_view&gid=360&Itemid=42



LAMPIRAN

BOJONEGARA STATION PIPING LAYOUT



43-R-1010
PIG RECEIVER
(EXISTING)

37-V-2130
SLUG CATCHER
(EXISTING)

37-V-2140
SUMP TANK-1
(EXISTING)

52-V-2110
SUMP TANK-2

52-V-2130
OUTLET GAS SCRUBBER

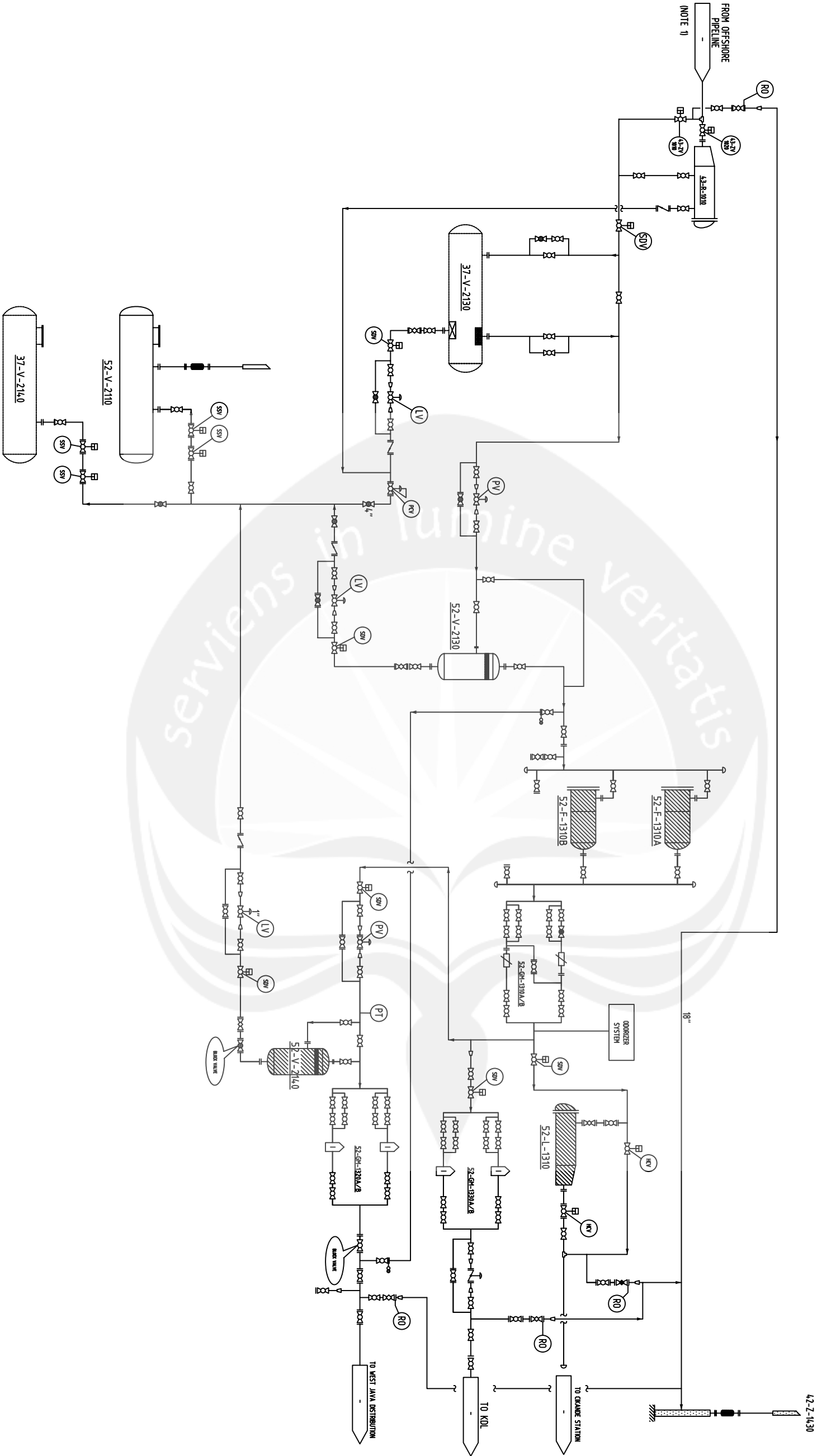
52-F-1310A/B
GAS FILTER

52-GM-1310A/B
METERING SYSTEM (USM)
52-V-2140
DELIVERY GAS SCRUBBER

52-L-1310
PIG LAUNCHER
52-GM-1330A/B
METERING SYSTEM (ORIFICE)

52-GM-1320A/B
METERING SYSTEM (ORIFICE)

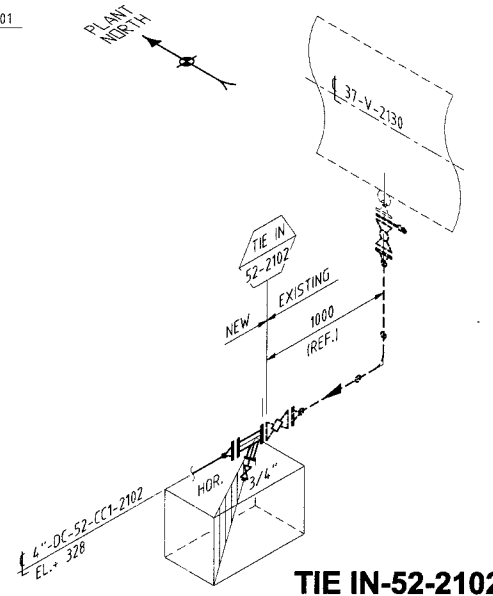
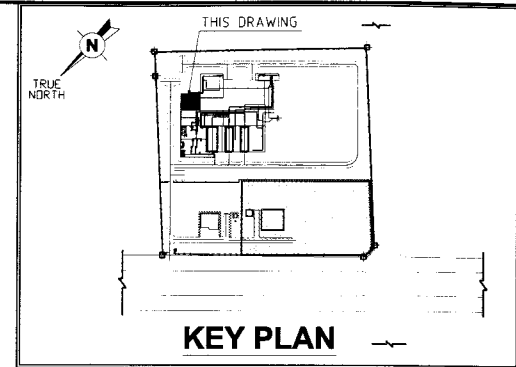
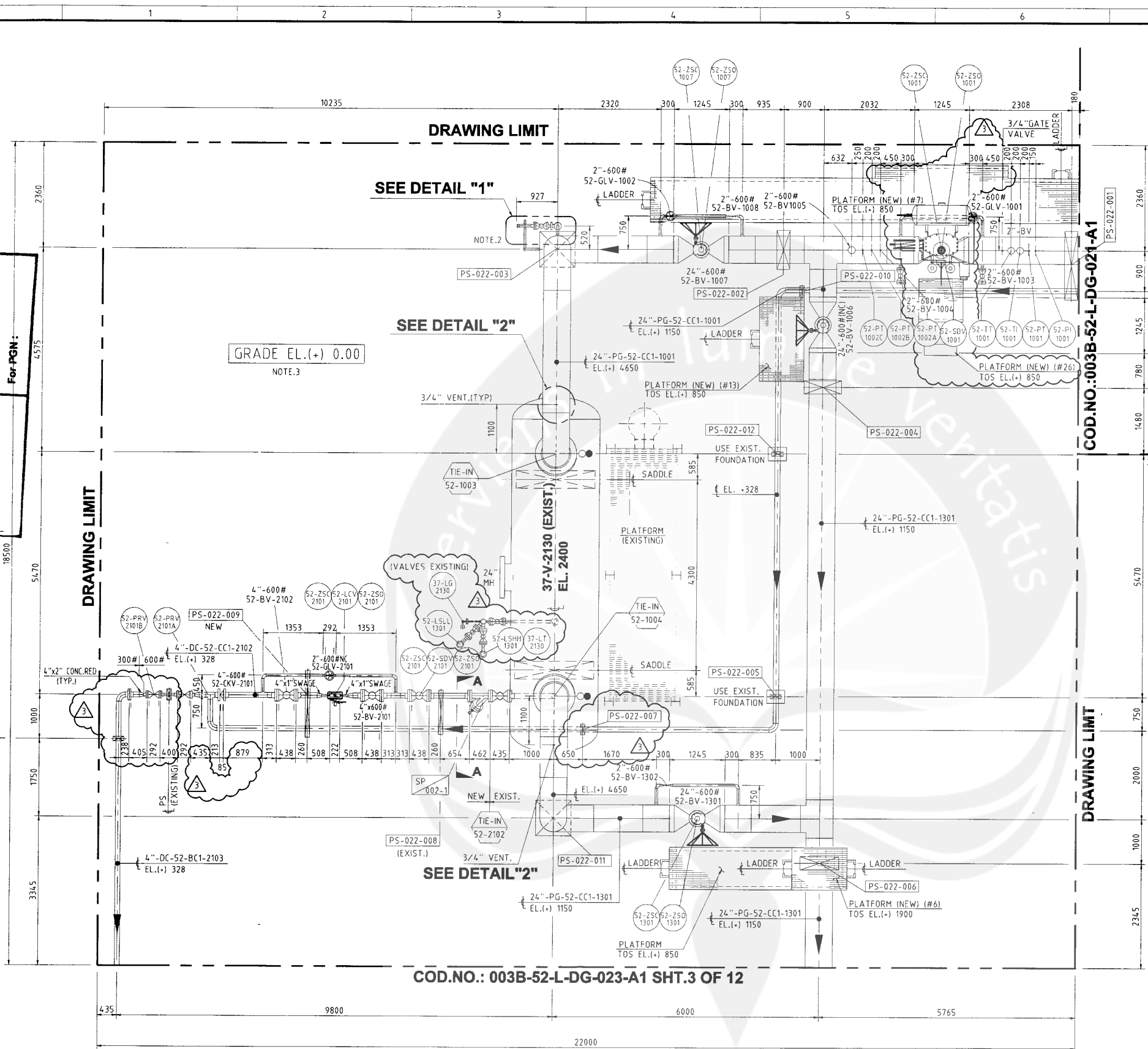
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NO OBJECTION

Notice of No Objection is given on the understanding that PT. Istana Karang Laut shall remain entirely responsible for fully complying with the Employer's Requirements and that the completed Works are fit for purpose.

For PGN:



REV. NO.	REVISION	DATE	CHK'D	EM	PM	PMC	PGN
3	RE-ISSUED FOR CONSTRUCTION	28 JAN '08	JSH	DS	SM	GZL	
2	RE-ISSUED FOR CONSTRUCTION	05 NOV '07	JSH	DS	SM	GZL	
1	RE-ISSUED FOR CONSTRUCTION	14 SEP '07	JSH	DS	SM	GZL	
0	ISSUED FOR CONSTRUCTION	31 AUG '07	JSH	DS	SM	GZL	
C	RE-ISSUED FOR REVIEW	09 AUG '07	JSH	DS	SM	GZL	

SCALE 1 : 50 DESIGNED : AS DRAWN : AH APP'D : GZL

PT. PERUSAHAAN GAS NEGARA (Persero) Tbk.

IN ASSOCIATION WITH

JOE OGE CONNUSUR

P.T. ISTANA KARANG LAUT

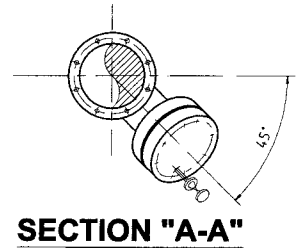
SOUTH SUMATRA - WEST JAVA GAS PIPELINE PROJECT
CONTRACT PACKAGE FOR BOJONEGARA STATION

JOB. NO.	DRAWING NO.	REV.	SHEET
C-199/07	003B-52-L-DG-022-A1	3	2

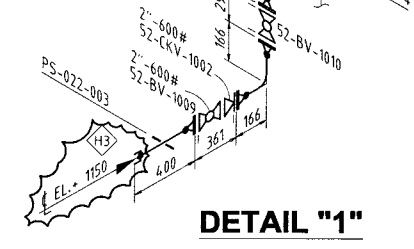
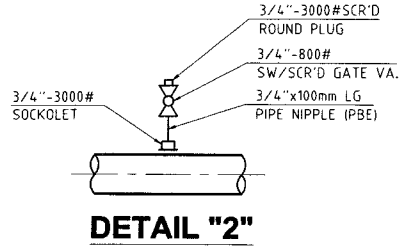
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- NOTE.**
1. ALL DIMENSIONS ARE IN MILLIMETER UNLESS OTHERWISE STATED
 2. NITROGEN CONNECTION
 3. GRADE EL.0.00 = MSL ±3.183 (LAT 3.783)



PLAN VIEW



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Lampiran 4 : Output Hysys

OK - HYSYS 3.2

File Edit Simulation Flowsheet Workbook Tools Window Help

Case (Main)
Steady State

Workbook - Case (Main)

Name	1	gas	liq	*** New ***
Vapour Fraction	0.9607	1.0000	0.0000	
Temperature [F]	68.00	68.00	68.00	
Pressure [psia]	1165	1165	1165	
Molar Flow [MMSCFD]	350.0	336.2	13.75	
Mass Flow [lb/hr]	7.846e+005	7.171e+005	6.756e+004	
Std Ideal Liq Vol Flow [barrel/day]	1.498e+005	1.409e+005	8860	
Heat Flow [Btu/hr]	-1.518e+009	-1.425e+009	-9.260e+007	
Molar Enthalpy [Btu/lbmole]	-3.950e+004	-3.860e+004	-6.131e+004	
Actual Volume Flow [barrel/day]	7.762e+005	7.673e+005	8919	
Mass Density [lb/ft3]	4.321	3.995	32.38	
Viscosity [cP]	<empty>	1.337e-002	0.1116	
Comp Mole Frac (Nitrogen)	0.0282	0.0293	0.0025	
Comp Mole Frac (Oxygen)	0.0027	0.0027	0.0003	
Comp Mole Frac (CO2)	0.0432	0.0425	0.0599	
Comp Mole Frac (Methane)	0.8167	0.8421	0.1974	
Comp Mole Frac (Ethane)	0.0496	0.0475	0.1010	
Comp Mole Frac (Propane)	0.0365	0.0273	0.2619	
Comp Mole Frac (i-Butane)	0.0070	0.0035	0.0927	
Comp Mole Frac (n-Butane)	0.0094	0.0038	0.1466	
Comp Mole Frac (i-Pentane)	0.0033	0.0006	0.0679	
Comp Mole Frac (n-Pentane)	0.0016	0.0002	0.0348	
Comp Mole Frac (n-Hexane)	0.0014	0.0001	0.0340	
Comp Mole Frac (H2O)	0.0003	0.0003	0.0010	
Comp Mole Frac (oil*)	0.0000	0.0000	0.0000	

Streams Unit Ops

Fluid Pkg All

☐ Include Sub-FI

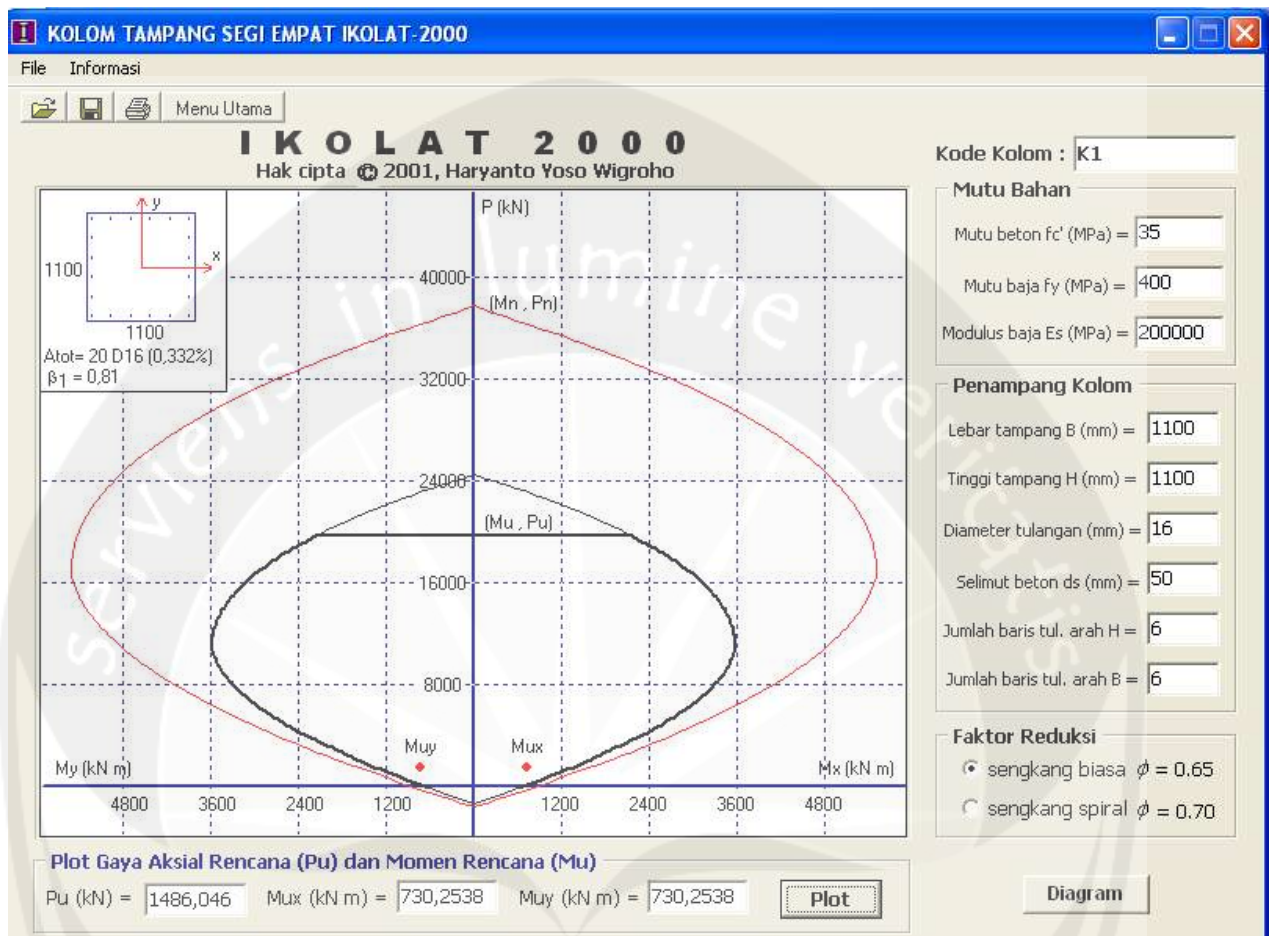
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Lampiran 5 : Output IKOLAT untuk Pedestal



Lampiran 6 : Design Specification for Civil and Structural (Doc No.003B-52-SP-001-A4)

 PT. PERUSAHAAN GAS NEGARA (Persero) Tbk.		 PT. ISTANA KARANG LAUT
DESIGN SPECIFICATION FOR CIVIL AND STRUCTURAL		
Document No. 003B-52-S-SP-001-A4	Rev. 3	CONTRACT PACKAGE FOR BOJONEGARA STATION
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2. DEFINITION.....	5
3. APPLICABLE STANDARD, CODES AND REFERENCES	6
4. SITE CONDITIONS.....	7
5. STRUCTURAL.....	9

ATTACHMENT:

1. Soil Investigation Report

- Water Level observation
- Soil Profile for Seismic Design
- Strength Parameter
- Settlement Analysis Result
- Negative Skin Friction
- Plates for allowable pile capacity

2. UBC 1997

- Reduction to live loads
- Wind Design
- Earthquake Design
- Tables

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3. ASCE 7 – 05

- Loading Combination
- Reduction to live loads

4. ACI 318 – 05

- Strength and Serviceability requirement

5. SKBI-1.3.53, 1987

- Loading Requirement for Buildings

6. Weather Data

- Average Wind Speed 2007- June 2010 at BMG Serang Station

Rangkuman Kapasitas Ijin Pile

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5.4. Stability

All buildings, structures and or foundations shall be designed so that the safety factor shall not be less than the values mentioned below at any circumstances.

- (1) Sliding
The ratio of resisting force due to friction to total shear cause by wind or earthquake load is not less than 1.5 and caused by permanent load is not less than 2.0
- (2) Overturning
The ratio of resisting moment due to dead load to over turning moment cause by wind or earthquake load is not less than 1.5 and caused by permanent load is not less than 2.0
- (3) Buoyancy
The ratio of total dead load of structure and soil weight above to total buoyancy is not less than 1.1

5.6. Soil Bearing Capacity

Based on the soil investigation report, shallow foundation is not recommended. Therefore soil bearing capacity will be ignored unless the structure integrity is not affected by the soil settlement.

5.7. Pile Bearing Capacity

For the heavy load and settlement sensitive structure/foundation, the piled foundation shall be used. The type of pile to be used shall conform to soil condition and also considering any plant environment condition.

1. Based on the Soil Investigation Report, the estimated length of piles from ground surface and allowable capacities are as follows.

Table 5.8 Summary of Allowable Pile Capacity

Pile Type	Pile Dimension (mm)	Depth (m)	Allowable Capacity (ton)			
			Comp.	Tension	Neg.Eric	Lateral
Square	250 x 250	15	82,5	12	14,2	4,3
Spun	300	15	85	11,5	13,4	4,9
Spun	350	15	112,5	13,5	15,6	6,2
Spun	400	15	142,5	15,4	17,9	7,6

Note: Allowable Pile Capacity is taken from Soil investigation report. See Attachment.

Keterangan Mengenai Beban Angin

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5.2.4 Equipment load ("E")

Equipment load means the weight of equipment. Equipment load can be taken from Piping stress analysis or Equipment Mechanical Data and shall consider the heaviest condition from Operation, Test, or Maintenance.

5.2.5 Impact load ("I")

Impact load means forces caused by a moving object such as crane, hoist and elevator. The load shall be taken accordance with SKBI-1.3.53. 1987 UDC 624.042 as follows :

- Vertically : $\frac{1}{10}$ of the maximum wheel load
- Horizontally, parallel with rail direction : $\frac{1}{7}$ of the maximum wheel load
- Horizontally, orthogonal with rail direction : $\frac{1}{15}$ of the maximum wheel load
- Elevator (at foundation) : 100% of max capacity (incl. self weight)

5.2.6 Wind load ("W")

Wind Load shall be calculated based on the equation from UBC 1997, chapter 16

$$P = C_e \times C_q \times q_s \times I_w$$

C_e = Combined height, exposure and gust factor coefficient as given in table 16-G UBC 1997

C_q = Pressure coefficient for the structure or portion of structure under consideration as given in table 16-H UBC 1997

I_w = Important factor as set forth in table 16-K UBC 1997

P = Design Wind pressure (kg/m²)

Q_s = Wind stagnation pressure at standard height of 33 feet as set forth in table 16-F UBC 1997 (See Attachment)

Those parameter above shall consider items as follows :

- (a) Standard wind velocity at the height of 9.0 m (33 feet) above ground level shall be 35 m/dtk (80 mph) refer to UBC 1997
- (b) Wind stagnation pressure (q_s) shall be 16.4 psf (80 kg/m²)
- (c) Wind load based on terrain exposure "C"
- (d) Important factor, $I = 1$ (for Building or Structure Outside Hazardous Area)
 $I = 1,15$ (for Building or Structure Inside Hazardous Area)



Keterangan Mengenai Beban Gempa

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5.2.7 Earthquake Load ("V")

Earth quake load for Building Structure shall be calculate accordance to formula given in UBC 1997, chapter 16.

Total lateral force or shear at the base :

$$V = \frac{C_v I W}{R T}$$

Total lateral force shall not more than :

$$V = \frac{2.5 C_a I W}{R}$$

Total lateral force shall not less than :

$$V = 0.11 C_a I W$$

- Where, V : Total lateral load load or shear at the base.
 Ca : Basic seismic coefficient, table 16-Q UBC 1997
 Cv : Basic seismic coefficient, table 16-R UBC 1997
 I : Importance factor, table 16-K UBC 1997
 R : Numerical Coefficient that represent the ductile capacity of the structure that support the lateral load, table 16-N & table 16-P UBC 1997
 T : Vibration period of the structure.
 W : Total dead load, reduced load, piping and equipment load.

Those parameters above shall consider items as follows:

1. Seismic Risk Zone 4 (Z = 0.4)
2. Important factor I = 1 (for Building or Structure Outside Hazardous Area)
3. Important factor I = 1,25 (for Building or Structure Inside Hazardous Area)
4. Soil profile (Soft Soil Profile SE, UBC 1997 and Soil Investigation Report)
5. Vibration period of the structure, T shall be calculated based on following formula :



$$T = C_t (h_n)^{3/4}$$

- Where,
- C_t = 0.0835 for steel moment resisting frames
 - C_t = 0.0731 for reinforced concrete moment resisting frames dan eccentrically braced frames
 - C_t = 0.0488 for other structures
 - h_n = height of the structure (m)

Lanjutan Mengenai Beban Gempa

 PT. PERUSAHAAN GAS NEGARA (Persero) Tbk.		 PT. ISTANA KARANG LAUT
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For Other Structure such as Pipe Support or Equipment foundation, Seismic load should be calculated according to UBC 97 Section 1634.5 for non-building structure.

$$V = 0,56 Ca I Wt$$

Where,

Ca = (refer to table 16-Q & 16 T UBC 97)

I = (refer to table 16-K UBC 97)

I = 1 (for Building or Structure Outside Hazardous Area)

I = 1,25 (for Building or Structure Inside Hazardous Area)



Z = (Zone 4)

Soil = S_E (Soft Soil)

Wt = Total self weight of structure

5.2.8 Piping load ("P")

Piping load means the weight of pipe, fittings, valves, insulation and the fluid contents of piping. Piping load shall be taken from pipe stress analysis result and consider the heaviest condition from Operation, Test or Maintenance.

5.2.9 Thermal force/Friction force ("T")

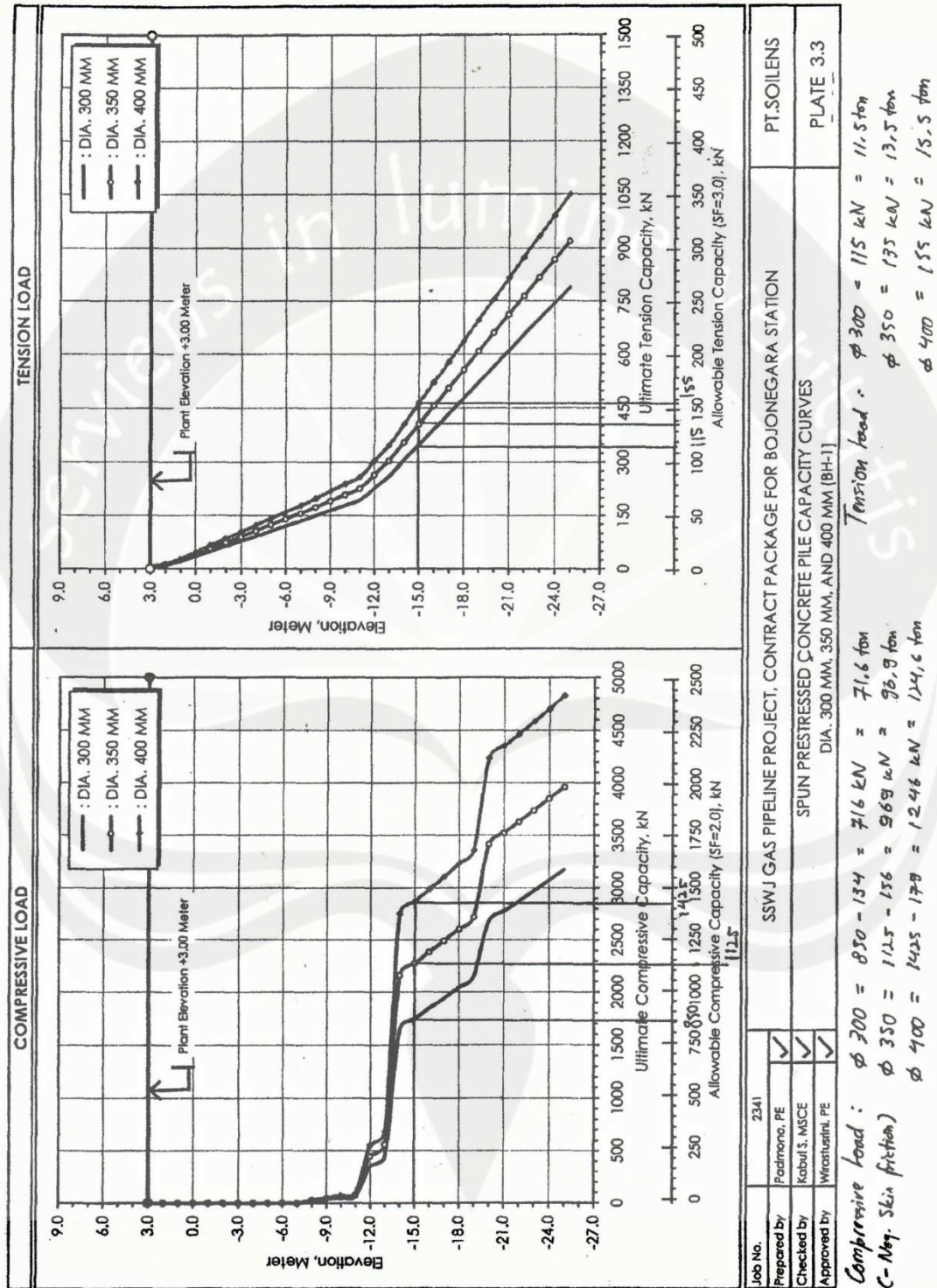
The magnitude and direction of thermal forces due to constraint of hot piping or equipment shall be calculated separately by piping or equipment engineer.

Reference: Concise Metals Handbook by J.R. davis ASM

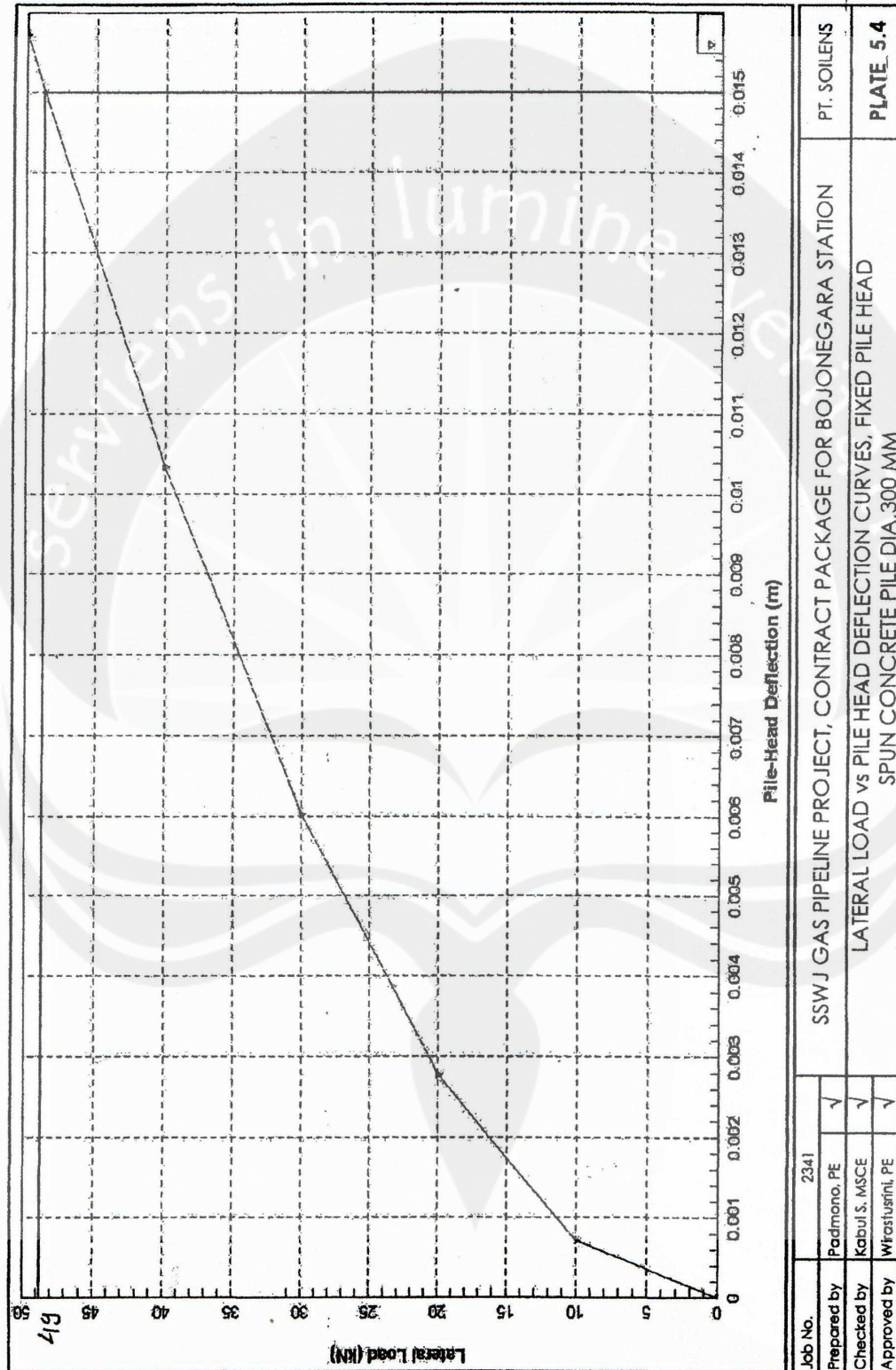
Friction factors, shall be as follows :

Concrete to soil	= 0.50
Steel to concrete	= 0.40
Steel to steel	= 0.30
Steel to lubricated bearing plate	= 0.10
Steel to teflon	= 0.04

Lampiran 7 : Grafik Kapasitas Pile (Compressive Load dan Tension Load)



Grafik Kapasitas Pile (Lateral Load)



Lampiran 8 : Design Basis and Procedures for 32" South Sumatra-West Java Offshore Pipeline (Doc No. 002-42-L-BD-1001)

as Negara	PT. PERUSAHAAN GAS NEGARA (Persero) Tbk.	
	CONTRACT PACKAGE – NO. 2 LABUHAN MARINGGAI – CILEGON OFFSHORE PIPELINE	<u>NIPPON STEEL</u>
	Document Title : DESIGN BASIS AND PROCEDURE FOR 32" SOUTH SUMATRA-WEST JAVA OFFSHORE PIPELINE	Page 16 of 152

2.3	<u>General Parameters</u>
	The following general parameters will be adopted in the detailed design of the proposed 32" offshore gas transmission pipeline.
▪	Steel Density : 7850 kg/m ³
▪	Young's Modulus of Elasticity of Steel : 2.07 x 10 ⁵ MPa
▪	Poisson's Ratio for Steel : 0.3
▪	Linear Coefficient of Thermal Expansion of Steel : 11.7 x 10 ⁻⁶ /°C
2.4	<u>Operating Parameters</u>
	The pipeline system operating parameters are presented below:
▪	Design Pressure ⁽¹⁾ : 1150 psig (7,929 kPag)
▪	Maximum Allowable Operating Pressure (MAOP) ⁽³⁾ : 1050 psig (7,239 kPag)
▪	Maximum Design Temperature ⁽¹⁾ : 120°F (48.9°C)
▪	Hydrotest Pressure (Offshore) ⁽²⁾ : 1312.5 psi (9,049 kPa) (= 1.25 x MAOP)
▪	Flow Rate for Design ⁽¹⁾ : 275 MMSCFD
▪	Content Density ⁽⁴⁾ : 50 kg/m ³ (3.12 pcf)

SOUTH SUMATRA-WEST JAVA GAS PIPELINE PROJECT	
Document No. :	002-42-L-BD-1001
Revision :	0
Date :	16.04.07

Lampiran 9 : Operation and Maintenance Manual for Offshore Pipeline (Doc No.002-40-G-MN-3301)

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OPERATION AND MAINTENANCE MANUAL		
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5. PROCESS DESIGN BASIS AND MATERIAL BALANCE

5.1. Process Design Basis

PT Perusahaan Gas Negara (PGN) Tbk. intends to provide the South Sumatra-West Java Gas pipeline distribution with a station for gas handling and metering. It is namely Bojonegara Station.

This natural gas handling facility at Bojonegara Station will receive the natural gas from South Sumatra to West Java through offshore pipeline, and then transfer and deliver it to the West Java distribution pipelines system for costumers in West Java area.

Bojonegara Station has several functions which are:

- a. To receive 275 MMSCFD of natural gas from Labuhan Maringgai – Bojonegara Offshore Pipeline (In the future, this flow rate will be increased up to 350 MMSCFD (maximum) or more to supply natural gas to another customers in West Java area).
- b. To control gas pressure, metering and transfer 110 MMSCFD of natural gas from Bojonegara Station to Serpong via Cikande and Bitung Stations.
- c. To control gas pressure, metering and distribute 165 MMSCFD of natural gas from Bojonegara Station to West Java Distribution Pipeline.
- d. To control gas flow, metering and deliver 75 MMSCFD of natural gas from Bojonegara Station to Krakatau Daya Listrik (KDL) at Cilegon industrial area through KDL's 20" pipeline
- e. To control Odorize injection rate into variable gas flow rate until 350 MMSCFD (maximum) flow rate.
- f. To provide all necessary interfaces with SCADA and Telecommunication Systems for the entire South Sumatra – West Java Gas Pipeline System.
- g. To ensure that the completed gas transmission and distribution system will operate safely and efficiently without danger to the general public or the operational staff, and without being adversely affected by the environment;

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OPERATION AND MAINTENANCE MANUAL		
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- h. To provide a safe facility that meets the capacity and quality requirements as defined in the Employer's Requirements and that is consistent with industrial standards;
- i. To provide appropriate materials and equipment that will allow optimum operation of the facility at the given design conditions, whilst requiring a minimum of supervisory, operating and maintenance personnel; be cost-effective to maintain;
- j. To function throughout the entirety of its specified design life in accordance with the requirements as may be more precisely defined elsewhere within the Employer's Requirements; and
- k. To accommodate future changes and expansions with minimum operational disruption.
- l. To protect the Station/Pipelines from overpressure and low pressure by Shutting Down related SDV.

For detail information, See Document No. 003B-52-P-BD-001-A4 for Process Design Basis

Lampiran 10: Calculation Sheet for Slug Catcher Foundation (Bojonegara Station) (SJ2-F-52-CA-702-A4)

 PT. PERUSAHAAN GAS NEGARA (Persero) Tbk.		 JOE In Association With	 OGC	 REKAYASA
CALCULATION SHEET FOR SLUG CATCHER FOUNDATION (BOJONEGARA STATION)				
Document No.	Rev.	CONTRACT NO.:		Page 4 of 38
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1. GENERAL

1.1 OUTLINE OF STRUCTURE

Project : SSWJ Phase II - Onshore/Station Facilities
 Client : PT. Perusahaan Gas Negara (Persero)
 Location : Bojonegara
 Equipment : Slug Catcher 37-V-2130
 Foundation type : Pile foundation

1.2 DESIGN PHILOSOPHY

1.2.1. Unit of measurement

Unit of measurement in design shall be in metric system.

1.2.2. Used of Computer Software used in design analysis, as follow :

-Math Cad
 -MS-Excel

1.3 CODE AND STANDARD

1.3.1. General codes and standard

-SKBI-1.3.53, 1987. UDC : 624.042
 Indonesian Loading Code for Building
 -SKBI-1.3.53, 1987. UDC : 699.841
 Earthquake Resistant Code for Building in Indonesia
 -ACI 318-95
 Building code for Structural Concrete

1.3.2. Specific Codes and Standard

-SJ2-F-00-S-SP-201-A4
 Design Specification for Civil and Structural

 PT. PERUSAHAAN GAS NEGARA (Persero) Tbk.	  	 REKAYASA
CALCULATION SHEET FOR SLUG CATCHER FOUNDATION (BOJONEGARA STATION)		
Document No. SJ2-F-52-S-CA-702-A4	Rev. 0	CONTRACT NO.: 001800.PK/24/UT/2006
		Page 5 of 38

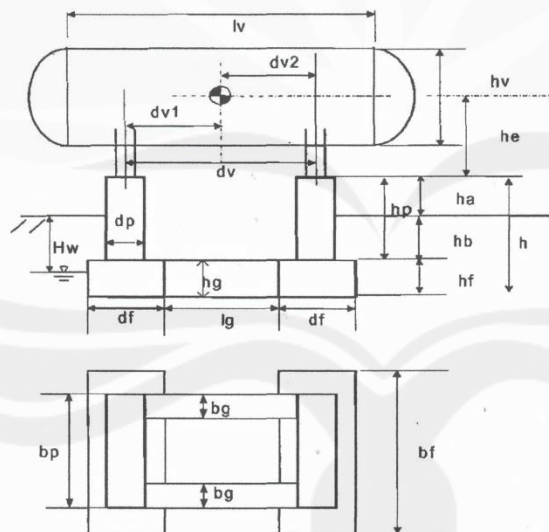
1.4 USED MATERIAL AND ALLOWABLE STRESS

Compressive concrete strength :	$f_c := 210 \cdot \text{kg} \cdot \text{cm}^{-2}$
Yield steel strength :	$f_y := 3200 \cdot \text{kg} \cdot \text{cm}^{-2}$
Specific gravity of reinf. concrete :	$\gamma_c := 2400 \cdot \text{kg} \cdot \text{m}^{-3}$
Specific gravity of steel :	$\gamma_s := 7850 \cdot \text{kg} \cdot \text{m}^{-3}$
Specific gravity of soil (refer to Soil Investigation Report) :	$\gamma_{\text{soil}} := 1800 \cdot \text{kg} \cdot \text{m}^{-3}$
Specific gravity of water :	$\gamma_{\text{water}} := 1000 \cdot \text{kg} \cdot \text{m}^{-3}$

1.5 OTHER REQUIREMENT DATA

Ground water table (refer to Soil Investigation Report) :	2.0 m below ground
---	--------------------

2. LAYOUT



 PT. PERUSAHAAN GAS NEGARA (Persero) Tbk.		 JOE In Association With	 OGC	 REKAYASA
CALCULATION SHEET FOR SLUG CATCHER FOUNDATION (BOJONEGARA STATION)				
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3. PRELIMINARY

3.1. DIMENSION

Bearing plate of equipment :

$$dw := 0.26 \cdot m$$

$$lw := 1.250 \cdot m$$

Estimated pedestal size due to dimension required from :

-Base plate of equipment :

$$bp := lw + 100 \cdot mm$$

$$bp = 1.35 \cdot m$$

$$dp := dw + 100 \cdot mm$$

$$dp = 0.36 \cdot m$$

For practical used :

$$bp := 1.50 \cdot m$$

$$dp := 0.50 \cdot m$$

Estimated Footing Dimension :

$$df := 0.8 \cdot m$$

$$bf := 3.5 \cdot m$$

Diameter of equipment :

$$hv := 1.981 \cdot m$$

Vertical distance from COG vessel to bot. of baseplate :

$$he := 1.250 \cdot m$$

Length of equipment :

$$lv := 7.111 \cdot m$$

Distance between pedestal to pedestal :

$$dv := 4.271 \cdot m$$

Height of footing:

$$hf := 0.40 \cdot m$$

Height of pedestal above ground :

$$ha := 1.15 \cdot m$$

Height of pedestal below ground :

$$hb := 0.20 \cdot m$$

Height of pedestal:

$$hp := hb + ha$$

$$hp = 1.35 \cdot m$$

Height of foundation :

$$h := hf + hp$$

$$h = 1.75 \cdot m$$

Length of grade beam :

$$lg := dv - df$$

$$lg = 3.471 \cdot m$$

Width of grade beam:

$$bg := 0.3 \cdot m$$

Height of grade beam:

$$hg := 0.40 \cdot m$$

3.2. WEIGHT ESTIMATION

Weight of pedestal :

$$Wp := 2 \cdot bp \cdot dp \cdot hp \cdot \gamma_c$$

$$Wp = 4.86 \text{ tonne}$$

Weight of footing :

$$Wf := 2 \cdot bf \cdot df \cdot hf \cdot \gamma_c$$

$$Wf = 5.376 \text{ tonne}$$

Weight of grade beam :

$$Wg := 2 \cdot lg \cdot bg \cdot hf \cdot \gamma_c$$

$$Wg = 1.999 \text{ tonne}$$

Total concrete weight :

$$Wc := Wp + Wf + Wg$$

$$Wc = 12.235 \text{ tonne}$$

Soil weight above footing :

$$W_{soil} := [2 \cdot (bf \cdot df - bp \cdot dp) \cdot hb + lg \cdot bg \cdot (hb + hf - hg)] \cdot \gamma_{soil}$$

$$W_{soil} = 1.851 \text{ tonne}$$

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4. LOADING DATA

4.1 EQUIPMENT (E)

Erection Weight / E(E) :	EE := 33.1 tonne
Operation Weight / E(O) :	EO := 47.4 tonne
Test Weight / E(T) :	ET := 57.0 tonne
Tube bundle Weight / E(B) : Not to be Considered	EB := 0 tonne

4.2 FOUNDATION (Dead Load) (D)

D := Wc + Wsoil	D = 14.086 tonne
-----------------	------------------

4.3 SEISMIC LOAD (Transversal Direction) due to EO (V)

Lateral force :	He := 11.9 tonne
Moment at base plate :	Me := 15 tonne·m
Moment at bottom found. : Meb := Me + He·h	Meb = 35.825 tonne·m

4.4 SEISMIC LOAD (Transversal Direction) due to EE (V)

Lateral force :	Hee := 0.15EE	Hee = 4.965 tonne
Moment at base plate :	Mee := Hee·he	Mee = 6.206 tonne·m
Moment at bottom found. : Meeb := Mee + Hee·h		Meeb = 14.895 tonne·m

4.5 WIND LOAD (Transversal Direction) (W)

Lateral force :	Hw := 0.8 tonne
Moment at base plate :	Mw := 1 tonne·m
Moment at bottom found. : Mwb := Mw + Hw·h	Mwb = 2.4 tonne·m

4.6 THERMAL LOAD (Longitudinal Direction) (T)

Coefficient of friction :		$U_o := 0.3$
Lateral force :	$H_t := U_o \cdot E_o$	$H_t = 14.22 \text{ tonne}$
Moment at bottom found. :	$M_{tb} := H_t \cdot h$	$M_{tb} = 24.885 \text{ tonne} \cdot \text{m}$

4.7 TUBE BUNDLE PULLING LOAD (B)

Lateral force :	Hb := Uo·100%·EB	Hb = 0 tonne
Moment at bottom found. : Mbb := Hb·he + Hb·h		Mbb = 0 tonne·m

4.8 BOUYANCY EFFECT (Ww)

Depth of water table from ground :	hw := 1.0 m
Foundation volume below water table :	
Vw := (2·bf·df + lg·bg)·(hb + hf - hw)	Vw := 0 m ³
Bouyancy effect : Ww := γwater·Vw	Ww = 0 tonne

	  	
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Document No.	Rev.	CONTRACT NO.:
SJ2-F-52-S-CA-702-A4	0	001800.PK/24/UT/2006
		Page 3 of 38

5. DESIGN ANALYSIS

5.1 CHECK OF PILE CAPACITY

5.1.1 Axial Bearing Capacity

for PC Pile dia. 300 mm : 7 m u/p

All. single pile bearing capacity due to the supporting strength of the soil (data from Soil Investigati Report for Bojonegara Station, July 2006, borehole no. BH PB-4) :

$$Q_{call} := 26.5 \text{ tonne}$$

$$Q_{tall} := 3.25 \text{ tonne}$$

$$\text{Total vertical load : } V_w = \max((EE \quad EO \quad ET)) + D$$

$$\text{Estimated number of pile : } \Sigma n := \left(\frac{V}{Q_{call}} \right)$$

$$\Sigma n = 2.682$$

For practical used :

$$\Sigma n := 6$$

Diameter of pile :

$$\text{Dia} := 300 \text{ mm}$$

Minimum distance to edge of foundation :

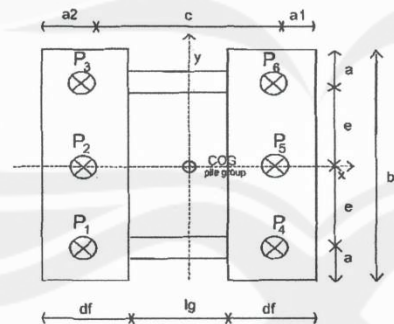
$$1.2 \cdot \text{Dia} = 0.36 \text{ m}$$

Minimum distance between pile :

$$3 \cdot \text{Dia} = 0.9 \text{ m}$$

Maximum distance between pile :

$$8 \cdot hf = 3.2 \text{ m}$$



Data :

$$\begin{aligned} c &:= lg + df & c &= 4.271 \text{ m} \\ a &:= 0.40 \text{ m} & a &= 0.4 \text{ m} \\ a1 &:= 0.50 \text{ m} & a1 &= 0.5 \text{ m} \\ a2 &:= df - a & a2 &= 0.4 \text{ m} \\ e_w &:= 0.5(bf - 2 \cdot a) & e &= 1.35 \text{ m} \end{aligned}$$

Lampiran 11 : Data Sheet for Slug Catcher (Doc No. SJ2-F-00-M-DS-003-A4)

PT. PERUSAHAAN GAS NEGARA (PERSERO) PENSPEN-PETROTEKNOLOGI UTAMA PT. REKAYASA INDUSTRI SOUTH SUMATERA WEST JAVA PHASE II PIPELINE ONSHORE CONTRACT NO. 001800.PK24A172006	Horizontal Slug Catcher	Doc No. SJ2-F-00-M-DS-003-A4 Rev. D Job No.: 06-1604 Tag No.: 37-V-2130 04/04/06 Approved for Construction 28/02/06 Preliminary Issue By: FR KR SGVDA CHKD: REKAYASA APPTD: PMC PGN																																
General Information																																		
Applicable To: ☐ Proposal ☒ Purchase ☐ As Built ☐ Vendor shall complete Data Sheet with information not otherwise provided by Buyer. For: PT Perusahaan Gas Negara (Persero) Unit Tag Number(s): 37-V-2130 Site: Labuhan Maringgai No. Required: 1 Service: Pipeline Slug Catcher Serial Number: _____ Vendor: _____																																		
Process Data																																		
<table style="width: 100%;"> <tr> <td style="width: 50%; vertical-align: top;"> SITE AND SERVICE CONDITIONS Ambient Temperature: 95°F (max.), 78.4°F (min.) Elevation/Barometric Pressure: 14,696 psia Relative Humidity: 81.5-90 percent - tropical climate Black Bulb Temperature: 178°F (max.) Solar Radiation: 300 BTU/hr-ft² (max.) Area Classification: Class I, Division 2, Group D (min.) Wind Velocity: 3 m/s (max.) Seismic Zone: Zone 3 per UBC </td> <td style="width: 50%; vertical-align: top;"> GAS COMPOSITION SPECIFICATION: Typical Gas Composition at Pasar Dewa during Upset Condition <table style="width: 100%;"> <tr><td>CO₂</td><td>4.324</td></tr> <tr><td>H₂S</td><td>-</td></tr> <tr><td>Nitrogen</td><td>2.82</td></tr> <tr><td>Methane</td><td>81.686</td></tr> <tr><td>Ethane</td><td>4.983</td></tr> <tr><td>Propane</td><td>3.656</td></tr> <tr><td>i-Butane</td><td>0.697</td></tr> <tr><td>n-Butane</td><td>0.942</td></tr> <tr><td>i-Pentane</td><td>0.329</td></tr> <tr><td>n-Pentane</td><td>0.18</td></tr> <tr><td>Hexane</td><td>-</td></tr> <tr><td>Heptane Plus</td><td>-</td></tr> <tr><td>n-Octane</td><td>0.14</td></tr> <tr><td>H₂O</td><td>0.015</td></tr> <tr><td>TOTAL</td><td>100</td></tr> </table> </td> </tr> </table>			SITE AND SERVICE CONDITIONS Ambient Temperature: 95°F (max.), 78.4°F (min.) Elevation/Barometric Pressure: 14,696 psia Relative Humidity: 81.5-90 percent - tropical climate Black Bulb Temperature: 178°F (max.) Solar Radiation: 300 BTU/hr-ft² (max.) Area Classification: Class I, Division 2, Group D (min.) Wind Velocity: 3 m/s (max.) Seismic Zone: Zone 3 per UBC	GAS COMPOSITION SPECIFICATION: Typical Gas Composition at Pasar Dewa during Upset Condition <table style="width: 100%;"> <tr><td>CO₂</td><td>4.324</td></tr> <tr><td>H₂S</td><td>-</td></tr> <tr><td>Nitrogen</td><td>2.82</td></tr> <tr><td>Methane</td><td>81.686</td></tr> <tr><td>Ethane</td><td>4.983</td></tr> <tr><td>Propane</td><td>3.656</td></tr> <tr><td>i-Butane</td><td>0.697</td></tr> <tr><td>n-Butane</td><td>0.942</td></tr> <tr><td>i-Pentane</td><td>0.329</td></tr> <tr><td>n-Pentane</td><td>0.18</td></tr> <tr><td>Hexane</td><td>-</td></tr> <tr><td>Heptane Plus</td><td>-</td></tr> <tr><td>n-Octane</td><td>0.14</td></tr> <tr><td>H₂O</td><td>0.015</td></tr> <tr><td>TOTAL</td><td>100</td></tr> </table>	CO ₂	4.324	H ₂ S	-	Nitrogen	2.82	Methane	81.686	Ethane	4.983	Propane	3.656	i-Butane	0.697	n-Butane	0.942	i-Pentane	0.329	n-Pentane	0.18	Hexane	-	Heptane Plus	-	n-Octane	0.14	H ₂ O	0.015	TOTAL	100
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FLUID DATA Fluid: Natural gas with condensates and carryovers Fluid State: Single Phase (norm), two phase (during plugging) Flowrate (Design): 159.7 MMSCFD Gas Specific Gravity at Base: 0.706 Gas Molecular Weight: 20.48 Gas Cp/Cv: 1.405 Gas Operating Viscosity: 0.0124 cP Liquid Density: 50.146 lbm/ft³ Liquid Molecular Weight: 80.88 Liquid N.B.P: _____ Liquid Operating Viscosity: _____ Base Pressure: 14,696 psia																																		
ID : 1981 mm (Note 4) LENGTH : 7011 mm S/S (Note 4)																																		

 PT. PERUSAHAAN GAS NEGARA (PERSERO)  PT. REKAYASA INDUSTRI SOUTH SUMATERA WEST JAVA PHASE II PIPELINE ONSHORE CONTRACT NO. 001800.PK24U/7/2006	Horizontal Slug Catcher	Doc No. SJ2-F-00-M-DS-003-A4 Rev. 0 Job No.: 06-1604 Tag No.: 37-V-2130 Approved for Construction Preliminary Issue By: CHKO APPD REKAYASA PMC PGN
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GENERAL INFORMATION	
Applicable To: ☒ Proposal ☐ Purchase ☐ As Built ☐ Seller shall complete Data Sheet with information not otherwise provided by Buyer. For: <u>PT. Perusahaan Gas Negara (Persero)</u> Site: <u>Labuhan Maringgai</u> Service: <u>Pipeline Slug Catcher</u> Vendor: _____	Unit Tag Number(s): <u>37-V-2130</u> No. Required: <u>1</u> Serial Number: _____

DESIGN & FABRICATION DATA Continuation to be in accordance with the latest edition of the ASME Code and Addenda Code Symbol: <u>U</u> Required: <u>Yes</u> Design Pressure: <u>1150</u> PSIG @ <u>150 °F</u> Operating Pressure (Normal/min): <u>1060/850</u> PSIG @ <u>95-100 °F</u> MDMT: <u>-30 °F</u> Stress Relieve: <u>Yes</u> Radiograph: <u>100%</u> Joint Eff.: Shell <u>1</u> Heads <u>1</u> Corr. Allowance: Shell <u>0.0625"</u> Heads <u>0.0625"</u> Min. Thickness: Shell <u>2.40" (Note 4)</u> Heads <u>2.32" (Note 4) (E81302a)</u> Material ASTM: Shell <u>SA-516-70</u> Heads <u>SA-516-70</u> Flanges: <u>(Note 5)</u> Pipe: <u>(Note 5)</u> Studs: <u>SA-193-B7</u> Nuts: <u>SA-194-2H</u> Internals: <u>SA-36</u> Saddles: <u>SA-36</u> Wire mesh type demisters: <u>316 Stainless Steel (Note 6)</u> Inlet Deflector: <u>SA-36</u> Vortex Breaker: <u>SA-36</u> Gaskets: <u>304 SS w/non Asbestos Filler (center ring + inner ring)</u> Nozz. Proj.: <u>250mm</u> M.W. Proj.: <u>300 mm</u> Dkn TOL: <u>3 mm</u> Saddles: <u>Yes</u> Lugs: <u>Yes</u> Hinges: <u>No</u> Davits: <u>Yes</u> Req'd for Manholes: _____ Insulation Thk.: <u>None</u> Insulation Rings: <u>None</u> Ladder Clips: <u>Yes</u> Platform Clips: <u>Yes</u> Paint: <u>As per project spec. Painting Doc.No. SJ2-F-00-M-SP-008-A4</u> Internal coating: <u>Yes</u>	Approximate Weight in Lbs Shipping: <u>(Note 3)</u> Erection: <u>(Note 3)</u> Operator: <u>(Note 3)</u> Hydrotest: <u>(Note 3)</u> NOZZLE SCHEDULE: <table border="1" style="width:100%; border-collapse: collapse; text-align: center;"> <tr> <th>MK</th> <th>NO.</th> <th>SIZE</th> <th>RATING</th> <th>TYPE</th> <th>SERVICE</th> </tr> <tr> <td>MW</td> <td>1</td> <td>24"</td> <td>600#</td> <td>RFWN</td> <td>Manway</td> </tr> <tr> <td>N10</td> <td>1</td> <td>4"</td> <td>600#</td> <td>RFWN</td> <td>Drain</td> </tr> <tr> <td>N9</td> <td>1</td> <td>4"</td> <td>600#</td> <td>RFWN</td> <td>Liquid Outlet</td> </tr> <tr> <td>N8</td> <td>1</td> <td>2"</td> <td>600#</td> <td>RFWN</td> <td>PSV</td> </tr> <tr> <td>N7</td> <td>1</td> <td>4"</td> <td>600#</td> <td>RFWN</td> <td>Washout</td> </tr> <tr> <td>N5 A/B</td> <td>2</td> <td>2"</td> <td>600#</td> <td>RFWN</td> <td>Bridglet/LG/LT</td> </tr> <tr> <td>N5</td> <td>1</td> <td>3/4"</td> <td>3000#</td> <td>NPT</td> <td>PI</td> </tr> <tr> <td>N4</td> <td>1</td> <td>3/4"</td> <td>3000#</td> <td>NPT</td> <td>PT</td> </tr> <tr> <td>N3</td> <td>1</td> <td>4"</td> <td>600#</td> <td>RFWN</td> <td>Vent</td> </tr> <tr> <td>N2</td> <td>1</td> <td>24"</td> <td>600#</td> <td>RFWN</td> <td>Gas Outlet</td> </tr> <tr> <td>N1</td> <td>1</td> <td>24"</td> <td>600#</td> <td>RFWN</td> <td>Gas Inlet</td> </tr> <tr> <td> </td> <td> </td> <td> </td> <td> </td> <td> </td> <td> </td> </tr> <tr> <td> </td> <td> </td> <td> </td> <td> </td> <td> </td> <td> </td> </tr> </table> All gage glass, level controls, alarms, etc. having two connections are to be set with jig Notes: _____ _____ _____	MK	NO.	SIZE	RATING	TYPE	SERVICE	MW	1	24"	600#	RFWN	Manway	N10	1	4"	600#	RFWN	Drain	N9	1	4"	600#	RFWN	Liquid Outlet	N8	1	2"	600#	RFWN	PSV	N7	1	4"	600#	RFWN	Washout	N5 A/B	2	2"	600#	RFWN	Bridglet/LG/LT	N5	1	3/4"	3000#	NPT	PI	N4	1	3/4"	3000#	NPT	PT	N3	1	4"	600#	RFWN	Vent	N2	1	24"	600#	RFWN	Gas Outlet	N1	1	24"	600#	RFWN	Gas Inlet												
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ADDITIONAL DATA 1. Unit shall be in accordance with Project Specification - Slug Catcher (Doc. no. SJ2-F-00-M-DS-005-A4) 2. Design Refer to ASME Sect. VIII Div. 1 3. Vendor to complete 4. Vendor to clarify 5. Refer to project specification General pipe, Valves and Fittings Doc. No. SJ2-F-00-M-SP-002-A4 6. Vendor to confirm the suitability of demister type with the gas composition 7. All material shall have the carbon equivalent not exceed 0.4 percent 8. The carbon content base on the product analysis shall not exceed 0.23 percent.
--

Lampiran 12 : Tabel 16-F Wind Stagnation Pressure (q_s)

TABLE 16-F
TABLE 16-G

1957 UNIFORM BUILDING CODE

TABLE 16-F—WIND STAGNATION PRESSURE (q_s) AT STANDARD HEIGHT OF 33 FEET (10 058 mm)

Basic wind speed (mph) ¹ (× 1.61 for km/h)	70	80	90	100	110	120	130
Pressure q_s (psf) (× 0.0479 for kN/m ²)	12.6	16.4	20.8	25.6	31.0	36.9	43.3

¹Wind speed from Section 1618.

Lampiran 13 : Tabel 16-G Combined Height, Exposure and Gust Factor Coefficient

TABLE 16-G—COMBINED HEIGHT, EXPOSURE AND GUST FACTOR COEFFICIENT (C_e)⁺

HEIGHT ABOVE AVERAGE LEVEL OF ADJOINING GROUND (feet) × 304.8 for mm	EXPOSURE D	EXPOSURE C	EXPOSURE B
0-15	1.39	1.00	0.62
20	1.45	1.03	0.67
25	1.50	1.06	0.72
30	1.54	1.09	0.76
40	1.62	1.13	0.84
60	1.73	1.20	0.95
80	1.81	1.25	1.04
100	1.88	1.30	1.13
120	1.93	1.34	1.20
160	2.02	1.40	1.31
200	2.10	1.45	1.42
300	2.23	1.60	1.63
400	2.34	1.75	1.80

⁺Values for intermediate heights above 15 feet (4572 mm) may be interpolated.

Lampiran 14 : Tabel 16-H Pressure Coefficient

1997 UNIFORM BUILDING CODE

TABLE 16-H¹TABLE 16-H—PRESSURE COEFFICIENTS (C_p)

STRUCTURE OR PART THEREOF	DESCRIPTION	C_p FACTOR
1. Primary frames and systems	Method 1 (Normal force method) Walls: Windward wall Leeward wall Roofs ¹ : Wind perpendicular to ridge Leeward roof or flat roof Windward roof less than 2:12 (16.7%) Slope 2:12 (16.7%) to less than 9:12 (75%) Slope 9:12 (75%) to 12:12 (100%) Slope > 12:12 (100%) Wind parallel to ridge and flat roofs	0.8 inward 0.5 outward 0.7 outward 0.7 outward 0.9 outward or 0.3 inward 0.4 inward 0.7 inward 0.7 outward
	Method 2 (Projected area method) On vertical projected area Structures 40 feet (12 192 mm) or less in height Structures over 40 feet (12 192 mm) in height On horizontal projected area ¹	1.3 horizontal any direction 1.4 horizontal any direction 0.7 upward
2. Elements and components not in areas of discontinuity ²	Wall elements All structures Enclosed and unenclosed structures Partially enclosed structures Parapets walls	1.2 inward 1.2 outward 1.6 outward 1.3 inward or outward
	Roof elements ³ Enclosed and unenclosed structures Slope < 2:12 (16.7%) Slope 2:12 (16.7%) to 12:12 (100%)	1.3 outward 1.3 outward or inward
	Partially enclosed structures Slope < 2:12 (16.7%) Slope 2:12 (16.7%) to 7:12 (58.3%) Slope > 7:12 (58.3%) to 12:12 (100%)	1.7 outward 1.6 outward or 0.8 inward 1.7 outward or inward
3. Elements and components in areas of discontinuities ^{2,4,5}	Wall corners ⁶	1.5 outward or 1.2 inward
	Roof eaves, rakes or ridges without overhangs ⁵ Slope < 2:12 (16.7%) Slope 2:12 (16.7%) to 7:12 (58.3%) Slope > 7:12 (58.3%) to 12:12 (100%) For slopes less than 2:12 (16.7%) Overhangs at roof eaves, rakes or ridges, and canopies	2.3 upward 2.6 outward 1.6 outward 0.5 added to values above
4. Chimneys, tanks and solid towers	Square or rectangular Hexagonal or octagonal Round or elliptical	1.4 any direction 1.1 any direction 0.8 any direction
	5. Open-frame towers ^{7,8} Square and rectangular Diagonal Normal Triangular	4.0 3.6 3.2
6. Tower accessories (such as ladders, conduit, lights and elevators)	Cylindrical members 2 inches (51 mm) or less in diameter Over 2 inches (51 mm) in diameter Flat or angular members	1.0 0.8 1.3
7. Signs, flagpoles, lightpoles, minor structures ⁸		1.4 any direction

¹For one story or the top story of multistory partially enclosed structures, an additional value of 0.5 shall be added to the outward C_p . The most critical combination shall be used for design. For definition of partially enclosed structures, see Section 1616.

² C_p values listed are for 10-square-foot (0.93 m²) tributary areas. For tributary areas of 100 square feet (9.29 m²), the value of 0.3 may be subtracted from C_p , except for areas at discontinuities with slopes less than 7 units vertical in 12 units horizontal (58.3% slope) where the value of 0.8 may be subtracted from C_p . Interpolation may be used for tributary areas between 10 and 100 square feet (0.93 m² and 9.29 m²). For tributary areas greater than 1,000 square feet (92.9 m²), use primary frame values.

³For slopes greater than 12 units vertical in 12 units horizontal (100% slope), use wall element values.

⁴Local pressures shall apply over a distance from the discontinuity of 10 feet (3048 mm) or 0.1 times the least width of the structure, whichever is smaller.

⁵Discontinuities at wall corners or roof ridges are defined as discontinuous breaks in the surface where the included interior angle measures 170 degrees or less.

⁶Load is to be applied on either side of discontinuity but not simultaneously on both sides.

⁷Wind pressures shall be applied to the total normal projected area of all elements on one face. The forces shall be assumed to act parallel to the wind direction.

⁸Factors for cylindrical elements are two thirds of those for flat or angular elements.

Lampiran 15 : Tabel 16-K Occupancy Category

TABLE 16-K—OCCUPANCY CATEGORY

OCCUPANCY CATEGORY	OCCUPANCY OR FUNCTIONS OF STRUCTURE	SEISMIC IMPORTANCE FACTOR, I_p	SEISMIC IMPORTANCE FACTOR ¹ , I_p	WIND IMPORTANCE FACTOR, I_w
1. Essential facilities ²	Group I, Division 1 Occupancies having surgery and emergency treatment areas Fire and police stations Garages and shelters for emergency vehicles and emergency aircraft Structures and shelters in emergency-preparedness centers Aviation control towers Structures and equipment in government communication centers and other facilities required for emergency response Standby power-generating equipment for Category 1 facilities Tanks or other structures containing housing or supporting water or other fire-suppression material or equipment required for the protection of Category 1, 2 or 3 structures	1.25	1.50	1.15
2. Hazardous facilities	Group H, Divisions 1, 2, 6 and 7 Occupancies and structures therein housing or supporting toxic or explosive chemicals or substances Nonbuilding structures housing, supporting or containing quantities of toxic or explosive substances that, if contained within a building, would cause that building to be classified as a Group H, Division 1, 2 or 7 Occupancy	1.25	1.50	1.15
3. Special occupancy structures ³	Group A, Divisions 1, 2 and 2.1 Occupancies Buildings housing Group E, Divisions 1 and 3 Occupancies with a capacity greater than 300 students Buildings housing Group B Occupancies used for college or adult education with a capacity greater than 500 students Group I, Divisions 1 and 2 Occupancies with 50 or more resident incapacitated patients, but not included in Category 1 Group I, Division 3 Occupancies All structures with an occupancy greater than 5,000 persons Structures and equipment in power-generating stations, and other public utility facilities not included in Category 1 or Category 2 above, and required for continued operation	1.00	1.00	1.00
4. Standard occupancy structures ³	All structures housing occupancies or having functions not listed in Category 1, 2 or 3 and Group U Occupancy towers	1.00	1.00	1.00
5. Miscellaneous structures	Group U Occupancies except for towers	1.00	1.00	1.00

¹The limitation of I_p for panel connections in Section 1633.2.4 shall be 1.0 for the entire connector.²Structural observation requirements are given in Section 1702.³For anchorage of machinery and equipment required for life-safety systems, the value of I_p shall be taken as 1.5.

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Lampiran 16 : Tabel 16-Q Seismic Coefficient

TABLE 16-Q—SEISMIC COEFFICIENT C_s

SOIL PROFILE TYPE	SEISMIC ZONE FACTOR, Z				
	$Z = 0.075$	$Z = 0.15$	$Z = 0.2$	$Z = 0.3$	$Z = 0.4$
S_A	0.06	0.12	0.16	0.24	$0.32N_a$
S_B	0.08	0.15	0.20	0.30	$0.40N_a$
S_C	0.09	0.18	0.24	0.33	$0.40N_a$
S_D	0.12	0.22	0.28	0.36	$0.44N_a$
S_E	0.19	0.30	0.34	0.36	$0.36N_a$
S_F	See Footnote 1				

¹Site-specific geotechnical investigation and dynamic site response analysis shall be performed to determine seismic coefficients for Soil Profile Type S_F .