

## CHAPTER V

### CONCLUSION AND SUGGESTION

#### 5.1 Conclusion

According to the result of table 4.5, it shows that the members that are designed based on maximum capacity of the bracing are much greater than based on overstrength factor  $P_u = \Omega_o P_e = 2 P_e$  (Option I AISC 341-2005). It indicates that the member which are designed based on the overstrength factor can not accommodate the local effect, caused by the redistribution forces from brace buckling, which increase drastically column axial force.

#### 5.2 Suggestion

SCBF column design with two story X-Bracing is designed based on the capacity of the brace, which will produce heavier column than design based on the provisions of AISC 341-05 to anticipate redistribution forces due to buckling forces on the brace.

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