

**DESIGN OF PPM BUILDING IN ACCORDANCE WITH
SNI 03 – 2847 – 2002 AND SNI 03 – 1726 – 2002**

Final Project

By:

**LUKAS NUGROHO WICAKSONO
Student Number: 03 13 11502**



**ATMA JAYA YOGYAKARTA UNIVERSITY
FACULTY OF ENGINEERING
DEPARTMENT OF CIVIL ENGINEERING
INTERNATIONAL S1 PROGRAM
JULY 2011**

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Final Project

DESIGN OF PPM BUILDING IN ACCORDANCE WITH SNI 03 – 2847 – 2002 AND SNI 03 – 1726 – 2002

By:

LUKAS NUGROHO WICAKSONO
Student Number: 03 13 11502


has been examined and approved by the examination committee

Signature

Date

Chairperson

: Prof. Ir. Yoyong Arfiadi, M.Eng., Ph.D.

 July 7, 2011


Member

: DR. Ir. AM. Ade Lisantono, M.Eng

 July 07, 2011

Member

: Ir. Junaedi Utomo, M.Eng.

 July 11, 2011

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Final Project

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By:


LUKAS NUGROHO WICAKSONO

Student Number: 03 13 11502

has been approved

Yogyakarta, *July 11, 2011*

Advisor



(Prof. Ir. Yoyong Arfiadi, M.Eng., Ph.D.)

Head of Civil Engineering Department



(Ir. Junaedi Utomo, M.Eng)

PREFACE

First and foremost, I would like to thank God for His blessing that has been given to me, so that I could prepare and finish this final project report. This report was arranged, due to finish the S1 degree at Faculty of Engineering, Department of Civil Engineering, Atma Jaya University Yogyakarta.

In this final project, I design “PPM Building” by using software ETABS non linier version 9.7 and design based on SNI 2002.

I would like express my appreciation to:

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I realized that, this report has some mistakes maybe, but I trust all critics from all of you can make it better. Finally I hope this report could give advantages for the readers.

Yogyakarta, July 2011

Lukas Nugroho Wicaksono

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ABSTRACT

DESIGN OF PPM BUILDING IN ACCORDANCE WITH SNI 03 – 2847 – 2002 AND SNI 03 – 1726 – 2002, Lukas Nugroho Wicaksono, Student Number: 11502, International Civil Engineering, Atma Jaya Yogyakarta University.

In earthquake prone area, buildings have to be designed according to the criteria specified based on earthquake loading code. This final project will consider PPM building which is located in Menteng Raya Streer. This building will be designed based on SNI 03 2847 - 2002 and SNI 03 1729 -2002. This building will be considered as Intermediate Moment Resisting Frame (IMRF) system and using concrete quality $f_c' = 25\text{MPa}$ and steel quality $f_y = 400\text{MPa}$ for steel that has more than 12mm diameter in size and $f_y = 240\text{MPa}$ for steel that has less or equal 12mm diameter in size.

In analyzing PPM building, ETABS non linear were used to obtain internal forces of the structure and stairs. For columns design interaction diagram of was used in order to obtain the longitudinal reinforcement. For designing the shear reinforcement of beams, ETABS was used by setting all of the structure to Frame Moment Release at the start and the end to simulate plasric hinge occurred at both ends of beam.

For the seismic design, the natural period fulfill the requirement, considering the story drift and the T-rayleigh are okay, so the building is considered okay. V static is used because V dynamic is less than $0.8 \times V$ static.

From structural design result, the entire slab are designed as two-way slab with thickness =120 mm and reinforcement bar 10 mm. Stair has the thickness 150 mm, with reinforcement bar 10 mm. Dimension of beam B134 at story 3 is 400x900, longitudinal reinforcement for support area are 9D25 (top reinforcement), 5D25 (bottom reinforcement), longitudinal reinforcement for midspan area are 4D25 (top reinforcement), 5D25 (bottom reinforcement). Dimension of column C28 at story 3 is 1100x1100 mm, longitudinal reinforcement has 52D25.

Keywords: design, intermediate moment resisting frame, reinforcement